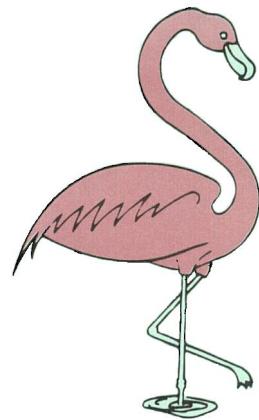


# **PARK STREET COMMERCE CENTER TRAFFIC STUDY**

OKEECHOBEE COUNTY, FLORIDA

July 2023



**BUCKHOLZ TRAFFIC**



**BUCKHOLZ TRAFFIC  
3585 KORI ROAD  
JACKSONVILLE, FLORIDA 32257  
(904) 886-2171    jwbuckholz@aol.com**

July 11, 2023

Mr. Johnny Herbert IV, P.E.  
American Civil Engineering Company  
207 N. Moss Road / #211  
Winter Springs, Florida 32708

**Re: Park Street Commerce Center, Revised Traffic Study**

Dear Mr. Herbert:

Attached is the revised traffic study. If there are any questions or comments regarding this study, please contact me.

Sincerely,

Jeffrey W. Buckholz, P.E., PTOE  
Principal

This item has been digitally signed and sealed by Jeffrey W. Buckholz, P.E. on the date indicated on the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

## INTRODUCTION

This proposed mixed-use development will include a 4600 sf automated car wash, a 5000 sf fast food restaurant with drive-thru window, a 100 room hotel, 52 apartment units, and 10,000 sf of retail space. The fast food restaurant will open at 10 AM and the car wash will open at 9:00 AM. The development will be located in the northwest quadrant of the NE Park Street/SE 13<sup>th</sup> Avenue intersection in Okeechobee, Florida. A cul-de-sac road that extends 13<sup>th</sup> Avenue to the north will be installed on site property and access will be provided to the various land uses from this road. NE Park Street (SR 70) is a four lane divided urban major arterial with an FDOT access management classification of 7 and a posted speed limit of 35 mph. SE 13<sup>th</sup> Avenue is a two lane undivided local road with a posted speed limit of 25 mph.

Figure 1 shows the site location and surrounding road network while Appendix A contains the proposed site plan. The development is expected to be complete and fully occupied by the end of 2024. Consequently, 2024 was chosen as the design year for this study.

## EXISTING TRAFFIC VOLUMES

Weekday peak period manual turning movement counts were conducted by Buckholz Traffic personnel at the intersection of NE Park Street with SE 13<sup>th</sup> Avenue and with the Hampton Inn driveway located west of SE 13<sup>th</sup> Avenue. These counts, which are provided in Appendix B, were conducted during the weekday AM peak period (6:45-8:45 AM) and the weekday PM peak period (3:45 – 6:00 PM) with school in session. The data was recorded at 15-minute intervals and includes a separate tabulation for trucks and pedestrians. Figure 2 graphically summarizes the AM and PM peak hour traffic counts.

Appendix C provides daily traffic volumes from two nearby FDOT traffic count stations on SR 70. The existing average daily traffic on NE Park Street in the vicinity of the site is about 29,000 vehicles per day.

## TRIP GENERATION

Trip generation calculations were carried out using the 11th edition of ITE's Trip Generation Manual by referencing land use codes 948 (Automated Car Wash), 934 (Fast Food Restaurant with Drive-Thru Window), 312 (Business Hotel), 220 (Low Rise Multifamily Housing) and 822 (Strip Retail Plaza). Tables 1 through 5 contain the daily, AM peak hour, and PM peak hour trip generation calculations. During an average weekday the development is expected to generate 4424 trips (2212 entering and 2212 exiting) with 87 trips (36 entering and 51 exiting) occurring during the AM peak hour and 369 trips (193 entering and 176 exiting) occurring during the PM peak hour.

## SITE TRIP DISTRIBUTION AND TRAFFIC ASSIGNMENT

Weekday AM and PM peak hour site trips for this commercial development were directionally distributed based on engineering judgment after reviewing the trip distribution percentages used in the 2020 traffic study for the nearby RaceTrac commercial development. The results are provided in Figures 3 and 4.

## **FUTURE TRAFFIC VOLUMES**

The expected weekday 2024 peak hour background (No Build) traffic volumes and total (Build) traffic volumes at intersections of interest are graphically depicted in Figures 5 through 8. The No Build traffic volumes were obtained by multiplying the existing traffic volumes by the appropriate FDOT seasonal adjustment factor (0.96) and then by an annual growth rate. A linear regression analysis of FDOT daily traffic counts in the area (see graphs C-1 and C-2 in Appendix C) indicates that daily traffic volumes have been increasing at an average annual rate of 1.5% over recent years. The 2024 Build traffic volumes were obtained by adding the traffic generated by the new development to the 2024 No Build traffic volumes.

## **TURN LANE EVALUATION**

A formal analysis was made to determine if a right turn lane is warranted on westbound NE Park Street at the two new roadways: NE 13<sup>th</sup> Avenue and NE 12<sup>th</sup> Avenue. The methodology contained in NCHRP Report 279 was used to conduct this analysis. As is indicated in Figures 9 and 10, right turn volumes under expected 2024 Build conditions will be high enough to warrant an exclusive right turn lane at NE 13<sup>th</sup> Avenue but will not be high enough to warrant an exclusive right turn lane at NE 12<sup>th</sup> Avenue. However, NCHRP Report 420 - which requires 110 right turns per hour to warrant a right turn lane on a multi-lane roadway with a posted speed of 45 mph or less ó does not support the installation of an exclusive right turn lane at either location.

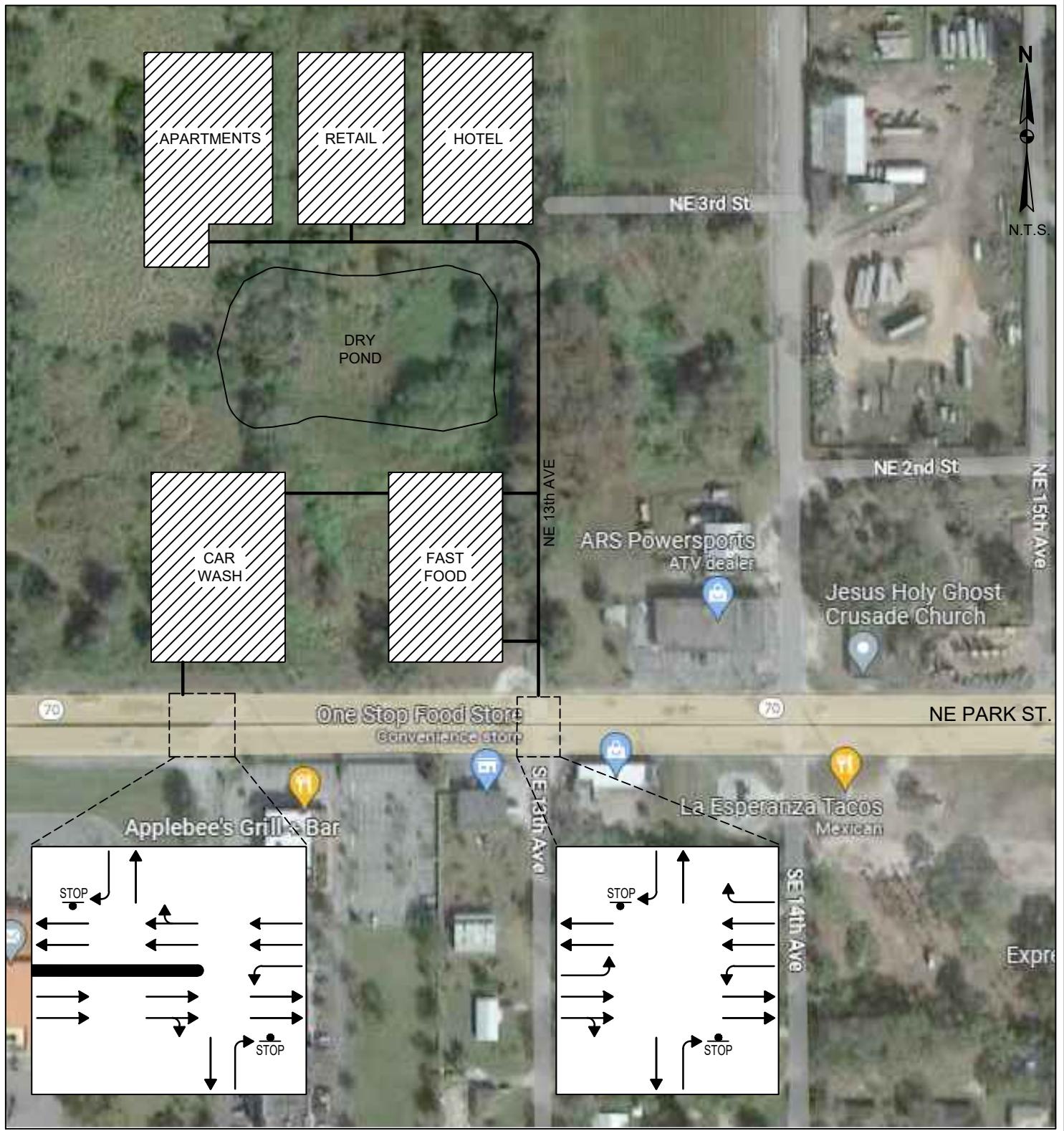
## **UNSIGNALIZED INTERSECTION CAPACITY ANALYSIS**

The NE Park Street/13th Avenue intersection and the NE Park Street/Hampton Inn Driveway/NE 12<sup>th</sup> Avenue intersection were analyzed using the two-way stop control methodology contained in the year 2023 version of the Highway Capacity Software. The supporting calculations are provided in Appendix D. Table 6 summarizes the capacity analysis results under existing conditions while Table 7 summarizes the capacity analysis results under 2024 Build conditions.

All minor movements currently operate at level of service C or better during both weekday peak hours at the NE Park Street/SE 13<sup>th</sup> Avenue intersection with minimal queuing and a volume-to-capacity ratio well below one. Under 2024 Build conditions at the new NE Park Street/13<sup>th</sup> Avenue intersection with dual directional median opening all minor movements are expected to continue to operate at level of service C or better during both peak hours with minimal queuing and a volume-to-capacity ratio still well below one.

At the NE Park Street/Hampton Inn Driveway intersection all minor movements currently operate at level of service B or better during both weekday peak hours with minimal queuing and a volume-to-capacity ratio well below one.

Under 2024 Build conditions all minor movements at the NE Park Street/Hampton Inn Driveway/NE 12<sup>th</sup> Avenue intersection are expected to operate at level of service C or better during both weekday peak hours ó with one exception. The westbound left turn is expected to operate at level of service F during the PM peak hour. However, only moderate queuing and a volume-to-capacity ratio below one are expected for this movement movements.



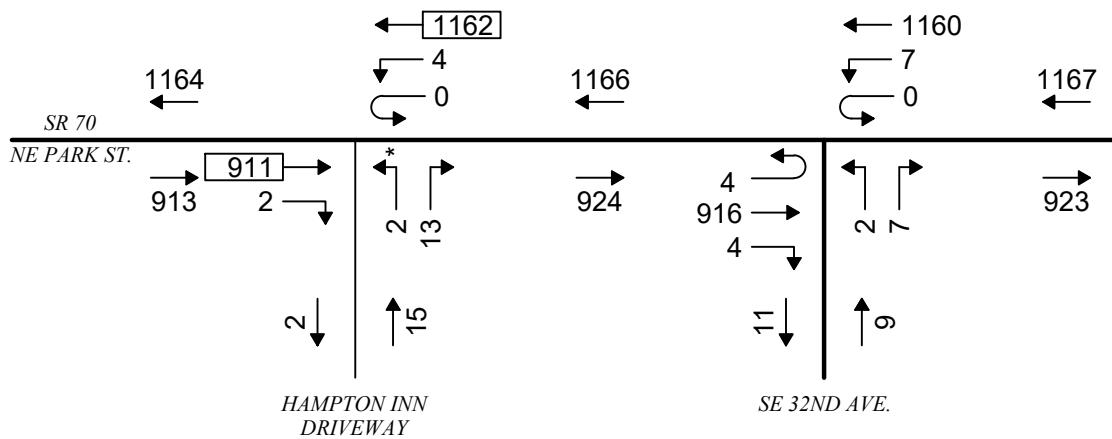
= DIRECTIONAL MEDIAN OPENING

Buckholz Traffic

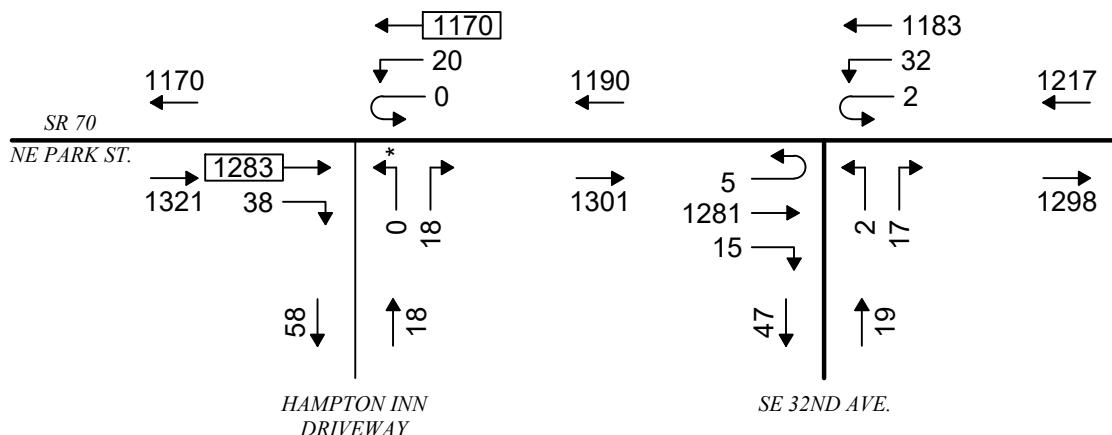
FIGURE 1  
SITE LOCATION AND  
PROPOSED INTERSECTION  
LAYOUTS



7:15-8:15 AM



5:00-6:00 PM



[XXX] = CALCULATED VALUE

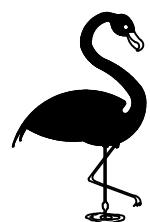
\* = ILLEGAL TURN

Buckholz Traffic

FIGURE 2

TRAFFIC  
COUNTS

WEEKDAY PEAK HOURS



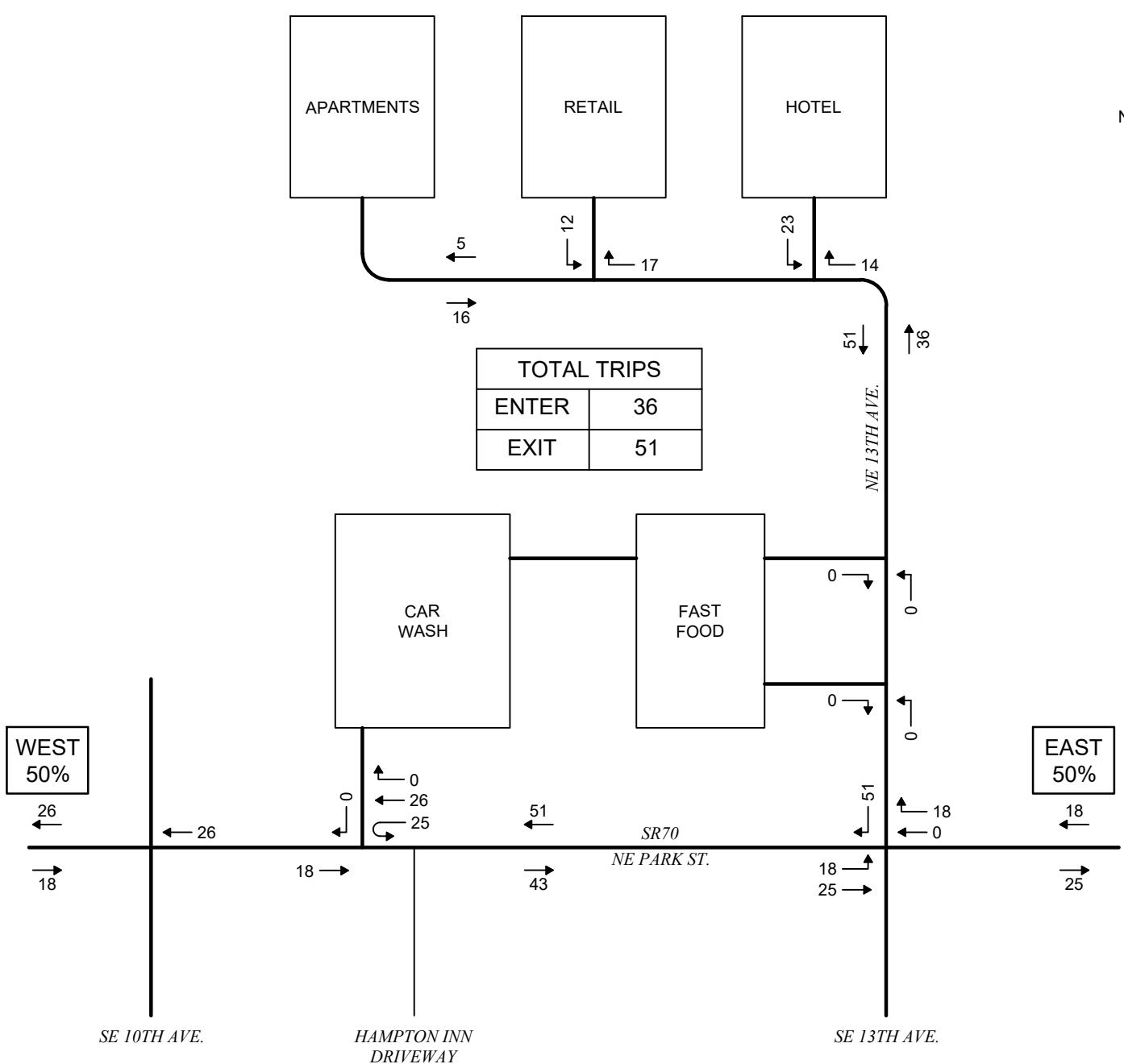


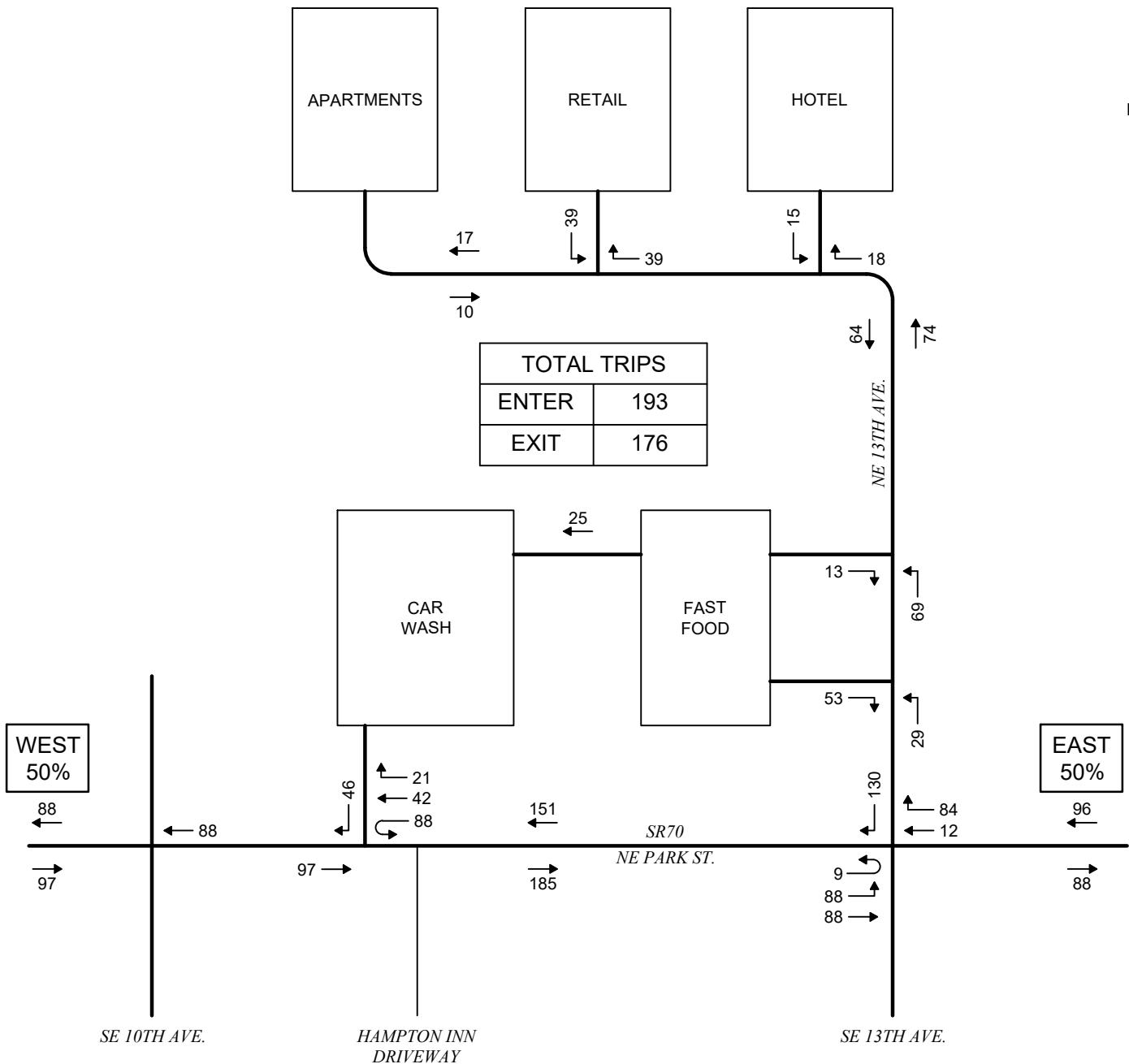
FIGURE 3

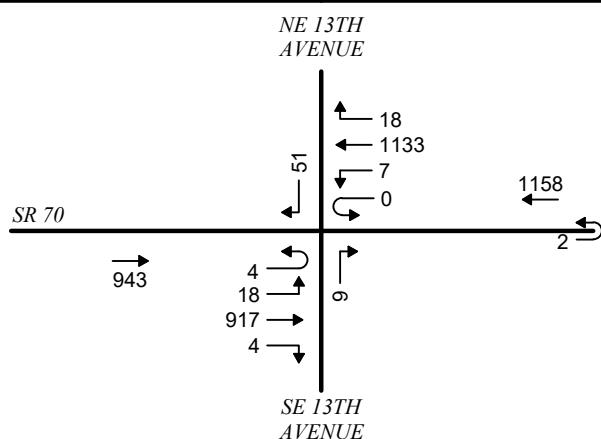
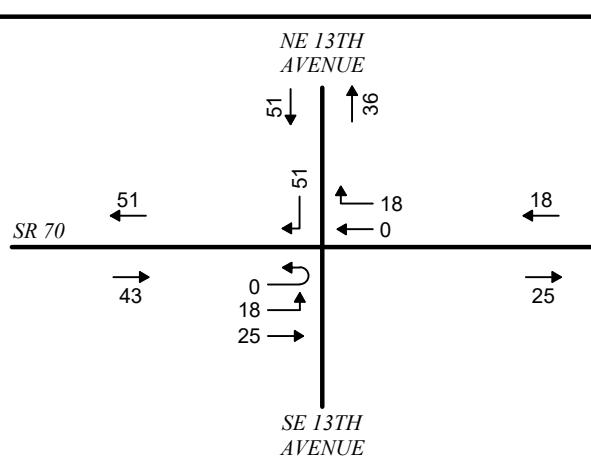
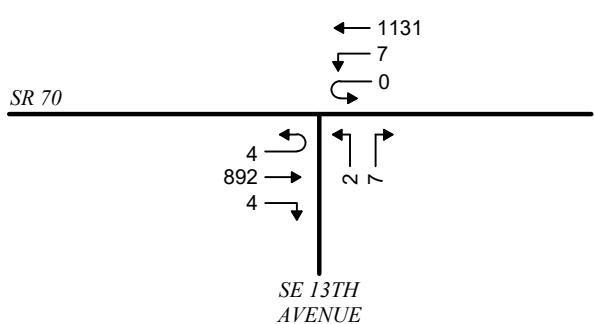
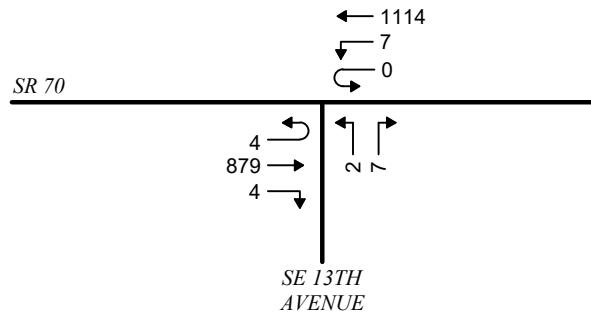
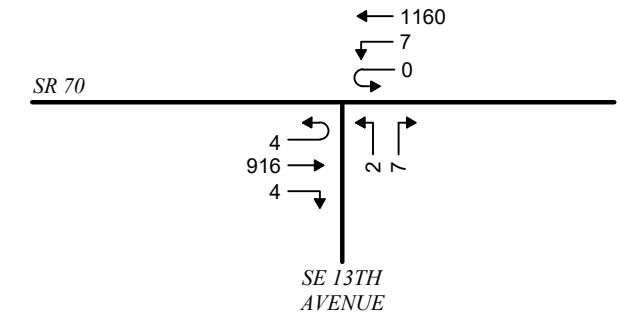
SITE TRAFFIC  
ASSIGNMENT

WEEKDAY AM PEAK HOUR

Buckholz Traffic







2024 BUILD TRAFFIC

Buckholz Traffic

FIGURE 5  
2024 BUILD TRAFFIC  
SR 70 / 13TH AVENUE  
WEEKDAY AM PEAK HOUR



N

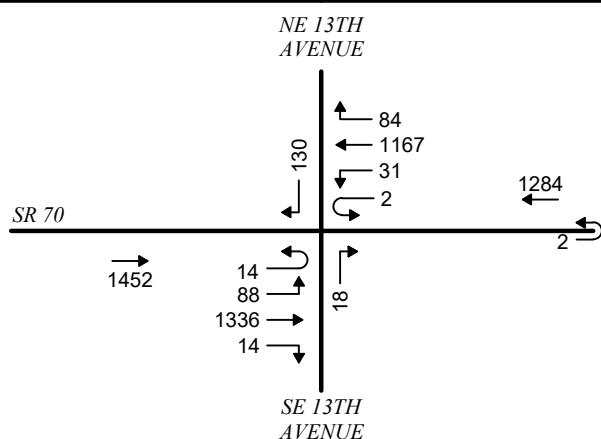
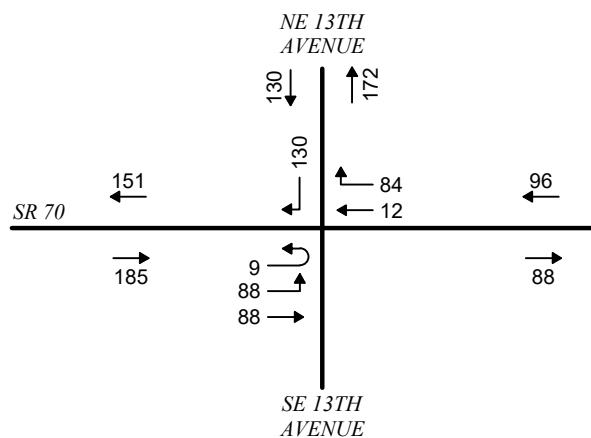
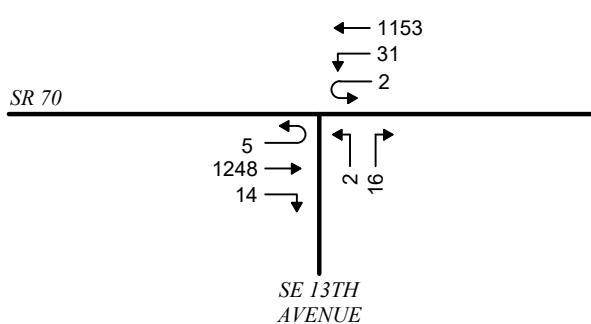
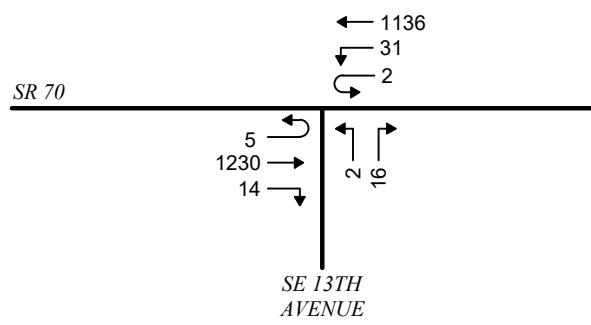
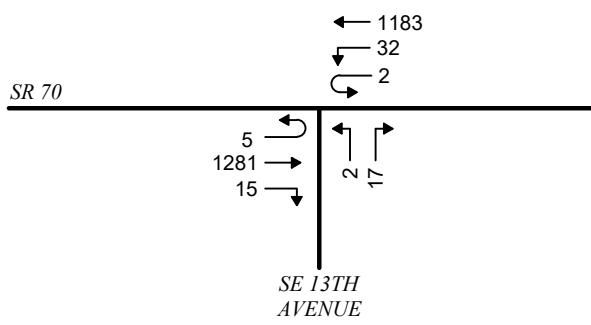
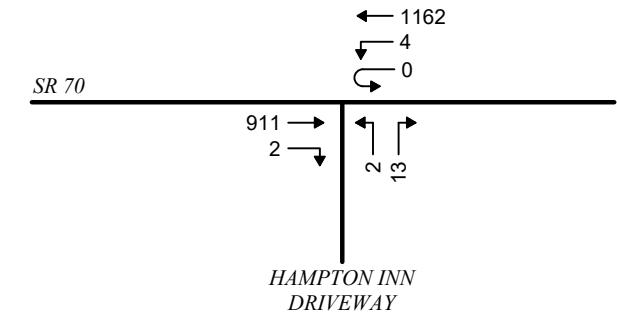


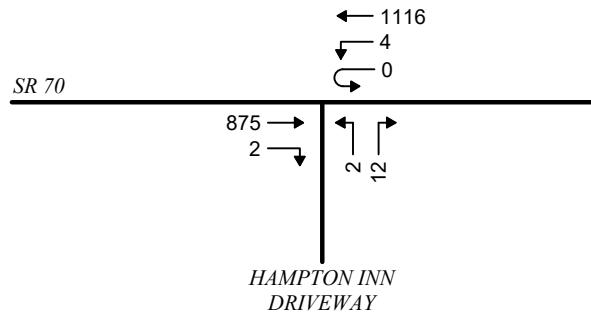
FIGURE 6  
2024 BUILD TRAFFIC  
SR 70 / 13TH AVENUE  
WEEKDAY PM PEAK HOUR



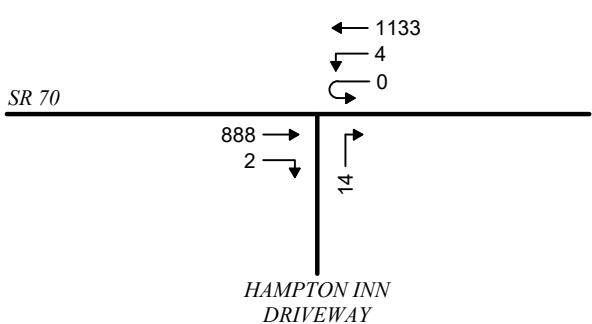
Buckholz Traffic



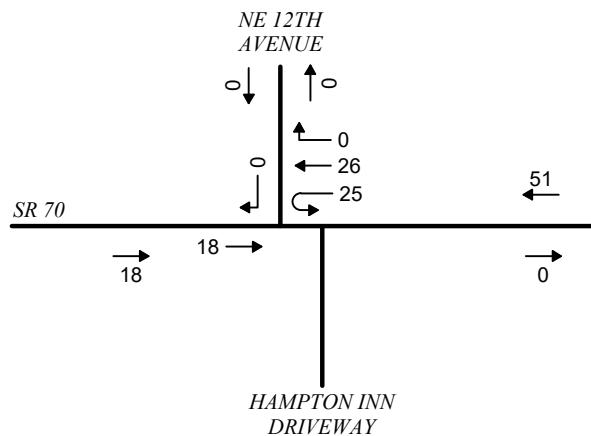
EXISTING TRAFFIC  
02/13/23  
7:15-8:15 AM



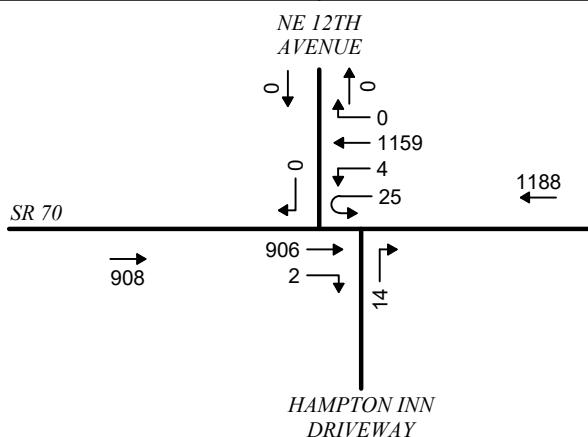
2023 SEASONALLY ADJUSTED TRAFFIC  
FDOT SEASONAL CORRECTION FACTOR = 0.96



2024 NO BUILD TRAFFIC  
AVERAGE ANNUAL GROWTH RATE = 1.5%



SITE TRAFFIC

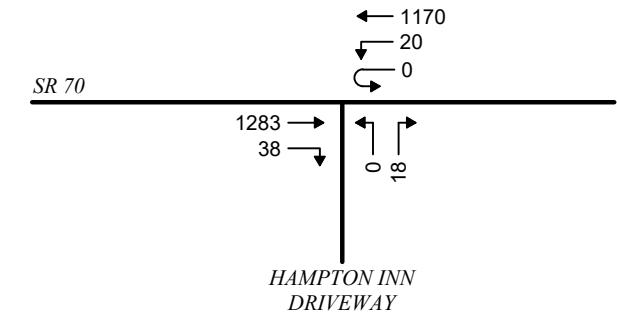


2024 BUILD TRAFFIC

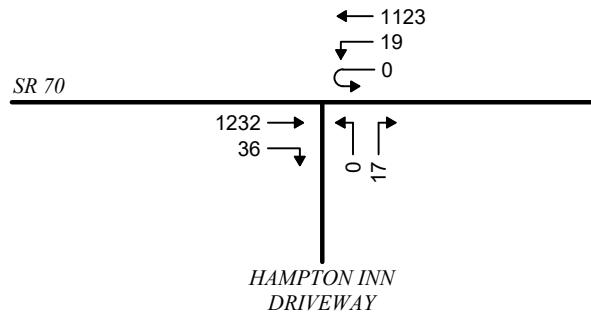
Buckholz Traffic

FIGURE 7  
2024 BUILD TRAFFIC  
SR 70 / HAMPTON DRIVEWAY  
WEEKDAY AM PEAK HOUR

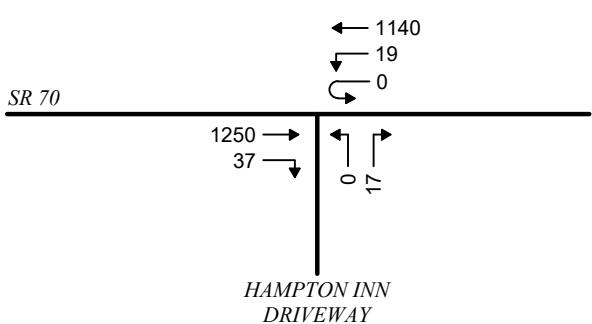




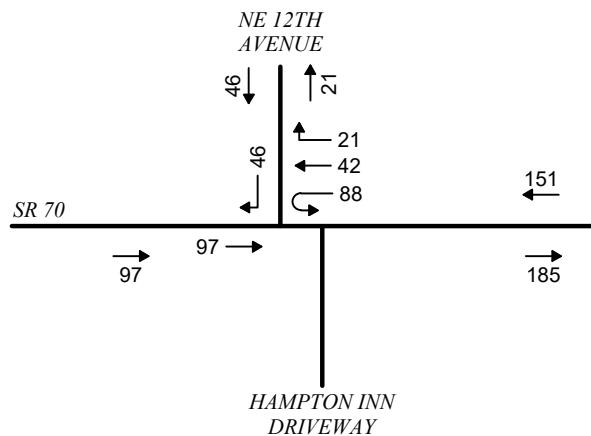
EXISTING TRAFFIC  
02/13/23  
5:00-6:00 PM



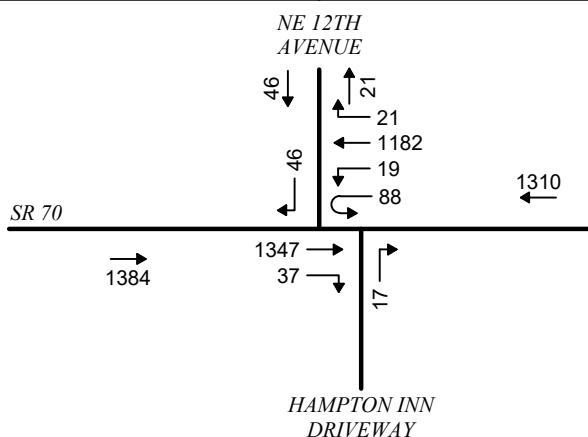
2023 SEASONALLY ADJUSTED TRAFFIC  
FDOT SEASONAL CORRECTION FACTOR = 0.96



2024 NO BUILD TRAFFIC  
AVERAGE ANNUAL GROWTH RATE = 1.5%



SITE TRAFFIC



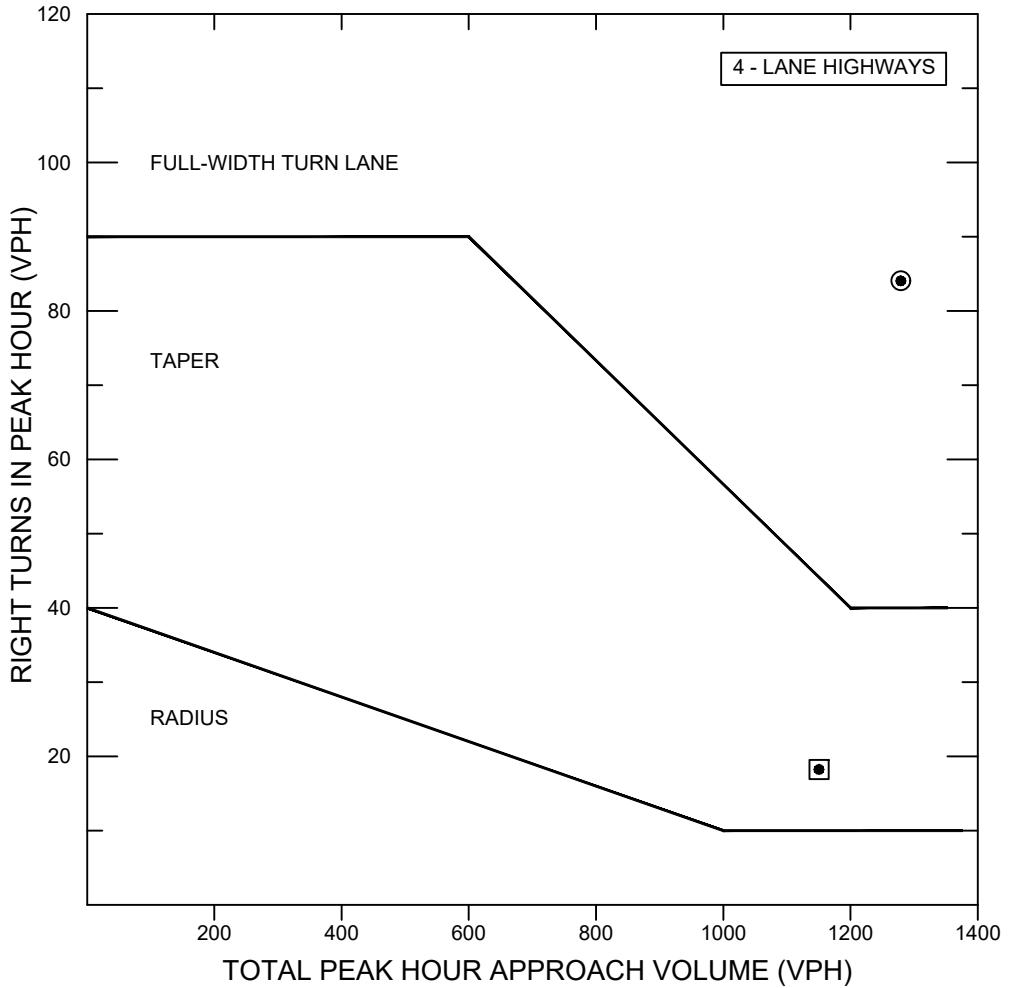
2024 BUILD TRAFFIC

Buckholz Traffic

FIGURE 8  
2024 BUILD TRAFFIC  
SR 70 / HAMPTON DRIVEWAY  
WEEKDAY PM PEAK HOUR



## WESTBOUND NE PARK STREET @ NE 13TH AVENUE



### NOMOGRAPH FOR RIGHT TURN LANES

SOURCE: TRANSPORTATION RESEARCH BOARD NCHRP REPORT #279

WEEKDAY AM PEAK  
HOUR

V <sub>A</sub>	1158
V <sub>R</sub>	18

WEEKDAY PM PEAK  
HOUR

V <sub>A</sub>	1284
V <sub>R</sub>	84

NCHRP 420	
MULTI- LANE	≤ 45 MPH

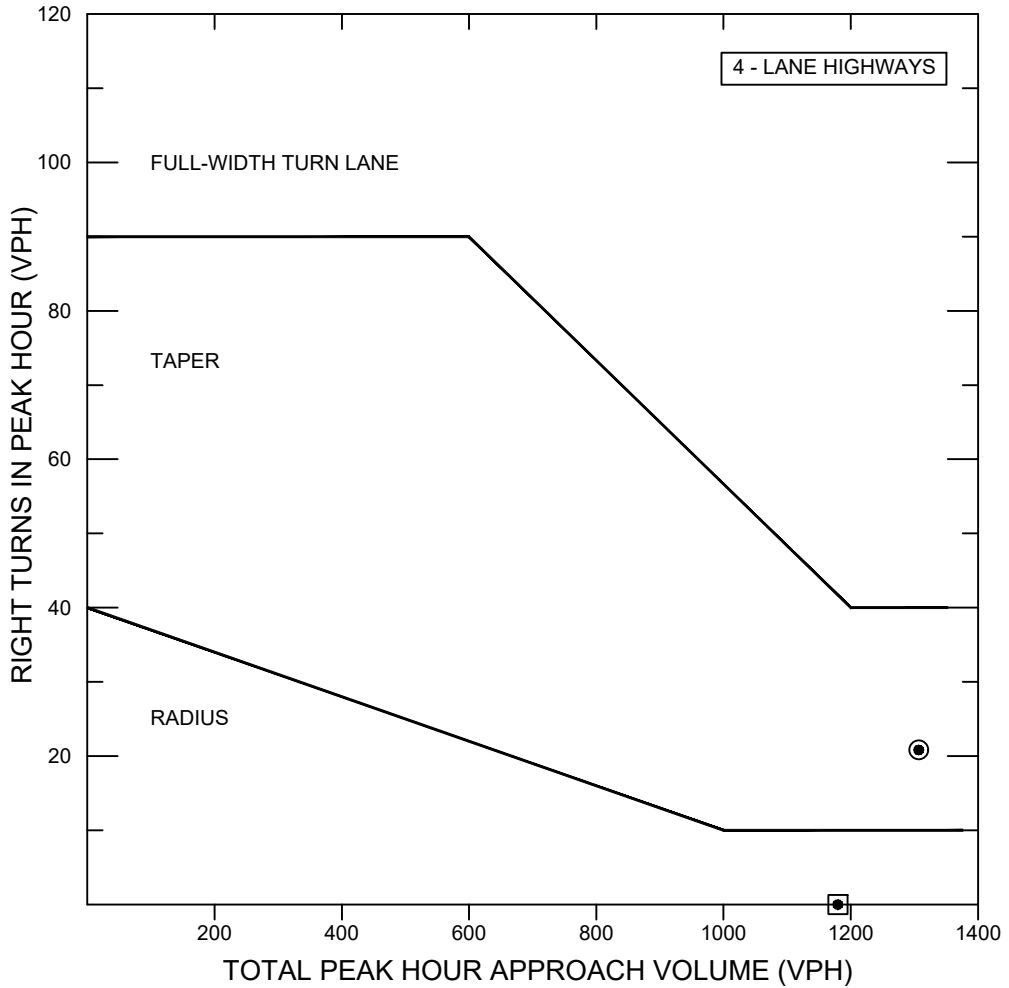
18 & 84 < 110 REQUIRED

FIGURE 9

RIGHT TURN LANE  
ANALYSIS



## WESTBOUND NE PARK STREET @ NE 12TH AVENUE



### NOMOGRAPH FOR RIGHT TURN LANES

SOURCE: TRANSPORTATION RESEARCH BOARD NCHRP REPORT #279

WEEKDAY AM PEAK  
HOUR

V <sub>A</sub>	1188
V <sub>R</sub>	0

WEEKDAY PM PEAK  
HOUR

V <sub>A</sub>	1310
V <sub>R</sub>	21

NCHRP 420	
MULTI-LANE	≤ 45 MPH

0 & 21 < 110 REQUIRED

FIGURE 10

RIGHT TURN LANE  
ANALYSIS



**TABLE 1****TRIP GENERATION CALCULATIONS****AUTOMATED CAR WASH**

Land Use Code 948

T = Number of Vehicle Trip Ends

X = 4600 GSF = 4.6

<u>TIME PERIOD</u>	<b>TOTAL</b> TRIP GENERATION <u>EQUATION</u>	<b>TOTAL</b> TRIP ENDS	PERCENT <u>ENTERING</u>	PERCENT <u>EXITING</u>	<b>TOTAL</b> TRIP ENDS <u>ENTERING</u>	<b>TOTAL</b> TRIP ENDS <u>EXITING</u>
	<b>AVERAGE WEEKDAY</b>					
Daily	<b>T = 14.2/8.7% (X)</b>	750	50%	50%	375	375
AM Peak Hour	<b>NOT OPEN</b>					
PM Peak Hour	T = 14.20 (X)	66	50%	50%	33	33

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

**Estimated Using ITE Hourly Percentages from LUC 949****BUCKHOLZ TRAFFIC**

**TABLE 2**  
**TRIP GENERATION CALCULATIONS**

**FAST-FOOD RESTAURANT WITH DRIVE-THRU WINDOW**

Land Use Code 934

T = Number of Vehicle Trip Ends

Size of Building = 5000 (X = 5.0)

<u>TIME PERIOD</u>	<b>TOTAL</b> TRIP GENERATION <u>EQUATION</u>	<b>TOTAL</b> TRIP ENDS	<u>PERCENT</u> <u>ENTERING</u>	<u>PERCENT</u> <u>EXITING</u>	<b>TOTAL</b> TRIP ENDS <u>ENTERING</u>	<b>TOTAL</b> TRIP ENDS <u>EXITING</u>	
<b>AVERAGE WEEKDAY</b>							
Daily	T = 467.48 (X)	2338	50%	50%	1169	1169	
AM Peak Hour			<b>NOT OPEN</b>				
PM Peak Hour	T = 33.03 (X)	165	52%	48%	86	79	

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

<u>TIME PERIOD</u>	<b>NEW</b> TRIP ENDS	<u>PERCENT</u> <u>ENTERING</u>	<u>PERCENT</u> <u>EXITING</u>	<b>NEW</b> TRIP ENDS <u>ENTERING</u>	<b>NEW</b> TRIP ENDS <u>EXITING</u>		
	<b>PERCENT NEW TRIPS</b>						
<b>AVERAGE WEEKDAY</b>							
Daily	<b>52%</b>	1216	50%	50%	608		
AM Peak Hour	<b>50%</b>		<b>NOT OPEN</b>				
PM Peak Hour	<b>55%</b>	91	52%	48%	47	44	

SOURCE: ITE, "Trip Generation", 11th Edition (2021), Excel Pass-By Tables

**Estimated Value**

**BUCKHOLZ TRAFFIC**

**TABLE 3**  
**TRIP GENERATION CALCULATIONS**

**BUSINESS HOTEL**

Land Use Code 312

T = Number of Vehicle Trip Ends

X = Rooms = 100

<u>TIME PERIOD</u>	<u>TOTAL TRIP GENERATION EQUATION</u>	<u>TOTAL TRIP ENDS</u>	<u>PERCENT ENTERING</u>	<u>PERCENT EXITING</u>	<u>TOTAL TRIP ENDS ENTERING</u>	<u>TOTAL TRIP ENDS EXITING</u>
<b>AVERAGE WEEKDAY</b>						
Daily	$T = 2.90 (X) + 151.69$	442	50%	50%	221	221
AM Peak Hour	$T = 0.30 (X) + 6.94$	37	39%	61%	14	23
PM Peak Hour	$T = 0.21 (X) + 12.03$	33	55%	45%	18	15

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

**BUCKHOLZ TRAFFIC**

**TABLE 4**  
**TRIP GENERATION CALCULATIONS**

**MULTIFAMILY HOUSING (LOW-RISE)**  
**Not Close to Rail Transit**

Land Use Code 220

T = Number of Vehicle Trip Ends

X = Number of Dwelling Units = 52

<u>TIME PERIOD</u>	TRIP GENERATION <u>EQUATION</u>	TOTAL TRIP ENDS	TOTAL		TOTAL TRIP ENDS <u>ENTERING</u>	TOTAL TRIP ENDS <u>EXITING</u>
			PERCENT <u>ENTERING</u>	PERCENT <u>EXITING</u>		
<b>WEEKDAY</b>						
Daily	T = 6.74 (X)	350	50%	50%	175	175
AM Peak Hour	T = 0.40 (X)	21	24%	76%	5	16
PM Peak Hour	T = 0.51 (X)	27	63%	37%	17	10

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

**BUCKHOLZ TRAFFIC**

**TABLE 5**  
**TRIP GENERATION CALCULATIONS**

**STRIP RETAIL PLAZA (Less Than 40,000 gsf)**

Land Use Code 822

T = Number of Vehicle Trip Ends

Size of Buildings = 10,000 gsf ----->  $\times 10.0$

<u>TIME PERIOD</u>	<u>TOTAL</u> <u>TRIP GENERATION</u>	<u>TOTAL</u> <u>TRIP ENDS</u>	<u>PERCENT</u> <u>ENTERING</u>	<u>PERCENT</u> <u>EXITING</u>	<u>TOTAL</u> <u>TRIP ENDS</u>	<u>TOTAL</u> <u>TRIP ENDS</u>
	<u>EQUATION</u>				<u>ENTERING</u>	<u>EXITING</u>
<b>AVERAGE WEEKDAY</b>						
Daily	$T = 54.45 X$	544	50%	50%	272	272
AM Peak Hour	$\ln(T) = 0.66\ln(X) + 1.84$	29	60%	40%	17	12
PM Peak Hour	$\ln(T) = 0.71\ln(X) + 2.72$	78	50%	50%	39	39

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

**NEW TRIPS**

<u>TIME PERIOD</u>	<u>PERCENT NEW TRIPS</u>	<u>NEW</u> <u>TRIP ENDS</u>	<u>PERCENT</u> <u>ENTERING</u>	<u>PERCENT</u> <u>EXITING</u>	<u>NEW</u> <u>TRIP ENDS</u>	<u>NEW</u> <u>TRIP ENDS</u>
					<u>ENTERING</u>	<u>EXITING</u>
<b>AVERAGE WEEKDAY</b>						
Daily	<b>64.0%</b>	348	50%	50%	174	174
AM Peak Hour	<b>64.0%</b>	18	60%	40%	11	7
PM Peak Hour	64.0%	50	50%	50%	25	25

SOURCE: ITE "Trip Generation Handbook", 3rd Edition, Table E.9

**Estimated Value**

**BUCKHOLZ TRAFFIC**

**TABLE 6**  
**UNSIGNALIZED INTERSECTION CAPACITY RESULTS**  
**EXISTING CONDITIONS**

**NE PARK STREET / SE 13TH AVENUE**

<b>WEEKDAY AM PEAK HOUR</b>				
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Eastbound U-Turn	C	21.9 sec/veh	0.02	1
Westbound Left Turn	B	11.0 sec/veh	0.01	1
Northbound Approach	C	15.6 sec/veh	0.03	1

<b>WEEKDAY PM PEAK HOUR</b>				
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Eastbound U-Turn	C	20.8 sec/veh	0.02	1
Westbound Left Turn	B	13.7 sec/veh	0.08	1
Northbound Approach	C	16.6 sec/veh	0.06	1

**NE PARK STREET / HAMPTON INN DRIVEWAY**

<b>WEEKDAY AM PEAK HOUR</b>				
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Westbound Left Turn	B	10.4 sec/veh	0.01	1
Northbound Right Turn	B	12.3 sec/veh	0.03	1

<b>WEEKDAY PM PEAK HOUR</b>				
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Westbound Left Turn	B	12.7 sec/veh	0.04	1
Northbound Right Turn	B	14.9 sec/veh	0.05	1

**TABLE 7**  
**UNSIGNALIZED INTERSECTION CAPACITY RESULTS**  
**2024 BUILD CONDITIONS**

**NE PARK STREET / 13<sup>TH</sup> AVENUE**

WEEKDAY AM PEAK HOUR				
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Eastbound Left Turn	C	15.1 sec/veh	0.07	1
Westbound Left Turn	B	11.3 sec/veh	0.01	1
Northbound Approach	B	13.7 sec/veh	0.02	1
Southbound Approach	C	15.4 sec/veh	0.15	1

WEEKDAY PM PEAK HOUR				
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Eastbound Left Turn	C	18.9 sec/veh	0.30	1.3
Westbound Left Turn	B	14.9 sec/veh	0.09	1
Northbound Approach	C	15.5 sec/veh	0.05	1
Southbound Approach	C	18.1 sec/veh	0.34	1.5

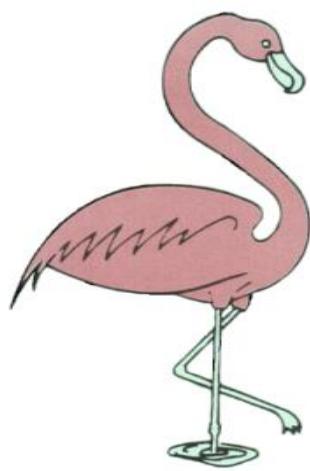
**NE PARK STREET / NE 12<sup>TH</sup> AVENUE / HAMPTON INN DRIVEWAY**

WEEKDAY AM PEAK HOUR				
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Westbound Left Turn	C	17.6 sec/veh	0.11	1
Northbound Right Turn	B	12.5 sec/veh	0.03	1
Southbound Right Turn	B	14.0 sec/veh	0.00	1

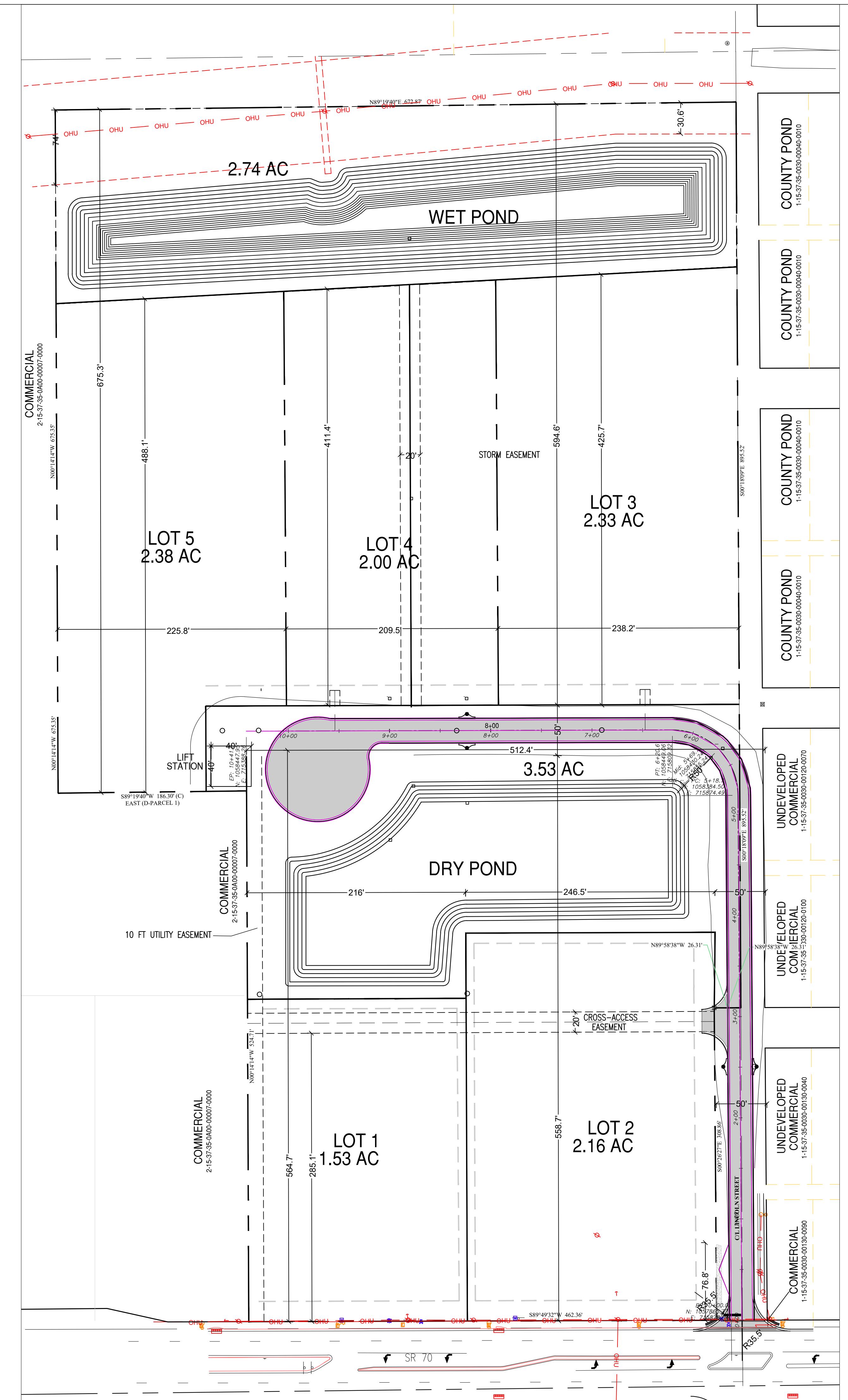
WEEKDAY PM PEAK HOUR				
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Westbound Left Turn	<b>F</b>	68.7 sec/veh	0.72	<b>4.3</b>
Northbound Right Turn	C	15.9 sec/veh	0.05	1
Southbound Right Turn	C	15.2 sec/veh	0.12	1

## **APPENDIX A**

### **SITE PLAN**







## DEVELOPMENT INFORMATION

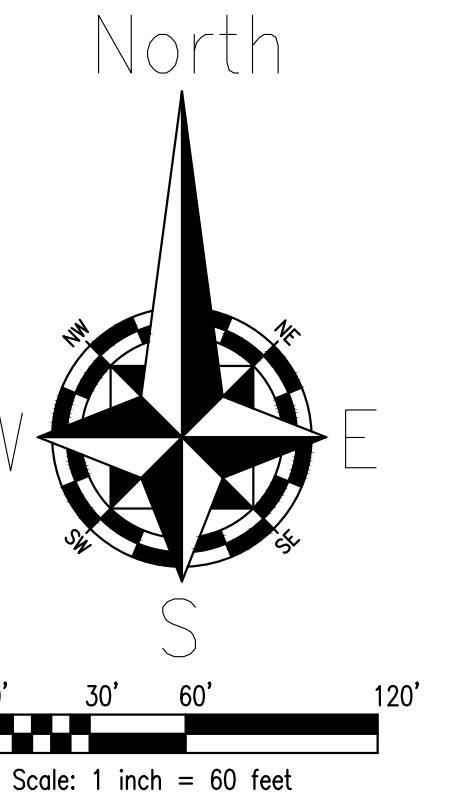
- |  |                                 |          |  |
|--|---------------------------------|----------|--|
| 1. PROJECT NAME:   | PARK STREET COMMERCE CENTER     |          |  |
| 2. TAX ID NUMBER:  | 2-15-37-35-0A00-00009-A000      | 0.485 AC |  |
|  | 2-15-37-35-0A00-00009-0000      | 6.626 AC |  |
|  | 2-15-37-35-0A00-00010-0000      | 2.129 AC |  |
|  | 2-15-37-35-0A00-00011-0000      | 4.950 AC |  |
|  | 2-15-37-35-0A00-00010-0010      | 2.028 AC |  |
| 3. TOTAL DEVELOPMENT AREA:   | 16.151 AC                       |          |  |
| 5. EXISTING USE:   | VACANT                          |          |  |
| 6. PROPOSED USE:   | MIXED USE COMMERCIAL            |          |  |
| 7. EXISTING LAND USE:  | COMMERCIAL                      |          |  |
| 8. EXISTING ZONING:  | COMMERCIAL HEAVY DISTRICT (CHV) |          |  |
| 9. MAX. BUILDING HEIGHT:   | 45 ft.                          |          |  |
| 10. 8" WATER UTILITY AND LIFT STATION CONNECTING TO GRAVITY SEWER SERVICE SERVED BY OKCHOBEE UTILITY AUTHORITY   |                                 |          |  |
| 11. THIS PROJECT WILL BE CONSTRUCTED IN ONE PHASE. INDIVIDUAL LOTS TO BE PERMITTED INDIVIDUALLY AT SEPARATE DATE |                                 |          |  |
| 12. ON-SITE SOILS CONSISTS OF #6 MANATEE LOAMY FINE SAND & #11 IMMOKALEE FINE SAND - HYDRAULIC GROUP B/D         |                                 |          |  |
| 13. DRAINAGE DESIGN TO MEET CITY OF OKCHOBEE AND SFWMD REQUIREMENTS.   |                                 |          |  |
| 14. FREESTANDING MONUMENT SIGN ALLOWED PER 90-571 OF COUNTY CODE, 64 SF FACE, 8 FT HEIGHT                        |                                 |          |  |

## REQUIRED BUILDING SETBACKS

FRONT SETBACK BLDG 20 FT  
FRONT SETBACK OTHER 10 FT  
SIDE SETBACK 8 FT  
REAR SETBACK 10 FT.

## ADJACENT ZONING / LAND USE

NORTH	UNDEVELOPED HOLDING	/	SINGLE FAMILY
SOUTH	COMMERCIAL HEAVY CHV	/	COM COMMERCIAL
EAST	COUNTY COMMERCIAL	/	COM COMMERCIAL
WEST	COMMERCIAL HEAVY CHV	/	COM COMMERCIAL



NOTE: ALL INFORMATION CONTAINED HEREIN IS PROPERTY OF AMERICAN CIVIL ENGINEERING CO., ALL RIGHTS RESERVED. COPY RIGHT 2022.	ENGINEER: JOHN J. HERBERT IV, P.E. CHECKED BY: TOM SKELTON, P.E. TECHNICIAN: J.W.H.	PROJECT NO. 22190
REVISIONS	DATE	

**AMERICAN CIVIL  
ENGINEERING CO.**  
207 N. MOSS RD., SUITE 211; WINTER SPRINGS, FLA 32708  
(407) 327-7700

rt. of authorization  
number 8729

**SITE PLAN**

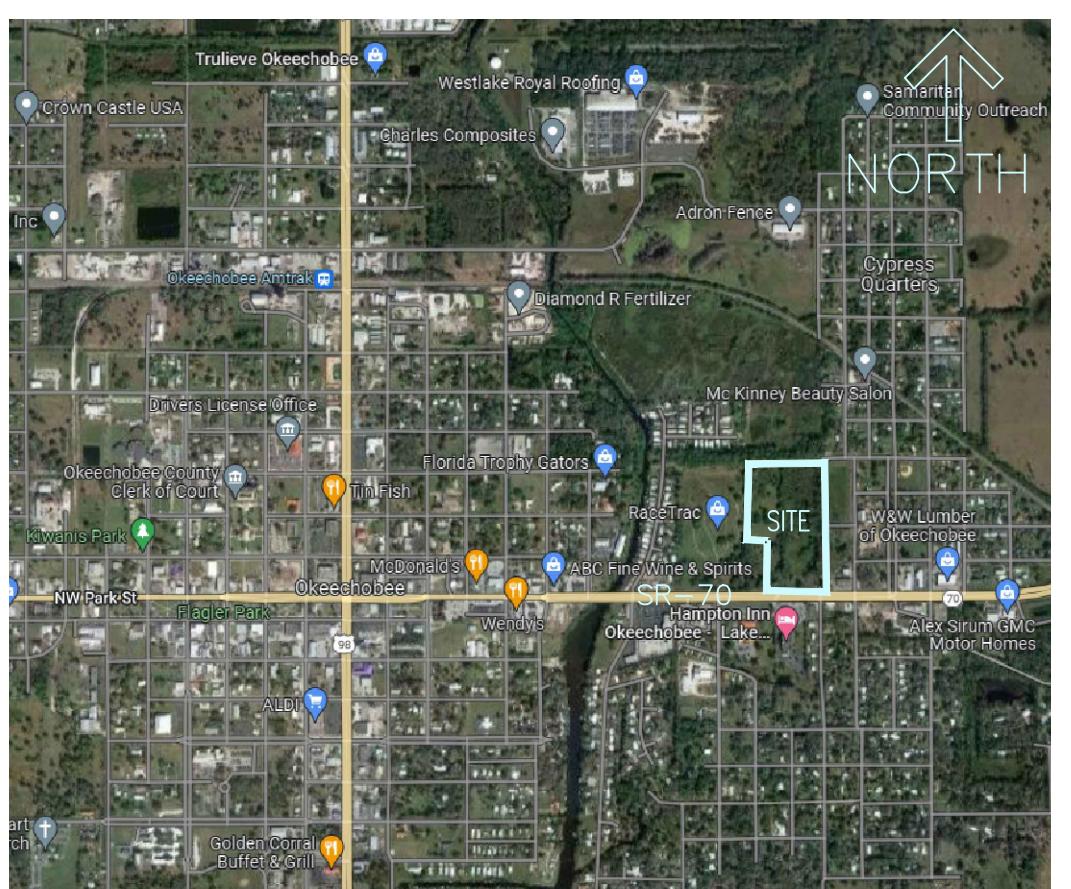
**PARK STREET**

**COMMERCE CENTER**

STATE ROAD 70 E, OKEECHOHOBEE, FLORIDA 34

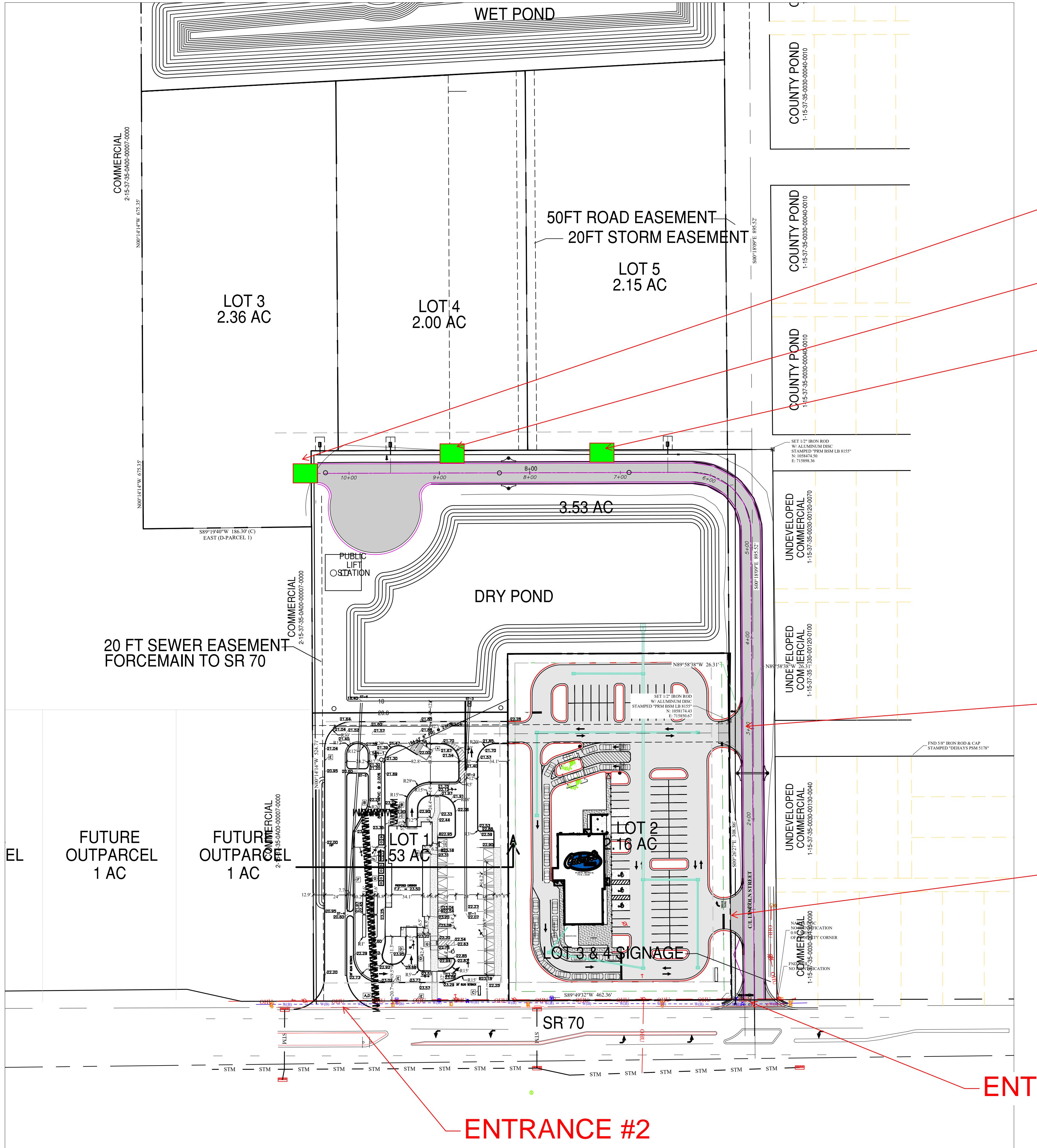
**SITE PL  
PARK ST  
COMMERCE  
STATE ROAD 70 E, OKEE**

100



# VICINITY MAP N.T.S.

C4.0



**DRIVEWAY #5**

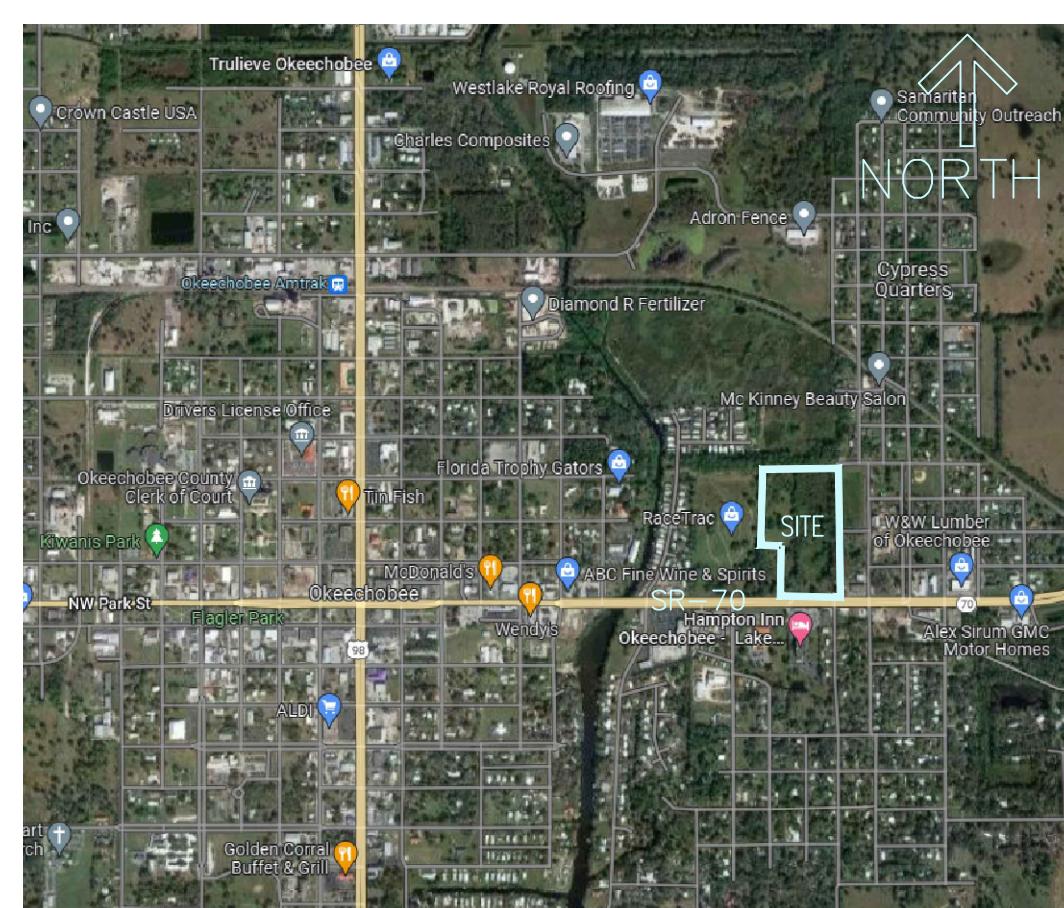
# — DRIVEWAY #4

# — DRIVEWAY #3

# - DRIVEWAY #2

# – DRIVEWAY #1

# – ENTRANCE #1



# TRAFFIC EXHIBIT PARK STREET COMMERCE CENTER

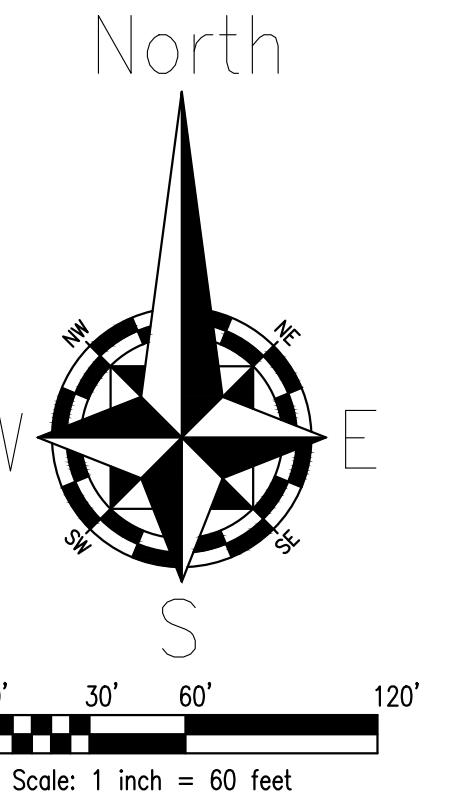
---

AMERICAN CIVIL  
ENGINEERING CO.

---

A red diamond shape with a black outline, positioned at the top right of the slide.

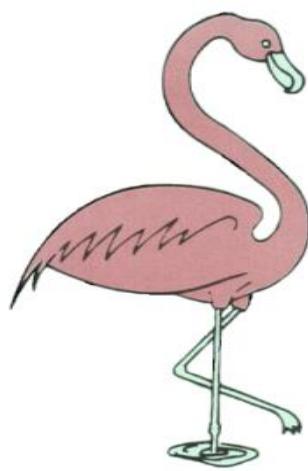
# TRAFFIC EXHIBIT PARK STREET COMMERCE CENTER



NOTE: ALL INFORMATION CONTAINED HEREIN IS PROPERTY OF AMERICAN CIVIL ENGINEERING CO., ALL RIGHTS RESERVED. COPY RIGHT 2022.	
ENGINEER:	JOHN J. HERBERT IV, P.E.
CHECKED BY:	TOM SKELTON, P.E.
TECHNICIAN:	J.W.H.

## **APPENDIX B**

## **TURNING MOVEMENT COUNTS**



**TABLE B-1**  
**NE Park Street (SR 70) / Hampton inn Driveway**  
**TURNING MOVEMENT COUNTS - ALL VEHICLES**

Monday, February 13, 2023

	NE PARK STREET			HAMPTON INN DRIVEWAY		All
	EB Right Turn	WB U-Turn	WB Left Turn	NB Left Turn	NB Right Turn	
6:45-7:00 AM	0	0	1	0	0	1
7:00-7:15 AM	0	0	0	0	0	0
<b>7:15-7:30 AM</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>1</b>	<b>4</b>
<b>7:30-7:45 AM</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>4</b>	<b>5</b>
<b>7:45-8:00 AM</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>2</b>	<b>4</b>	<b>8</b>
<b>8:00-8:15 AM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>4</b>	<b>4</b>
8:15-8:30 AM	3	0	0	0	3	6
8:30-8:45 AM	2	0	0	0	2	4
<b>AM PEAK PERIOD:</b>	<b>7</b>	<b>0</b>	<b>5</b>	<b>2</b>	<b>18</b>	<b>32</b>

<b>AM PEAK HOUR:</b>	2	0	4	2	13	21
<b>7:15-8:15 AM</b>						

Monday, February 13, 2023

	NE PARK STREET			HAMPTON INN DRIVEWAY		All
	EB Right Turn	WB U-Turn	WB Left Turn	NB Left Turn	NB Right Turn	
3:45-4:00 PM	4	0	3	0	5	12
4:00-4:15 PM	4	1	7	0	5	17
4:15-4:30 PM	1	0	3	0	4	8
4:30-4:45 PM	3	0	5	0	9	17
4:45-5:00 PM	13	0	6	0	12	31
<b>5:00-5:15 PM</b>	<b>8</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>5</b>	<b>15</b>
<b>5:15-5:30 PM</b>	<b>7</b>	<b>0</b>	<b>5</b>	<b>0</b>	<b>4</b>	<b>16</b>
<b>5:30-5:45 PM</b>	<b>14</b>	<b>0</b>	<b>5</b>	<b>0</b>	<b>4</b>	<b>23</b>
<b>5:45-6:00 PM</b>	<b>9</b>	<b>0</b>	<b>8</b>	<b>0</b>	<b>5</b>	<b>22</b>
<b>PM PEAK PERIOD:</b>	<b>63</b>	<b>1</b>	<b>44</b>	<b>0</b>	<b>53</b>	<b>161</b>

<b>PM PEAK HOUR:</b>	38	0	20	0	18	76
<b>5:00-6:00 PM</b>						

**BUCKHOLZ TRAFFIC**

DAY: MONDAY  
 DATE: 02/13/23  
 WEATHER: CLEAR & DRY  
 BEGIN TIME (MILITARY): 06:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC

MANUAL TURNING MOVEMENT COUNTS  
 NE PARK STREET @ SE 13TH AVENUE  
 OKEECHOBEE COUNTY, FLORIDA

Site Code : 44444444  
 Start Date: 02/13/23  
 File I.D. : 021323AM  
 Page : 1

AUTOMOBILES, COMMERCIAL VEHICLES

From North				NE PARK STREET				SE 13TH AVENUE				NE PARK STREET							
				From East				From South				From West							
Left	Thru	Right	Other	Left	Thru	Right	U-TURN	Left	Thru	Right	Other	Left	Thru	Right	U-TURN	Total			
Date 02/13/23																			
06:45	0	0	0	0	1	220	0	0	0	0	2	0	0	218	1	1	443		
07:00	0	0	0	0	0	169	1	1	0	0	3	0	0	249	1	1	425		
07:15	0	0	0	0	4	220	0	0	0	0	2	0	0	209	1	1	437		
07:30	0	0	0	0	1	314	0	0	0	0	2	0	0	217	1	2	537		
Hr Total	0	0	0	0	6	923	1	1	0	0	9	0	0	893	4	5	1842		
07:45	0	0	0	0	2	332	0	0	1	0	1	0	0	244	1	1	582		
08:00	0	0	0	0	0	294	0	0	1	0	2	0	0	246	1	0	544		
08:15	0	0	0	0	2	211	0	0	1	0	0	0	0	184	0	1	399		
08:30	0	0	0	0	0	237	0	0	1	0	0	0	0	239	0	0	477		
Hr Total	0	0	0	0	4	1074	0	0	4	0	3	0	0	913	2	2	2002		
*TOTAL*	0	0	0	0	10	1997	1	1	4	0	12	0	0	1806	6	7	3844		

Peak Hour Analysis By Entire Intersection for the Period: 07:15 to 08:15 on 02/13/23

Peak start 07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15	07:15
Volume	0	0	0	0	7	1160	0	0	2	0	7	0	0	916	4	4
Percent	0%	0%	0%	0%	1%	99%	0%	0%	22%	0%	78%	0%	0%	99%	0%	0%
Pk total	0				1167				9				924			
Highest	06:45				07:45				08:00				08:00			
Volume	0	0	0	0	2	332	0	0	1	0	2	0	0	246	1	0
Hi total	0				334				3				247			
PHF	.0				.87				.75				.94			

## JW BUCKHOLZ TRAFFIC ENGINEERING INC

DAY: MONDAY

DATE: 02/13/23

WEATHER: CLEAR &amp; DRY

BEGIN TIME (MILITARY): 06:45 Hrs

MANUAL TURNING MOVEMENT COUNTS

NE PARK STREET @ SE 13TH AVENUE

OKEECHOBEE COUNTY, FLORIDA

Site Code : 44444444

Start Date: 02/13/23

File I.D. : 021323AM

Page : 1

## AUTOMOBILES

From North				NE PARK STREET				SE 13TH AVENUE				NE PARK STREET							
				From East				From South				From West							
Left	Thru	Right	Other	Left	Thru	Right	U-TURN	Left	Thru	Right	Other	Left	Thru	Right	U-TURN	Total			
Date 02/13/23 -																			
06:45	.0	0	0	0	1	186	0	0	0	0	1	0	0	174	1	1	364		
07:00	0	0	0	0	0	143	1	1	0	0	2	0	0	208	1	1	357		
07:15	0	0	0	0	3	195	0	0	0	0	1	0	0	176	0	1	376		
07:30	0	0	0	0	1	266	0	0	0	0	2	0	0	177	1	2	449		
Hr Total	0	0	0	0	5	790	1	1	0	0	6	0	0	735	3	5	1546		
07:45	0	0	0	0	2	288	0	0	1	0	0	0	0	201	1	1	494		
08:00	0	0	0	0	0	237	0	0	1	0	2	0	0	197	1	0	438		
08:15	0	0	0	0	1	177	0	0	1	0	0	0	0	143	0	1	323		
08:30	0	0	0	0	0	179	0	0	1	0	0	0	0	195	0	0	375		
Hr Total	0	0	0	0	3	881	0	0	4	0	2	0	0	736	2	2	1630		
*TOTAL*	0	0	0	0	8	1671	1	1	4	0	8	0	0	1471	5	7	3176		

## Peak Hour Analysis By Entire Intersection for the Period: 07:15 to 08:15 on 02/13/23

Peak start 07:15				07:15				07:15				07:15							
Volume	0	0	0	0	6	986	0	0	2	0	5	0	0	751	3	4			
Percent	0%	0%	0%	0%	1%	99%	0%	0%	29%	0%	71%	0%	0%	99%	0%	1%			
Pk total	0				992				7				758						
Highest	06:45				07:45				08:00				07:45						
Volume	0	0	0	0	2	288	0	0	1	0	2	0	0	201	1	1			
Hi total	0				290				3				203						
PHF	.0				.86				.58				.93						

## JW BUCKHOLZ TRAFFIC ENGINEERING INC

DAY: MONDAY

DATE: 02/13/23

WEATHER: CLEAR &amp; DRY

BEGIN TIME (MILITARY) :06:45 Hrs

MANUAL TURNING MOVEMENT COUNTS

NE PARK STREET @ SE 13TH AVENUE

OKEECHOBEE COUNTY, FLORIDA

Site Code : 44444444

Start Date: 02/13/23

File I.D. : 021323AM

Page : 1

## COMMERCIAL VEHICLES

From North				NE PARK STREET				SE 13TH AVENUE				NE PARK STREET								
				From East				From South				From West								
Left	Thru	Right	Other	Left	Thru	Right	Other	Left	Thru	Right	Other	Left	Thru	Right	Other	Left	Thru	Right	Other	Total
Date 02/13/23																				
06:45	0	0	0	0	0	34	0	0	0	0	1	0	0	44	0	0	0	0	79	
07:00	0	0	0	0	0	26	0	0	0	0	1	0	0	41	0	0	0	0	68	
07:15	0	0	0	0	1	25	0	0	0	0	1	0	0	33	1	0	0	0	62	
07:30	0	0	0	0	0	48	0	0	0	0	0	0	0	40	0	0	0	0	88	
Hr Total	0	0	0	0	1	133	0	0	0	0	3	0	1	158	1	0	0	0	297	
07:45	0	0	0	0	0	44	0	0	0	0	1	0	0	43	0	0	0	0	88	
08:00	0	0	0	0	0	57	0	0	0	0	0	0	0	49	0	0	0	0	106	
08:15	0	0	0	0	1	34	0	0	0	0	0	0	0	41	0	0	0	0	76	
08:30	0	0	0	0	0	58	0	0	0	0	0	0	0	44	0	0	0	0	102	
Hr Total	0	0	0	0	1	193	0	0	0	0	1	0	0	177	0	0	0	0	372	
*TOTAL*	0	0	0	0	2	326	0	0	0	0	4	0	1	335	1	0	0	0	669	

## Peak Hour Analysis By Entire Intersection for the Period: 07:15 to 08:15 on 02/13/23

Peak start 07:15	07:15	07:15	07:15	07:15
Volume	0 0 0 0   1 174	0 0   0 0   0 2	0   1 165	1 0
Percent	0% 0% 0% 0%   1% 99%	0% 0%   0% 0%   100%	0%   1% 99%	1% 0%
Pk total	0   175	2   167		
Highest	06:45   08:00	07:15   08:00		
Volume	0 0 0 0   0 57	0 0   0 1	0   0 49	0 0
Hi total	0   57	1   49		
PHF	.0   .77	.50   .85		

## JW BUCKHOLZ TRAFFIC ENGINEERING INC

DAY: MONDAY

DATE: 02/13/23

WEATHER: CLEAR &amp; DRY

BEGIN TIME (MILITARY): 06:45 Hrs

MANUAL TURNING MOVEMENT COUNTS

NE PARK STREET @ SE 13TH AVENUE

OKEECHOBEE COUNTY, FLORIDA

Site Code : 44444444

Start Date: 02/13/23

File I.D. : 021323AM

Page : 1

## PEDESTRIAN &amp; BICYCLE

From North				NE PARK STREET				SE 13TH AVENUE				NE PARK STREET								
				From East				From South				From West								
Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	Total
Date 02/13/23																				
06:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Hr Total	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:45	0	0	0	2	0	0	0	0	0	0	1	0	0	0	1	0	0	0	1	4
08:00	0	0	0	2	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	3
08:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Hr Total	0	0	0	5	0	0	0	0	0	0	2	0	0	0	1	0	0	0	1	8
*TOTAL*	0	0	0	6	0	0	0	0	0	0	2	0	0	0	1	0	0	0	1	9

## Peak Hour Analysis By Entire Intersection for the Period: 07:15 to 08:15 on 02/13/23

Peak start 07:15				07:15				07:15				07:15							
Volume	0	0	0	5	0	0	0	0	0	0	0	2	0	0	0	1			
Percent	0%	0%	0%	100%	0%	0%	0%	0%	0%	0%	0%	100%	0%	0%	0%	100%			
Pk total	5				0				2			1							
Highest	07:45				06:45				07:45			07:45							
Volume	0	0	0	2	0	0	0	0	0	0	0	1	0	0	0	1			
Hi total	2				0				1			1							
PHF	.62				.0				.50			.25							

## JW BUCKHOLZ TRAFFIC ENGINEERING INC

DAY: MONDAY

DATE: 02/13/23

WEATHER: CLEAR &amp; DRY

BEGIN TIME (MILITARY): 15:45 Hrs

MANUAL TURNING MOVEMENT COUNTS

NE PARK STREET @ SE 13TH AVENUE

OKEECHOBEE COUNTY, FLORIDA

Site Code : 02132023

Start Date: 02/13/23

File I.D. : 021323PM

Page : 1

## AUTOMOBILES, COMMERCIAL VEHICLES

From North				NE PARK STREET				SE 13TH AVENUE				NE PARK STREET				From West				
Left	Thru	Right	Other	Left	Thru	Right	U-TURN	Left	Thru	Right	Other	Left	Thru	Right	U-TURN	Left	Thru	Right	U-TURN	Total
<u>Date 02/13/23</u>																				
15:45	0	0	0	0	1	269	0	1	1	0	3	0	0	291	3	1	1	570		
16:00	0	0	0	0	4	257	0	0	2	0	5	0	0	324	9	1	1	602		
16:15	0	0	0	0	2	295	0	2	1	0	1	0	0	313	4	0	0	618		
16:30	0	0	0	0	6	300	2	1	4	0	4	0	0	255	4	3	3	579		
Hr Total	0	0	0	0	13	1121	2	4	8	0	13	0	0	1183	20	5	5	2369		
16:45	0	0	0	0	6	289	0	1	2	0	2	0	0	265	0	2	2	567		
17:00	0	0	0	0	12	306	0	0	0	0	2	0	0	375	1	2	2	698		
17:15	0	0	0	0	10	289	0	0	0	0	4	0	0	332	2	1	1	638		
17:30	0	0	0	0	4	277	0	0	1	0	4	0	0	301	5	2	2	594		
Hr Total	0	0	0	0	32	1161	0	1	3	0	12	0	0	1273	8	7	7	2497		
<u>17:45</u>	0	0	0	0	6	311	0	2	1	0	7	0	0	273	7	0	0	607		
Hr Total	0	0	0	0	6	311	0	2	1	0	7	0	0	273	7	0	0	607		
<b>*TOTAL*</b>	0	0	0	0	51	2593	2	7	12	0	32	0	0	2729	35	12	12	5473		

## Peak Hour Analysis By Entire Intersection for the Period: 17:00 to 18:00 on 02/13/23

Peak start 17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00
Volume	0	0	0	0	32	1183	0	2	2	0	17	0	0	1281	15	5	5	
Percent	0%	0%	0%	0%	3%	97%	0%	0%	11%	0%	89%	0%	0%	98%	1%	0%	0%	
Pk total	0		1217						19				1301					
Highest	15:45		17:45			17:45			17:45				17:00					
Volume	0	0	0	0	6	311	0	2	1	0	7	0	0	375	1	2	2	
Hi total	0		319						8				378					
PHF	.0		.95						.59				.86					

## JW BUCKHOLZ TRAFFIC ENGINEERING INC

DAY: MONDAY

DATE: 02/13/23

WEATHER: CLEAR &amp; DRY

BEGIN TIME (MILITARY): 15:45 Hrs

MANUAL TURNING MOVEMENT COUNTS

NE PARK STREET @ SE 13TH AVENUE

OKEECHOBEE COUNTY, FLORIDA

Site Code : 02132023

Start Date: 02/13/23

File I.D. : 021323PM

Page : 1

## AUTOMOBILES

From North				NE PARK STREET				SE 13TH AVENUE				NE PARK STREET									
				From East				From South				From West									
Left	Thru	Right	Other	Left	Thru	Right	U-TURN	Left	Thru	Right	Other	Left	Thru	Right	U-TURN	Left	Thru	Right	U-TURN	Total	
Date 02/13/23																					
15:45	0	0	0	0		1	236	0	1		1	0	3	0		0	261	3	1		507
16:00	0	0	0	0		4	214	0	0		2	0	5	0		0	284	9	1		519
16:15	0	0	0	0		2	252	0	2		1	0	1	0		0	267	4	0		529
16:30	0	0	0	0		6	258	2	1		4	0	4	0		0	222	4	3		504
Hr Total	0	0	0	0		13	960	2	4		8	0	13	0		0	1034	20	5		2059
16:45	0	0	0	0		6	257	0	1		2	0	2	0		0	221	0	2		491
17:00	0	0	0	0		11	279	0	0		0	0	2	0		0	337	1	2		632
17:15	0	0	0	0		10	259	0	0		0	0	4	0		0	293	2	1		569
17:30	0	0	0	0		4	254	0	0		1	0	4	0		0	276	5	2		546
Hr Total	0	0	0	0		31	1049	0	1		3	0	12	0		0	1127	8	7		2238
<u>17:45</u>	0	0	0	0		6	282	0	2		1	0	7	0		0	247	7	0		552
Hr Total	0	0	0	0		6	282	0	2		1	0	7	0		0	247	7	0		552
<b>*TOTAL*</b>	0	0	0	0		50	2291	2	7		12	0	32	0		0	2408	35	12		4849

## Peak Hour Analysis By Entire Intersection for the Period: 17:00 to 18:00 on 02/13/23

Peak start 17:00		17:00		17:00		17:00		17:00		17:00										
Volume	0	0	0	0		31	1074	0	2		2	0	17	0		0	1153	15	5	
Percent	0%	0%	0%	0%		3%	97%	0%	0%		11%	0%	89%	0%		0%	98%	1%	0%	
Pk total	0					1107					19					1173				
Highest	15:45					17:00					17:45					17:00				
Volume	0	0	0	0		11	279	0	0		1	0	7	0		0	337	1	2	
Hi total	0					290					8					340				
PHF	.0					.95					.59					.86				

## JW BUCKHOLZ TRAFFIC ENGINEERING INC

DAY: MONDAY

DATE: 02/13/23

WEATHER: CLEAR &amp; DRY

BEGIN TIME (MILITARY): 15:45 Hrs

MANUAL TURNING MOVEMENT COUNTS

NE PARK STREET @ SE 13TH AVENUE

OKEECHOBEE COUNTY, FLORIDA

Site Code : 02132023

Start Date: 02/13/23

File I.D. : 021323PM

Page : 1

## COMMERCIAL VEHICLES

From North	NE PARK STREET				SE 13TH AVENUE				NE PARK STREET					
	From East				From South				From West					
	Left	Thru	Right	Other	Left	Thru	Right	Other	Left	Thru	Right	Other		
Date 02/13/23														
15:45	0	0	0	0	0	33	0	0	0	0	0	0	63	
16:00	0	0	0	0	0	43	0	0	0	0	0	40	83	
16:15	0	0	0	0	0	43	0	0	0	0	0	46	89	
16:30	0	0	0	0	0	42	0	0	0	0	0	33	75	
Hr Total	0	0	0	0	0	161	0	0	0	0	0	149	310	
16:45	0	0	0	0	0	32	0	0	0	0	0	44	76	
17:00	0	0	0	0	1	27	0	0	0	0	0	38	66	
17:15	0	0	0	0	0	30	0	0	0	0	0	39	69	
17:30	0	0	0	0	0	23	0	0	0	0	0	25	48	
Hr Total	0	0	0	0	1	112	0	0	0	0	0	146	259	
17:45	0	0	0	0	0	29	0	0	0	0	0	26	55	
Hr Total	0	0	0	0	0	29	0	0	0	0	0	26	55	
*TOTAL*	0	0	0	0	1	302	0	0	0	0	0	321	624	

## Peak Hour Analysis By Entire Intersection for the Period: 17:00 to 18:00 on 02/13/23

Peak start 17:00		17:00		17:00		17:00		17:00		17:00			
Volume	0	0	0	0	1	109	0	0	0	0	0	128	0
Percent	0%	0%	0%	0%	1%	99%	0%	0%	0%	0%	0%	100%	0%
Pk total	0				110			0			128		
Highest	15:45				17:15			15:45			17:15		
Volume	0	0	0	0	0	30	0	0	0	0	0	39	0
Hi total	0				30			0			39		
PHF	.0				.92			.0			.82		

## JW BUCKHOLZ TRAFFIC ENGINEERING INC

DAY: MONDAY

DATE: 02/13/23

WEATHER: CLEAR &amp; DRY

BEGIN TIME (MILITARY): 15:45 Hrs

MANUAL TURNING MOVEMENT COUNTS

NE PARK STREET @ SE 13TH AVENUE

OKEECHOBEE COUNTY, FLORIDA

Site Code : 02132023

Start Date: 02/13/23

File I.D. : 021323PM

Page : 1

## PEDESTRIAN &amp; BICYCLE

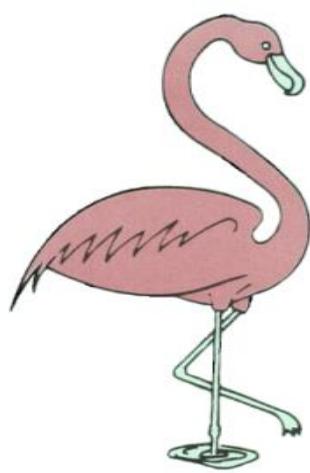
From North	NE PARK STREET					SE 13TH AVENUE					NE PARK STREET										
	From East					From South					From West										
	Left	Thru	Right	PEDS		Left	Thru	Right	PEDS		Left	Thru	Right	PEDS		Left	Thru	Right	PEDS		Total
<b>Date 02/13/23</b>																					
15:45	0	0	0	0		0	0	0	0		0	0	0	0		0	0	0	0		0
16:00	0	0	0	0		0	0	0	1		0	0	0	1		0	0	0	0		2
16:15	0	0	0	0		0	0	0	0		0	0	0	0		0	0	0	0		0
16:30	0	0	0	0		0	0	0	0		0	0	0	0		0	0	0	0		0
Hr Total	0	0	0	0		0	0	0	1		0	0	0	1		0	0	0	0		2
16:45	0	0	0	0		0	0	0	0		0	0	0	0		0	0	0	0		0
17:00	0	0	0	0		0	0	0	0		0	0	0	0		0	0	0	0		0
17:15	0	0	0	2		0	0	0	0		0	0	0	0		0	0	0	0		2
17:30	0	0	0	1		0	0	0	0		0	0	0	0		0	0	0	0		1
Hr Total	0	0	0	3		0	0	0	0		0	0	0	0		0	0	0	0		3
17:45	0	0	0	3		0	0	0	0		0	0	0	0		0	0	0	0		3
Hr Total	0	0	0	3		0	0	0	0		0	0	0	0		0	0	0	0		3
<b>*TOTAL*</b>	0	0	0	6		0	0	0	1		0	0	0	1		0	0	0	0		8

## Peak Hour Analysis By Entire Intersection for the Period: 17:00 to 18:00 on 02/13/23

Peak start 17:00		17:00		17:00		17:00		17:00		17:00		17:00		17:00		17:00		17:00		17:00	
Volume	0	0	0	6		0	0	0	0		0	0	0	0		0	0	0	0		0
Percent	0%	0%	0%	100%		0%	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%
Pk total	6					0					0					0					
Highest	17:45					15:45					15:45					15:45					
Volume	0	0	0	3		0	0	0	0		0	0	0	0		0	0	0	0		0
Hi total	3					0					0					0					
PHF	.50					.0					.0					.0					

## **APPENDIX C**

### **FDOT TRAFFIC DATA**

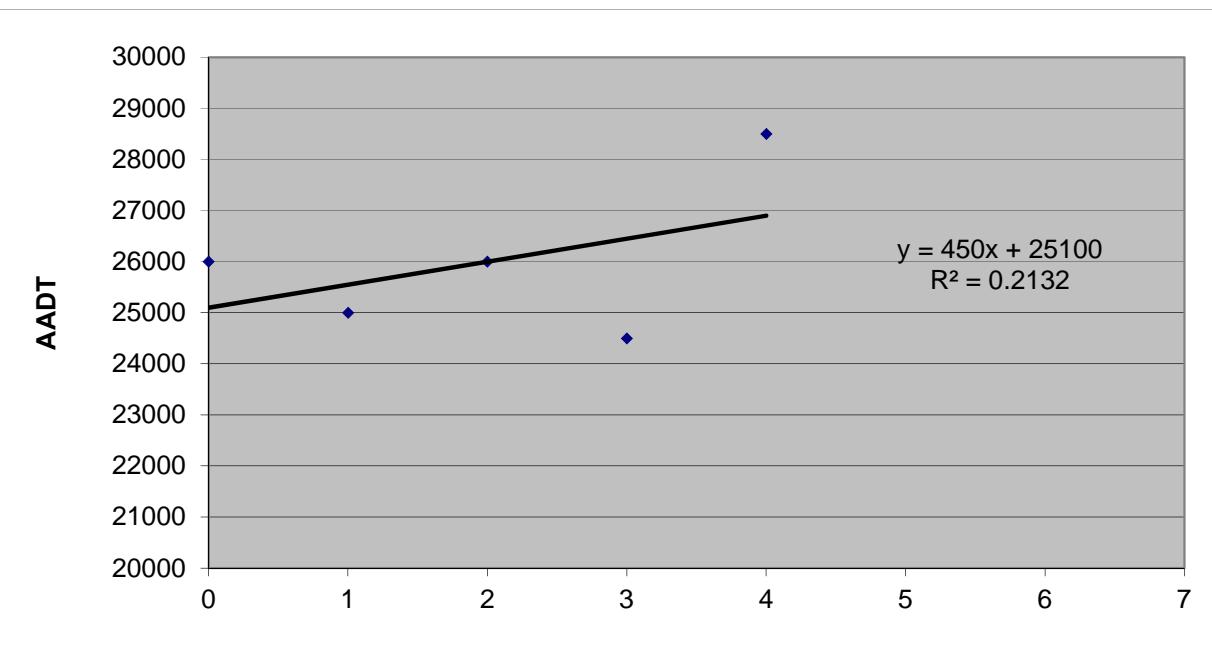


**TABLE C-1**  
**LINEAR REGRESSION ANALYSIS**

**SR 70, West of SR 710**

<u>Year</u>	<u>X</u>	Actual AADT (Y)	Predicted AADT
2017	0	26000	<b>25100</b>
2018	1	25000	25550
2019	2	26000	26000
2020	3	24500	26450
2021	4	28500	26900
2022	5		27350
2023	6		27800
2024	7		<b>28250</b>

i = 1.7%



**BUCKHOLZ TRAFFIC**

FLORIDA DEPARTMENT OF TRANSPORTATION  
 TRANSPORTATION STATISTICS OFFICE  
 2021 HISTORICAL AADT REPORT

COUNTY: 91 - OKEECHOBEE

SITE: 0007 - SR 70, WEST OF SR 710/EAST OF OKEECHOBEE

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	28500 C	E 14000	W 14500	9.00	58.00	18.20
2020	24500 C	E 12000	W 12500	9.00	57.20	16.50
2019	26000 C	E 13000	W 13000	9.00	57.30	14.80
2018	25000 C	E 12500	W 12500	9.00	57.90	17.90
2017	26000 C	E 13000	W 13000	9.00	58.80	15.30
2016	25500 C	E 12500	W 13000	9.00	57.40	13.00
2015	20000 C	E 10000	W 10000	9.00	56.60	13.60
2014	21000 S	E 10500	W 10500	9.00	58.10	14.40
2013	21000 F	E 10500	W 10500	9.00	58.10	14.40
2012	21000 C	E 10500	W 10500	9.00	57.50	14.40
2011	22500 F	E 11000	W 11500	9.00	56.90	11.60
2010	22500 C	E 11000	W 11500	10.99	56.24	11.60
2009	22000 C	E 11000	W 11000	10.97	57.93	13.60
2008	23000 C	E 11500	W 11500	11.05	57.88	16.50
2007	23500 C	E 11500	W 12000	10.65	60.38	15.70
2006	23500 C	E 11500	W 12000	10.64	58.36	15.50

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE  
 S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE  
 V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

\*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

COUNTY: 91  
 STATION: 0007  
 DESCRIPTION: SR 70, WEST OF SR 710/EAST OF OKEECHOBEE  
 START DATE: 08/11/2021  
 START TIME: 1730

TIME	DIRECTION: E					DIRECTION: W					COMBINED	
	1ST	2ND	3RD	4TH	TOTAL	1ST	2ND	3RD	4TH	TOTAL	TOTAL	
0000	22	22	14	15	73	30	23	12	14	79	152	
0100	11	15	8	11	45	8	12	27	14	61	106	
0200	9	8	6	8	31	17	17	15	21	70	101	
0300	11	12	14	17	54	19	11	18	23	71	125	
0400	36	25	46	62	169	15	24	36	50	125	294	
0500	66	106	137	162	471	56	79	77	106	318	789	
0600	175	188	175	204	742	123	130	203	233	689	1431	
0700	209	193	193	182	777	167	239	268	324	998	1775	
0800	213	180	186	166	745	221	190	207	212	830	1575	
0900	161	161	148	178	648	179	172	157	196	704	1352	
1000	172	170	170	161	673	194	183	191	181	749	1422	
1100	183	159	191	179	712	206	198	227	188	819	1531	
1200	211	204	179	200	794	184	171	195	212	762	1556	
1300	197	179	174	201	751	181	187	192	192	752	1503	
1400	195	209	201	222	827	184	209	227	240	860	1687	
1500	254	252	202	223	931	213	239	258	212	922	1853	
1600	314	255	248	243	1060	212	262	274	274	1022	2082	
1700	312	300	282	223	1117	279	272	224	236	1011	2128	
1800	223	188	166	129	706	252	227	175	181	835	1541	
1900	154	122	125	113	514	138	149	133	115	535	1049	
2000	114	145	116	115	490	132	107	81	78	398	888	
2100	106	65	60	53	284	79	70	55	63	267	551	
2200	60	43	48	51	202	50	39	35	49	173	375	
2300	50	36	22	20	128	32	24	25	17	98	226	

24-HOUR TOTALS: 12944 13148 26092

#### PEAK VOLUME INFORMATION

DIRECTION: E		DIRECTION: W		COMBINED DIRECTIONS		
HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME	
A.M.	645	799	715	1052	715	1833
P.M.	1645	1137	1630	1099	1630	2202
DAILY	1645	1137	1630	1099	1630	2202

TRUCK PERCENTAGE 18.25 18.22 18.23

#### CLASSIFICATION SUMMARY DATABASE

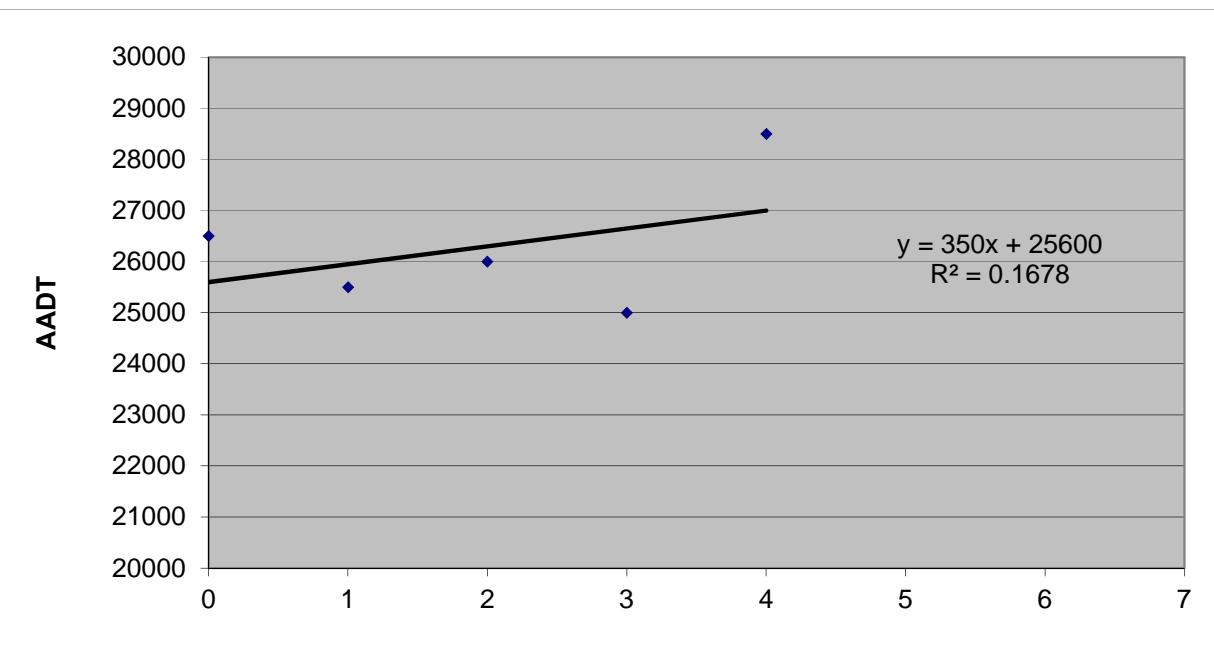
DIR	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	TOTTRK	TOTVOL
E	174	6138	4243	64	1045	259	41	257	673	21	0	2	0	0	27	2362	12944
W	148	6049	4048	55	897	383	24	279	730	23	2	1	1	0	508	2395	13148

**TABLE C-2**  
**LINEAR REGRESSION ANALYSIS**

**SR 70, East of US 441**

<u>Year</u>	<u>X</u>	Actual AADT (Y)	Predicted AADT
2017	0	26500	<b>25600</b>
2018	1	25500	25950
2019	2	26000	26300
2020	3	25000	26650
2021	4	28500	27000
2022	5		27350
2023	6		27700
2024	7		<b>28050</b>

i = 1.3%



**BUCKHOLZ TRAFFIC**

FLORIDA DEPARTMENT OF TRANSPORTATION  
 TRANSPORTATION STATISTICS OFFICE  
 2021 HISTORICAL AADT REPORT

COUNTY: 91 - OKEECHOBEE

SITE: 5012 - SR 70, EAST OF SR 15/700/US 98/441

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	28500 C	E 14000	W 14500	9.00	58.00	15.60
2020	25000 C	E 12500	W 12500	9.00	57.20	18.00
2019	26000 C	E 13000	W 13000	9.00	57.30	16.40
2018	25500 C	E 13000	W 12500	9.00	57.90	17.60
2017	26500 C	E 13500	W 13000	9.00	58.80	14.00
2016	25500 C	E 13000	W 12500	9.00	57.40	13.50
2015	27000 C	E 13500	W 13500	9.00	56.60	13.00
2014	28000 C	E 14000	W 14000	9.00	58.10	13.20
2013	26000 F	E 13000	W 13000	9.00	58.10	11.30
2012	26000 C	E 13000	W 13000	9.00	57.50	11.30
2011	29500 F	E 14500	W 15000	9.00	56.90	8.20
2010	29500 C	E 14500	W 15000	10.99	56.24	8.20
2009	31500 C	E 16000	W 15500	10.97	57.93	8.70
2008	29500 C	E 15000	W 14500	11.05	57.88	8.00
2007	31500 C	E 16000	W 15500	10.65	60.38	8.40
2006	35000 C	E 18000	W 17000	10.64	58.36	7.50

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE  
 S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE  
 V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

\*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

COUNTY: 91  
 STATION: 5012  
 DESCRIPTION: SR 70, EAST OF SR 15/700/US 98/441  
 START DATE: 08/11/2021  
 START TIME: 1645

TIME	DIRECTION: E					DIRECTION: W					COMBINED	
	1ST	2ND	3RD	4TH	TOTAL	1ST	2ND	3RD	4TH	TOTAL	TOTAL	
0000	24	20	14	17	75	30	23	12	16	81	156	
0100	12	13	6	10	41	7	11	26	18	62	103	
0200	10	6	9	6	31	16	17	14	19	66	97	
0300	11	13	14	19	57	21	12	17	23	73	130	
0400	32	27	47	65	171	14	27	30	49	120	291	
0500	69	111	141	159	480	58	76	73	102	309	789	
0600	170	194	187	203	754	121	136	204	232	693	1447	
0700	209	195	193	185	782	166	227	268	331	992	1774	
0800	215	192	177	175	759	219	199	201	218	837	1596	
0900	153	165	154	176	648	170	176	157	200	703	1351	
1000	180	174	176	165	695	191	190	187	189	757	1452	
1100	182	165	192	182	721	202	204	220	186	812	1533	
1200	218	212	179	201	810	195	163	208	216	782	1592	
1300	205	182	177	201	765	184	171	205	196	756	1521	
1400	196	216	202	231	845	175	201	231	234	841	1686	
1500	268	252	210	226	956	225	239	260	221	945	1901	
1600	303	267	239	226	1035	217	259	261	264	1001	2036	
1700	300	274	278	222	1074	256	274	221	242	993	2067	
1800	224	187	163	137	711	250	235	171	187	843	1554	
1900	150	124	131	109	514	149	142	142	117	550	1064	
2000	118	144	120	108	490	124	103	84	79	390	880	
2100	108	64	61	54	287	79	65	59	67	270	557	
2200	54	48	46	57	205	43	42	31	48	164	369	
2300	46	35	20	22	123	39	27	26	15	107	230	

24-HOUR TOTALS: 13029 13147 26176

PEAK VOLUME INFORMATION											
DIRECTION: E				DIRECTION: W				COMBINED DIRECTIONS			
HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME
A.M.	645	800	715	1045	715	1833					
P.M.	1645	1078	1630	1055	1630	2094					
DAILY	1645	1078	1630	1055	1630	2094					

TRUCK PERCENTAGE 14.63 16.56 15.60

#### CLASSIFICATION SUMMARY DATABASE

DIR	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	TOTTRK	TOTVOL
E	138	7300	3611	50	665	198	40	200	723	27	0	2	1	0	74	1906	13029
W	141	6641	4132	45	874	247	20	220	743	24	2	1	1	0	56	2177	13147

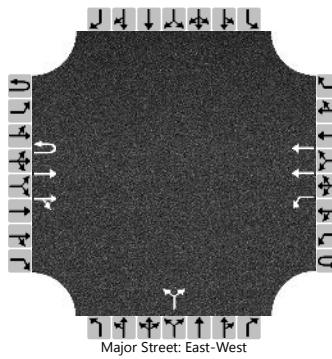
## **APPENDIX D**

### **CAPACITY CALCULATIONS UNSIGNALIZED INTERSECTIONS**

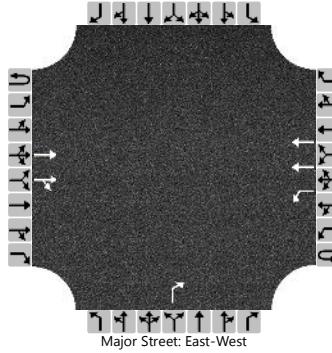


**AM PEAK HOUR**

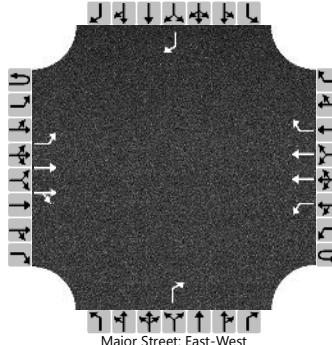
# HCS Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	J. Buckholz			Intersection			NE Park Street / SE 13th Avenue																							
Agency/Co.	BUCKHOLZ TRAFFIC			Jurisdiction			Okeechobee County																							
Date Performed	2/14/2023			East/West Street			NE Park Street																							
Analysis Year	2023			North/South Street			SE 13th Avenue																							
Time Analyzed	Weekday AM Peak Hour			Peak Hour Factor			0.86																							
Intersection Orientation	East-West			Analysis Time Period (hrs)			0.25																							
Project Description	#23-1820																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound				Westbound				Northbound																					
Movement	U	L	T	R	U	L	T	R	U	L	T																			
Priority	1U	1	2	3	4U	4	5	6	7	8	9																			
Number of Lanes	1	0	2	0	0	1	2	0	0	1	0																			
Configuration	U		T	TR		L	T			LR																				
Volume (veh/h)	4		879	4	0	7	1114		2		7																			
Percent Heavy Vehicles (%)	0				0	14			0		29																			
Proportion Time Blocked																														
Percent Grade (%)									0																					
Right Turn Channelized																														
Median Type   Storage	Left + Thru						1																							
Critical and Follow-up Headways																														
Base Critical Headway (sec)	6.4					4.1			7.5		6.9																			
Critical Headway (sec)	6.40					4.38			6.80		7.48																			
Base Follow-Up Headway (sec)	2.5					2.2			3.5		3.3																			
Follow-Up Headway (sec)	2.50					2.34			3.50		3.59																			
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)	5					8			10																					
Capacity, c (veh/h)	218					605			350																					
v/c Ratio	0.02					0.01			0.03																					
95% Queue Length, Q <sub>95</sub> (veh)	0.1					0.0			0.1																					
Control Delay (s/veh)	21.9					11.0			15.6																					
Level of Service (LOS)	C					B			C																					
Approach Delay (s/veh)	0.1			0.1			15.6																							
Approach LOS	A			A			C																							

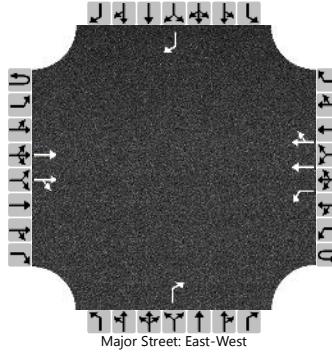
# HCS Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	J. Buckholz			Intersection		NE Park Street / Hampton Inn Driveway																								
Agency/Co.	BUCKHOLZ TRAFFIC			Jurisdiction		Okeechobee County																								
Date Performed	2/14/2023			East/West Street		NE Park Street																								
Analysis Year	2023			North/South Street		Hampton Inn Driveway																								
Time Analyzed	Weekday AM Peak Hour			Peak Hour Factor		0.86																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	#23-1820																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	U	L	T	U	L	T	U	L	T																		
Priority	1U	1	2	4U	4	5	6	7	8	9	10	11																		
Number of Lanes	0	0	2	0	1	2	0	0	0	1	0	0																		
Configuration			T	TR		L	T			R																				
Volume (veh/h)			875	2	0	4	1116			14																				
Percent Heavy Vehicles (%)					0	2				2																				
Proportion Time Blocked																														
Percent Grade (%)									0																					
Right Turn Channelized									No																					
Median Type   Storage	Left Only									1																				
Critical and Follow-up Headways																														
Base Critical Headway (sec)						4.1				6.9																				
Critical Headway (sec)						4.14				6.94																				
Base Follow-Up Headway (sec)						2.2				3.3																				
Follow-Up Headway (sec)						2.22				3.32																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)						5				16																				
Capacity, c (veh/h)						676				509																				
v/c Ratio						0.01				0.03																				
95% Queue Length, Q <sub>95</sub> (veh)						0.0				0.1																				
Control Delay (s/veh)						10.4				12.3																				
Level of Service (LOS)						B				B																				
Approach Delay (s/veh)	0.0			12.3																										
Approach LOS	A			B																										

# HCS Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	J. Buckholz			Intersection		NE Park Street / SE 13th Avenue																								
Agency/Co.	BUCKHOLZ TRAFFIC			Jurisdiction		Okeechobee County																								
Date Performed	7/7/2023			East/West Street		NE Park Street																								
Analysis Year	2024			North/South Street		SE 13th Avenue																								
Time Analyzed	AM Peak Hr. BUILD Traffic			Peak Hour Factor		0.86																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	#23-1820																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority	1U	1	2	3	4U	4	5	6	7	8	9	10	11	12																
Number of Lanes	0	1	2	0	0	1	2	1	0	0	1	0	0	1																
Configuration		L	T	TR		L	T	R			R			R																
Volume (veh/h)	4	18	917	4	0	7	1133	18			9			51																
Percent Heavy Vehicles (%)	0	2			0	14					29			2																
Proportion Time Blocked																														
Percent Grade (%)									0		0																			
Right Turn Channelized						No			No		No																			
Median Type   Storage		Left + Thru								1																				
Critical and Follow-up Headways																														
Base Critical Headway (sec)	6.4	4.1				4.1				6.9			6.9																	
Critical Headway (sec)	6.40	4.14				4.38				7.48			6.94																	
Base Follow-Up Headway (sec)	2.5	2.2				2.2				3.3			3.3																	
Follow-Up Headway (sec)	2.50	2.22				2.34				3.59			3.32																	
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)		26				8				10			59																	
Capacity, c (veh/h)		383				580				425			406																	
v/c Ratio		0.07				0.01				0.02			0.15																	
95% Queue Length, Q <sub>95</sub> (veh)		0.2				0.0				0.1			0.5																	
Control Delay (s/veh)		15.1				11.3				13.7			15.4																	
Level of Service (LOS)		C				B				B			C																	
Approach Delay (s/veh)	0.4			0.1			13.7			15.4																				
Approach LOS	A			A			B			C																				

# HCS Two-Way Stop-Control Report

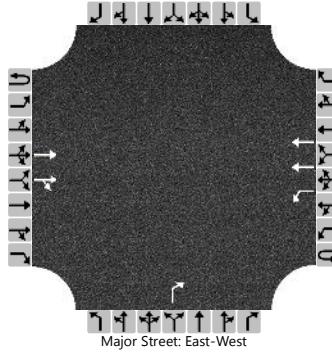
General Information				Site Information																																
Analyst	J. Buckholz			Intersection			NE Park St. / Hampton Inn / NE 12th Ave.																													
Agency/Co.	BUCKHOLZ TRAFFIC			Jurisdiction			Okeechobee County																													
Date Performed	7/7/2023			East/West Street			NE Park Street																													
Analysis Year	2024			North/South Street			Hampton Inn Drive / NE 12th Avenue																													
Time Analyzed	AM Peak Hr. BUILD Traffic			Peak Hour Factor			0.86																													
Intersection Orientation	East-West			Analysis Time Period (hrs)			0.25																													
Project Description	#23-1820																																			
Lanes																																				
 Major Street: East-West																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority	1U	1	2	3	4U	4	5	6		7	8	9	10																							
Number of Lanes	0	0	2	0	0	1	2	0	0	0	0	1	0																							
Configuration			T	TR		L	T	TR			R		R																							
Volume (veh/h)			906	2	25	4	1159	0			14		1																							
Percent Heavy Vehicles (%)					2	2					2		0																							
Proportion Time Blocked																																				
Percent Grade (%)									0		0																									
Right Turn Channelized									No		No																									
Median Type   Storage	Left Only										1																									
Critical and Follow-up Headways																																				
Base Critical Headway (sec)					6.4	4.1					6.9		6.9																							
Critical Headway (sec)					6.44	4.14					6.94		6.90																							
Base Follow-Up Headway (sec)					2.5	2.2					3.3		3.3																							
Follow-Up Headway (sec)					2.52	2.22					3.32		3.30																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)					34						16		1																							
Capacity, c (veh/h)					320						495		402																							
v/c Ratio					0.11						0.03		0.00																							
95% Queue Length, Q <sub>95</sub> (veh)					0.4						0.1		0.0																							
Control Delay (s/veh)					17.6						12.5		14.0																							
Level of Service (LOS)					C						B		B																							
Approach Delay (s/veh)				0.4			12.5			14.0																										
Approach LOS				A			B			B																										

**PM PEAK HOUR**

# HCS Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	J. Buckholz			Intersection			NE Park Street / SE 13th Avenue																													
Agency/Co.	BUCKHOLZ TRAFFIC			Jurisdiction			Okeechobee County																													
Date Performed	2/14/2023			East/West Street			NE Park Street																													
Analysis Year	2023			North/South Street			SE 13th Avenue																													
Time Analyzed	Weekday PM Peak Hour			Peak Hour Factor			0.91																													
Intersection Orientation	East-West			Analysis Time Period (hrs)			0.25																													
Project Description	#23-1820																																			
Lanes																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority	1U	1	2	3	4U	4	5	6	7	8	9	10	11																							
Number of Lanes	1	0	2	0	0	1	2	0	0	1	0	0	0																							
Configuration	U		T	TR		L	T			LR																										
Volume (veh/h)	5		1230	14	2	31	1136		2		16																									
Percent Heavy Vehicles (%)	0				0	3			0		0																									
Proportion Time Blocked																																				
Percent Grade (%)									0																											
Right Turn Channelized																																				
Median Type   Storage		Left + Thru									1																									
Critical and Follow-up Headways																																				
Base Critical Headway (sec)	6.4				6.4	4.1			7.5		6.9																									
Critical Headway (sec)	6.40				6.40	4.16			6.80		6.90																									
Base Follow-Up Headway (sec)	2.5				2.5	2.2			3.5		3.3																									
Follow-Up Headway (sec)	2.50				2.50	2.23			3.50		3.30																									
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)	5				36				20																											
Capacity, c (veh/h)	234				449				329																											
v/c Ratio	0.02				0.08				0.06																											
95% Queue Length, Q <sub>95</sub> (veh)	0.1				0.3				0.2																											
Control Delay (s/veh)	20.8				13.7				16.6																											
Level of Service (LOS)	C				B				C																											
Approach Delay (s/veh)		0.1				0.4			16.6																											
Approach LOS		A				A			C																											

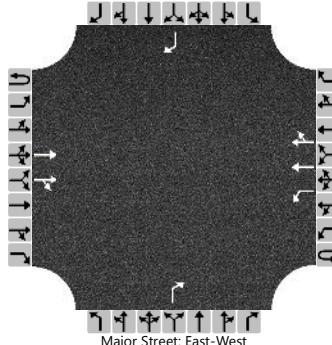
# HCS Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	J. Buckholz			Intersection		NE Park Street / Hampton Inn Driveway																								
Agency/Co.	BUCKHOLZ TRAFFIC			Jurisdiction		Okeechobee County																								
Date Performed	2/14/2023			East/West Street		NE Park Street																								
Analysis Year	2023			North/South Street		Hampton Inn Driveway																								
Time Analyzed	Weekday PM Peak Hour			Peak Hour Factor		0.91																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	#23-1820																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	U	L	T	U	L	T	U	L	T																		
Priority	1U	1	2	4U	4	5	6	7	8	9	10	11																		
Number of Lanes	0	0	2	0	1	2	0	0	0	1	0	0																		
Configuration			T	TR		L	T			R																				
Volume (veh/h)			1232	36	0	19	1123			17																				
Percent Heavy Vehicles (%)					0	2				2																				
Proportion Time Blocked																														
Percent Grade (%)									0																					
Right Turn Channelized									No																					
Median Type   Storage	Left Only									1																				
Critical and Follow-up Headways																														
Base Critical Headway (sec)						4.1				6.9																				
Critical Headway (sec)						4.14				6.94																				
Base Follow-Up Headway (sec)						2.2				3.3																				
Follow-Up Headway (sec)						2.22				3.32																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)						21				19																				
Capacity, c (veh/h)						487				384																				
v/c Ratio						0.04				0.05																				
95% Queue Length, Q <sub>95</sub> (veh)						0.1				0.2																				
Control Delay (s/veh)						12.7				14.9																				
Level of Service (LOS)						B				B																				
Approach Delay (s/veh)				0.2		14.9																								
Approach LOS				A		B																								

# HCS Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	J. Buckholz			Intersection			NE Park Street / SE 13th Avenue																													
Agency/Co.	BUCKHOLZ TRAFFIC			Jurisdiction			Okeechobee County																													
Date Performed	7/7/2023			East/West Street			NE Park Street																													
Analysis Year	2024			North/South Street			SE 13th Avenue																													
Time Analyzed	PM Peak Hr. BUILD Traffic			Peak Hour Factor			0.91																													
Intersection Orientation	East-West			Analysis Time Period (hrs)			0.25																													
Project Description	#23-1820																																			
Lanes																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority	1U	1	2	3	4U	4	5	6	7	8	9	10	11																							
Number of Lanes	0	1	2	0	0	1	2	1	0	0	1	0	0																							
Configuration		L	T	TR		L	T	R			R		R																							
Volume (veh/h)	14	88	1336	14	2	31	1167	84			18		130																							
Percent Heavy Vehicles (%)	2	2			0	3					0		2																							
Proportion Time Blocked																																				
Percent Grade (%)									0		0																									
Right Turn Channelized						No			No		No																									
Median Type   Storage		Left + Thru									1																									
Critical and Follow-up Headways																																				
Base Critical Headway (sec)	6.4	4.1			6.4	4.1					6.9		6.9																							
Critical Headway (sec)	6.44	4.14			6.40	4.16					6.90		6.94																							
Base Follow-Up Headway (sec)	2.5	2.2			2.5	2.2					3.3		3.3																							
Follow-Up Headway (sec)	2.52	2.22			2.50	2.23					3.30		3.32																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)		112			36				20				143																							
Capacity, c (veh/h)		370			400				363				417																							
v/c Ratio		0.30			0.09				0.05				0.34																							
95% Queue Length, Q <sub>95</sub> (veh)		1.3			0.3				0.2				1.5																							
Control Delay (s/veh)		18.9			14.9				15.5				18.1																							
Level of Service (LOS)		C			B				C				C																							
Approach Delay (s/veh)	1.3			0.4			15.5			18.1																										
Approach LOS	A			A			C			C																										

# HCS Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	J. Buckholz			Intersection			NE Park St. / Hampton Inn / NE 12th Ave.																													
Agency/Co.	BUCKHOLZ TRAFFIC			Jurisdiction			Okeechobee County																													
Date Performed	7/7/2023			East/West Street			NE Park Street																													
Analysis Year	2024			North/South Street			Hampton Inn Driveway / NE 12th Avenue																													
Time Analyzed	PM Peak Hr. BUILD Traffic			Peak Hour Factor			0.91																													
Intersection Orientation	East-West			Analysis Time Period (hrs)			0.25																													
Project Description	#23-1820																																			
Lanes																																				
 Major Street: East-West																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority	1U	1	2	3	4U	4	5	6		7	8	9	10																							
Number of Lanes	0	0	2	0	0	1	2	0	0	0	0	1	0																							
Configuration			T	TR		L	T	TR			R		R																							
Volume (veh/h)			1347	37	88	19	1182	21			17		46																							
Percent Heavy Vehicles (%)					2	2					2		2																							
Proportion Time Blocked																																				
Percent Grade (%)									0		0																									
Right Turn Channelized									No		No																									
Median Type   Storage	Left Only										1																									
Critical and Follow-up Headways																																				
Base Critical Headway (sec)					6.4	4.1					6.9		6.9																							
Critical Headway (sec)					6.44	4.14					6.94		6.94																							
Base Follow-Up Headway (sec)					2.5	2.2					3.3		3.3																							
Follow-Up Headway (sec)					2.52	2.22					3.32		3.32																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)					118					19			51																							
Capacity, c (veh/h)					164					348			405																							
v/c Ratio					0.72					0.05			0.12																							
95% Queue Length, Q <sub>95</sub> (veh)					4.3					0.2			0.4																							
Control Delay (s/veh)					68.7					15.9			15.2																							
Level of Service (LOS)					F					C			C																							
Approach Delay (s/veh)				5.6			15.9			15.2																										
Approach LOS				A			C			C																										