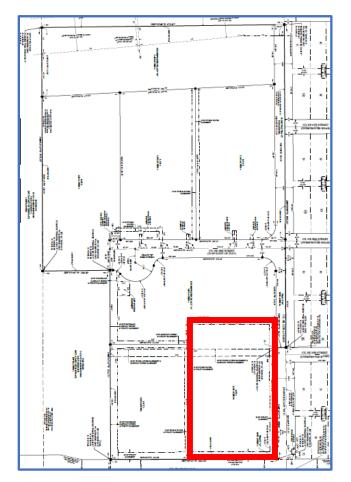


Site Plan Review Staff Report



Applicant | Park Street Okeechobee, LLC Parcel Identification | 3-15-37-35-0210-00010-0010 and 2-15-37-35-0A00-



Prepared for The City of Okeechobee

MORRIS

DEPEW

General Information

Owner: William R. Grigsby, Jr. Applicant: Park Street Okeechobee, LLC Primary Contact: Josh Cockriel, <u>josh.cockriel@kimley-horn.com</u>, (904) 828-3900 Site Address: Highway 70 E, Okeechobee, FL Parcel Identification: 3-15-37-35-0210-00010-0010 and 2-15-37-35-0A00-00010-0000

Note: For the legal description of the project or other information relating this application, please refer to the application submittal package which is available by request at City Hall and is posted on the City's website prior to the advertised public meeting at: <u>https://www.cityofokeechobee.com/agendas.html</u>

Future Land Use, Zoning, and Existing Use of Subject Property(s)

	Existing	Proposed
Future Land Use	Commercial	Commercial
Zoning	Heavy Commercial (CHV)	Heavy Commercial (CHV)
Use of Property	Vacant	Culver's Restaurant
Acreage	2.16 (replat)	2.16 acres

Future Land Use, Zoning, and Existing Use of Surrounding Properties

	Future Land Use	Zoning	Existing Use
North	Commercial	Heavy Commercial (CHV)	Vacant
East	(Okeechobee County) Commercial Corridor Mixed-Use		Unimproved NE 13 th Ave, Vehicle Sales, Vacant Property
South	Commercial	Heavy Commercial (CHV)	SR-70, post office, service commercial
West	Commercial	Heavy Commercial (CHV)	Vacant, proposed car wash



General Description

The request for consideration by the City's Technical Review Committee is an application for Site Plan Review of a 4,827 sq/ft fast-food restaurant with drive through service, located on Lot 2 of the Park Street Commerce Center. Please see application 23-003-TRC for more information related to the replat and associated infrastructure plans. The applicant has submitted a concurrent special exception application (23-001-SE) for approval of the proposed drive-through. Access to the 2.16-acre subject property is provided by NE 13th Ave, which will be improved as part of the replat. Staff analysis of the submitted application and plans is provided below. Areas of deficiency or concern are highlighted in yellow.

Adequacy of Public Facilities

Potable Water and Sewer: OUA has provided a letter indicating the water and sewer plants have available remaining capacity and that any extensions or upgrades necessary due to the designs or demands of the proposed project will be at the owner's expense.

<u>Traffic Generation</u>: The applicant has provided a traffic analysis demonstrating available roadway capacity for a 4,600 sq/ft drive-through restaurant.

Access and Internal Circulation: Two ingress/egress driveways are proposed on the eastern property boundary with access to NE 13th Ave. An ingress/egress driveway is proposed on the western property boundary, providing through-access to the abutting commercial parcel to the west. Internal two way drive aisles are a minimum 24' wide. A crosswalk for pedestrians is provided from the northern parking area through the drive-through lanes to provide access to the building sidewalk.

Additionally, the applicant has provided a queuing analysis with the special exception application. The analysis demonstrates that the site contains sufficient vehicle stacking capacity to accommodate peak demand without causing vehicle stacking to exceed the boundaries of the site.

Service Vehicle Access and Egress:

- **A.** <u>Fire Truck</u> Sufficiency of fire truck access and egress to be addressed by the Fire Department.
- **B.** <u>Loading Zone</u>
 No Loading zones are required per the City's zoning code, and none are proposed.
- C. <u>Dumpster Location and/or Trash Collection</u> The site plan provides a dumpster enclosure. Sufficiency of the dumpster enclosure and location will be addressed by the solid waste provider.

Consistency and Compatibility with Adjacent Uses

The proposed drive-through restaurant use is consistent with the comprehensive plan and is compatible with surrounding uses. Drive-through service is a special exception use within the Heavy Commercial (CHV) zoning district. The applicant has submitted a concurrent special exception application, 23-001-SE, to approve the drive-through use. The City's Board of Adjustment will determine whether the drive through is approved, denied, or approved with conditions. The subject property has frontage on SR-70, proximate to other commercial uses, and it is not expected that the proposed restaurant use will create any adverse impacts on the health, safety, and welfare of the surrounding community.



Compliance with Land Development Code

Regulation	Requirement	Provided
Permitted Uses §90-282	Restaurant, take-out restaurant, café are permitted principal uses within the CHV district	In compliance
Permitted Special Exception Uses §90-283	Drive-through service is a permitted special exception use within the CHV district	The applicant has submitted a concurrent special exception application for approval of drive-through service.
Minimum Lot Area §90-196(1)	6,250 square feet for all uses	93,872
Minimum Lot Width §90-285(1)	50'	245'
Min front yard setback §90-285(2)(a)	20' to buildings; ten feet to parking and driveway	In compliance
Yards on corner lots §90-447	Any yard adjoining a street shall be considered a front yard. That yard upon which the property is addressed is required to comply with the minimum depth requirements of the regulations of this article. All other front yards shall be not less than 75 percent of the required minimum depth.	Addressing to be completed subsequent to final plat approval. Secondary front yard not determined. Site plan meets setbacks whether addressed on SR-70 or 13 th Ave.
Minimum Required Side Setbacks §90-285(2)(a)	8'; 20' abutting residential zoning district	In compliance
Minimum Required Rear Yard §90-285(2)(a)	10'; 20' abutting a residential zoning district.	In compliance
Max lot coverage §90-285(3)	50%	5.14%
Max impervious surface §90-285(3)	85%	67.8%
Max height §90-285(4)	45 feet	In compliance
Parking spaces location §90-511(a)	Required off-street parking and loading spaces shall be located on the same parcel as the primary use, unless approved by TRC upon submittal of written agreement to ensure continued availability	In compliance
Min parking space dimensions §90-511(b)	9' by 20'	In compliance



Regulation	Requirement	Provided
Min parking access width §90-511(d)(2)	 a. Parking spaces between 75° and 90° to the driveway: 24' b. Parking spaces angled from 60° up to but not including 75° to the driveway: 20' c. Parking spaces any other angle to the driveway: 16' 	In compliance
Paving §90-511(e)(1)	Each parking and loading space shall be paved	In compliance
Parking and loading space layout §90-511(e)(2)	Each parking space shall be designed to permit access without moving another vehicle.	in compliance
Parking and loading space layout §90-511(e)(3)	Buildings, parking and loading areas, landscaping and open spaces shall be designed so that pedestrians moving between parking areas and buildings are not unreasonably exposed to vehicular traffic hazards.	Internal pedestrian paths provided and marked
Parking and loading space layout §90-511(e)(4)	Paved pedestrian walks shall be provided along the lines of the most intense use, particularly between building entrances to streets, parking areas, and adjacent buildings.	Internal pedestrian paths provided and marked
Parking and loading space layout §90-511(e)(6)	For new construction, no parking space accessed via a driveway from a public road shall be located closer than 20 feet from the right-of-way line of said public road.	In compliance
Space Regulations §90-512(2)	1 space per 75 sq/ft of customer service area <u>Exact customer svc area not provided.</u> <u>Assuming CSA is 60% of total:</u> <u>4,867 x 0.6 = 2,920</u> <u>2,920 ÷ 75 = 39</u>	~39 spaces required 77 total spaces proposed
Min number of ADA parking spaces FL Accessibility Code §208.2	4 ADA spaces required for 76 to 100 spaces provided	4 ADA spaces proposed
Min ADA parking space dimensions FL Accessibility Code §502	12' by 20' w/ a 5' wide access aisle	In compliance



Regulation	Requirement	Provided
Off-street loading space	1 loading space for 5,000 to 25,000	No loading space proposed
requirement	square feet floor area	
regulations;		
Commercial, Industrial	No loading space required for	
§90-513(2)	proposed 4,827 sq/ft structure	
Required Landscaping	At least 1 tree and 3 shrubs shall be	32 trees and 94 shrubs are
§90-532	planted for every 3,000 sq/ft of lot	required;
	area, excluding areas of existing	
	vegetation which are preserved.	62 trees and 630 shrubs are proposed.
	<u>93,872 ÷ 3,000 = 32 trees req</u>	proposed.
	<u>94 shrubs req</u>	
All vehicular use areas cor	ntaining eight or more parking spaces, or	r containing an area greater than
	ovide perimeter and interior landscapin	
Landscaping	At least 18 square feet of landscaped	In compliance
Requirements for	area for each required parking space.	
Parking and Vehicular		
Use Areas	<u>39 x 18 = 702</u>	
§90-533(1)		
Landscaping	At least one tree for each 72 square	In compliance
Requirements for	feet of required landscaped area.	
Parking and Vehicular		
Use Areas	<u>702 ÷ 72 = 10</u>	
§90-533(2)		· · · · ·
Landscaping	Shade trees shall be planted at no	In compliance
Requirements for	more than 20 feet on centers	
Parking and Vehicular		
Use Areas		
§90-533(3) Landscaping	A minimum two feet of landscaping	At least 2.42' provided
Requirements for	shall be required between vehicular	At least 2.42 provided
Parking and Vehicular	use areas and on-site buildings and	
Use Areas	structures, except at points of ingress	
§90-533(4)	and egress.	
Landscaping	The minimum dimension for any	At least 8' width provided
Requirements for	required landscaped area within a	•
Parking and Vehicular	parking or vehicular use area shall be	
Use Areas	four feet except for that adjacent to	
§90-533(5)	on-site buildings and structures.	
Landscaping	A landscaped island, minimum five	No more than 10 uninterrupted
Requirements for	feet by 15 feet and containing at least	parking spaces in a row proposed
Parking and Vehicular	one tree, shall be required for every	
Use Areas	ten parking spaces with a maximum	
§90-533(6)	of 12 uninterrupted parking spaces in	
	a row.	





Regulation	Requirement	Provided
Landscaping Requirements for Parking and Vehicular Use Areas §90-533(7)	The remainder of a parking landscape area shall be landscaped with grass, ground cover, or other landscape material.	In compliance
Landscape buffer areas §90-534(1)	Minimum width of buffer along street frontage shall be ten feet and on other property lines, two feet.	In compliance
Landscape buffer areas §90-534(2)	At least 1 tree and 3 shrubs for each 300 sq/ft of required landscaped buffer	
	245 linear ft of north property line requires 490 sf of landscaped area with 2 trees and 5 shrubs	5 trees & 16 shrubs
	<u>317 linear ft of non-driveway</u> <u>frontage on 13th Ave requires 3,170 sf</u> <u>of landscaped area with 11 trees and</u> <u>32 shrubs</u>	13 trees & 119 shrubs
	<u>245 linear ft of frontage on SR70</u> <u>requires 2,450 sf of landscaped area</u> <u>with 9 trees and 25 shrubs</u>	9 trees & 63 shrubs
	<u>359 linear ft of non-driveway west</u> property line requires 766 sf of landscaped area with 2 trees and 4 shrubs	14 trees & at least 42 shrubs
Landscape buffer areas §90-534(3)	Trees may be planted in clusters, but shall not exceed 50' on centers abutting the street	In compliance
Landscape buffer areas §90-534(4)	The remainder of a landscape buffer shall be landscaped with grass, ground cover, or other landscape material.	In compliance
Landscape design and plan §90-538(a)	Proposed development, vehicular and pedestrian circulation systems, and site drainage shall be integrated into the landscaping plan.	In compliance
Landscape design and plan §90-538(b)	Existing native vegetation shall be preserved where feasible, and may be used in calculations to meet these landscaping requirements.	The applicant is not proposing to preserve any native vegetation.



Regulation	Requirement	Provided
Species diversification §90-538(c)	When more than ten trees are required to be planted, two or more species shall be used.	4 tree species proposed
Tree spacing from utility structures §90-538(d)	Trees and shrubs shall not be planted in a location where at their maturity they would interfere with utility services	In compliance
Landscape design and plan §90-538(e)	Trees should maximize the shading of pedestrian walks and parking spaces.	In compliance
Landscape design and plan §90-538(f)	Landscaping ground covers should be used to aid soil stabilization and prevent erosion.	In compliance
Landscape design and plan §90-538(g)	Landscaping shall be protected from vehicular encroachment by means of curbs, wheel stops, walks or similar barriers.	In compliance
Drought tolerance §90-540(b)	At least 75 percent of the total number of plants required shall be state native very drought tolerant species as listed in the South Florida Water Management District Xeriscape Plant Guide. However, when a landscape irrigation system is installed, at least 75 percent or the total number of plants required shall be state native moderate or very drought tolerant species.	Native trees make up 100% of proposed plantings;
Min tree size §90-540(c)	Trees shall be at least ten feet high and two inches in diameter measured four feet above ground level at the time of planting.	In compliance



Regulation	Requirement	Provided
Utility Corridor	No tree shall be planted where it could,	In compliance
Requirements	at mature height, conflict with overhead	•
§90-543(b)	utility lines. Larger trees (trees with a	
350 545(2)	mature height of 30 feet or more) shall	
	be planted no closer than a horizontal	
	distance of 30 feet from the nearest	
	overhead utility line. Medium trees (trees	
	with a height of 20 to 30 feet) shall be	
	offset at least 20 feet horizontally from	
	the nearest overhead utility line. Small	
	trees (trees with a mature height of less	
	than 20 feet) shall not be required to	
	meet a minimum offset, except that no	
	tree, regardless of size shall be planted	
	within five feet of any existing or	
	proposed utility implement.	
Sign definitions	Freestanding sign means any sign, which is	
§90-564	incorporated into or supported by structures	
-	or supports in or upon the ground,	
	independent of support from any building.	
	Freestanding sign includes pole sign, pylon sign, ground sign or monument sign or	
	"sandwich sign."	
	Ground sign means a freestanding sign, other	
	than a pole sign, supported by the ground, or	
	by uprights or braces placed on or in the	
	ground, and wholly independent of any building for support.	
	Illuminated sign means any sign which	
	contains a source of light or which is designed	
	or arranged to reflect light from an artificial	
	source including indirect lighting, neon,	
	incandescent lights, backlighting, and also shall include signs with reflector that depend	
	upon motor vehicle headlights for an image.	
	Monument sign means a ground sign in which	
	the entire bottom of the sign is in contact with the ground.	
	Sign area means the area within the smallest	
	regular geometric shape which contains the	
	entire sign copy, but not including any	
	supporting framework, braces or supports.	
	Sign copy means the linguistic or graphic content, including trim and borders, of a sign.	
	Sign face means the part of the sign that is or may be used to display sign copy.	
	Sign height means the vertical distance from	
	the finished grade at the base of the	
	supporting structure to the top of the sign, or	
	its frame or supporting structure, whichever is	
	higher.	



Regulation	Requirement	Provided
Sign area computation §90-569(b)	 (a) For freestanding signs, the sign area shall be the area within the smallest geometric shape that touches the outer points or edges of the sign face. (c) For freestanding signs where two sign faces are placed back to back on a single sign structure, and the faces are at no point more than four feet apart, the sign area shall be the area 	Area of oval portion of sign not provided Area of rectangular portion of sign is ~28 sf Site plan indicates sign width at 3 ft, which allows for computation of sign area to be one of the faces.
Free standing monument signs §90-571(1)	of one of the faces. The sign area for each multiple occupancy complex and each occupant not located in a multiple occupancy complex shall not exceed 64 square feet in area, or eight feet in height	Total sign area not provided. Height proposed at 12 ft
Free standing monument signs §90-571(2)	No development shall have more than one freestanding monument sign.	One monument sign proposed
Free standing monument signs §90-571(3)	Permitted if located consistent with all applicable set back requirements and are not located in a public right- of-way	Monument sign located to meet 20 ft setbacks from both the east and south property line.
Ground signs §90-573(a)(1)	One ground sign or pole sign is allowed in the front yard, and such sign shall not exceed 50 square feet in sign area and 20 feet in height, and shall not be closer than 25 feet to a residential district.	Applicant may propose signage consistent with this provision, provided sign plans demonstrate compliance. <mark>Exact sign area not</mark> provided
Sidewalks § 78-36	Sidewalks shall be provided along each right-of-way.	Existing sidewalks provided along SR-70. Sidewalks proposed on 13 th Ave as part of infrastructure improvements for plat 23-003- TRC.



Recommendation

Based on the foregoing analysis, we recommend approval with the following conditions:

- 1. Plans must be consistent with final plat approval and all conditions of that approval.
- 2. Sign plans and location of sign shall not be approved until submittal of sign plans consistent with LDC or consistent with an approved sign variance.
- 3. Approval of this site plan is contingent on approval of special exception request 23-001-SE.
- 4. No building permit may be issued until all conditions of approval of 23-003-TRC have been met.

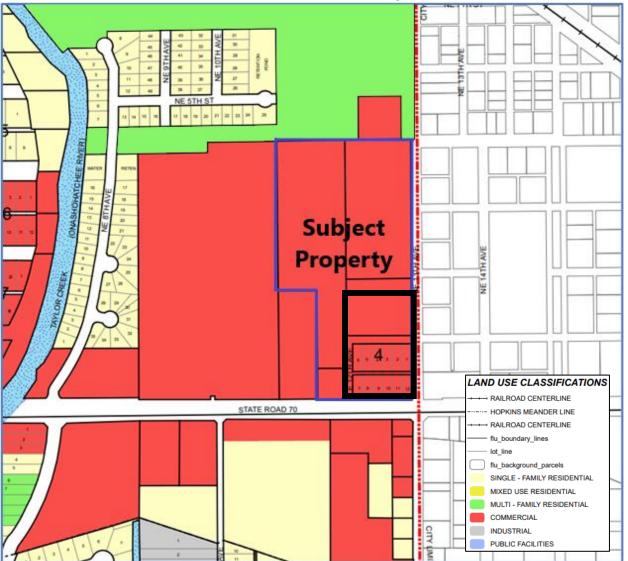
Submitted by:

Ben Smith, AICP Director of Planning November 8, 2023

Okeechobee Technical Review Committee Hearing: November 16, 2023









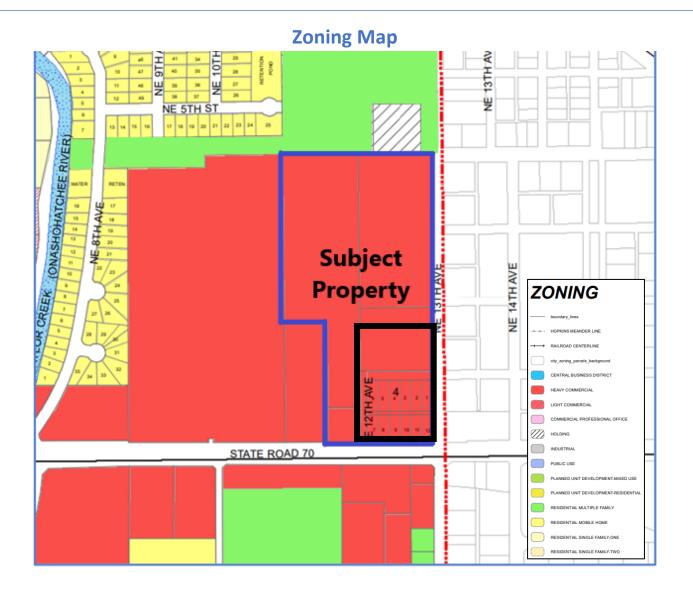




Exhibit C: Existing Land Use Map



CITY OF OKEECHOBEE

Application for Site Plan Review

Pag 1 of 3

		City of Okeechobee General Services Departme 55 S.E. 3 rd Avenue, Room 10 Okeechobee, Florida 34974 Phone: (863) 763-3372, ext. Fax: (863)763-1686 E-mail <u>: pburnette@cityofoke</u>	01 9820	Date Received8-15-23Application No.23-004-TRC.Fee Paid:2/23/23 \$Receipt No.59178 \$Hearing Date:9-21-23	
		APPLICANT I	NFORMATI	ON	
1	Name of property owner(s): Willi	am R. Grigsby, Jr.			
2	Owner mailing address: 10282	Payne Road, Sebring, Fl	orida 3387	5	
3	Name of applicant(s) if other the	an owner: Park Street Oke	echobee, L	LC	
4	Applicant mailing address: 603	East Fort King Street, Oc	cala, Florida	34471	
5	Name of contact person (state rela	ationship): Josh Cockriel,	P.E. (Civil E	ngineer)	
6	Contact person daytime phone(s)	and email address: (904) 82	8-3900 / jos	sh.cockriel@kimley-horn.com	
7	Engineer: Name, address and phone number:				-3909
	Surveyor: Name, address and phone number:				
8	BSM & Associates, Inc. c/o Richard Barnes, 80 SE 31st Lane, Okeechobee, Florida 34974, (863) 484-8324				
	PROPERTY and PROJECT INFORMATION				
	Property address/directions to pro				
9	Those certain 2 parcels of la Road 70 East, Okeechobee	pperty: Future Lot & I and totaling approximately , Florida	PARK_STREE 2.16+/- acre	F CommERCE CENTER	ate
9 10	Those certain 2 parcels of la Road 70 East, Okeechobee Parcel Identification Number 3-1	operty: Future Lot & I and totaling approximately , Florida 5-37-35-0210-00010-0	PARK_STREE 2.16+/- acre	COMMERCE CENTER	ate
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10	Those certain 2 parcels of la Road 70 East, Okeechobee Parcel Identification Number 3-1	pperty: Future Lot & I and totaling approximately , Florida 5-37-35-0210-00010-00 ^{tion:} Commercial	PARK_STREE 2.16+/- acre	F CommERCE CENTER	ate
10 11	Those certain 2 parcels of la Road 70 East, Okeechobee Parcel Identification Number 3-1 Current Future Land Use designa Current Zoning district: Heavy Describe the project including all is expected to operate on the site, extent and type of any outdoor sto Proposed 4,827 sqft. Culve	pperty: Future Lot & I and totaling approximately Florida 5-37-35-0210-00010-00 tion: Commercial Commercial proposed uses, type of constr including but not limited to: brage or sales, etc., and fire fl er's quick service resta	2.16+/- acre 2.16+/- acre 010 and 2- ruction and co number of em ow layout. Us urant and a	T CommERCE CENTER es located directly across from 1000 Sta 15-37-35-0A00-00010-0000 nceptual building layout, how the business of ployees expected; hours of operation; location	or use on, N
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10 11 12	Those certain 2 parcels of la Road 70 East, Okeechobee, Parcel Identification Number 3-1 Current Future Land Use designa Current Zoning district: Heavy Describe the project including all is expected to operate on the site, extent and type of any outdoor sto Proposed 4,827 sqft. Culve space. Culver's typically of	operty: Future Lot 2 / and totaling approximately , Florida 5-37-35-0210-00010-00 tion: Commercial Commercial proposed uses, type of constru- including but not limited to: orage or sales, etc., and fire flu- er's quick service resta operates less than 16 h on property (for example, the if necessary.	2.16+/- acre 2.16+/- acre 010 and 2- ouction and co number of em ow layout. Us urant and a ours per da	T CommERCE CENTER as located directly across from 1000 Sta 15-37-35-0A00-00010-0000 nceptual building layout, how the business of ployees expected; hours of operation; location e additional page if necessary. Associated parking areas and gree by and does not include outdoor sa	or use on, n ales.
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CITY OF OKEECHOBEE

Application for Site Plan Review

17	Number and description of phases: One Phase.
18	Source of potable water: Okeechobee Utility Authority
19	Method of sewage disposal: Gravity sewer to master development system.

	ATTACHMENTS REQUIRED FOR ALL APPLICATIONS
20	Applicant's statement of interest in property
21	One (1) copy of last recorded warranty deed
22	Notarized letter of consent from property owner (if applicant is different from property owner)
23	 Three (3) CERTIFIED BOUNDARY and TOPOGRAPHIC surveys, (one to be no larger than 11 x 17; scale not less than one inch to 20 feet; North point) containing: a. Date of survey, surveyor's name, address and phone number b. Legal description of property pertaining to the application c. Computation of total acreage to nearest tenth of an acre d. Location sketch of subject property, and surrounding area within one-half mile radius
24	Two (2) sets of aerials of the site.
25	Two (2) copies of sealed site plan drawings (see attached checklist for details of items to be included)
26	Two (2) copies of drawing indicating facades for all buildings, including architectural elevations.
27	Two (2) copies of landscape plan, including a separate table indicating the number of trees and shrubs by type and showing both the official and common name of each type of tree and shrub.
28	Two (2) copies of photometric lighting plan (see Code of Ordinances & LDR's Section 78-71 (A) (5)).
29	Two (2) copies of sealed drainage calculations.
30	Attach a Traffic Impact Study prepared by a professional transportation planner or transportation engineer, if the rezoning or proposed use will generate 100 or more peak hour vehicle trip ends using the trip generation factors for the most similar use as contained in the Institute of Transportation Engineers most recent edition of <u>Trip Generation</u> . The TIA must identify the number of net new external trips, pass-bay calculations, internal capture calculations, a.m. and p.m. peak hour trips and level of service on all adjacent roadway links with and without the project.
31	USB flash drive of application
32	Nonrefundable application fee: \$1,000.00 plus \$30.00 per acre. NOTE: Resolution No. 98-11 Schedule of Land Development Regulation Fees and Charges - When the cost for advertising, publishing and mailing notices of public hearings exceeds the established fee, or when a professional consultant is hired to advise the City on the application, the applicant shall pay the actual costs.
doc	ΓΕ: Submissions will be reviewed by the General Services Coordinator and City Planner for all necessary umentation. The Applicant will be notified at least 10 days prior to the TRC meeting whether or not itional information is required to proceed or if the review will be rescheduled to the next TRC meeting.
	Confirmation of Information Accuracy
	I hereby certify that the information in this application is correct. The information included in this application is for use by the City of Okeechobee in processing my request. False or misleading information may be punishable by a fine of up to \$500.00 and imprisonment of up to 30 days and may result in the summary denial of this application.
	Adam Ramsay 8/15/23
L	Signature Printed Name Date

For questions relating to this application packet, call the General Services Dept. at (863) 763-3372, Ext. 9820 Rev. 08/23

CITY OF OKEECHOBEE

Application for Site Plan Review

City of Okeechobee Checklist for Site Plan Review

		REQUIRED INFORMATION
1		Completed application (1)
2		Map showing location of site (may be on the cover sheet of site plan)
3		Two (2) copies of sealed site plan drawings prepared at a scale no smaller than one inch equals 60 feet, or in the case of small projects, the largest scale that can accommodate the entire site and all areas within 50 feet of the project boundary, and the scale, legend, and author block all on one 24" by 36" sheet. The site plan drawings shall include the location of all existing and proposed improvements, including, but not limited to:
	3.1	Water courses, water bodies, floodplains, wetlands, important natural features and wildlife areas, soil types, protected trees and vegetation or environmentally sensitive areas
	3.2	Streets, sidewalks, property lines and rights-of-way
	3.3	Utility lines/facilities, fire hydrants, septic tanks and drainfields
	3.4	Bridges, culverts and stormwater management facilities
	3.5	Buildings and structures and their distances from boundaries of the property, streets, and other structures
	3.6	Setback lines and required yards
	3.7	Ingress and egress to the site and buildings
	3.8	Vehicular use areas including off-street parking and loading areas
	3.9	On-site recreation and open space
	3.10	
	3.11	Method of solid waste collection and locations of and access to dumpsters
	3.12	
4		Drawing notes and tabulations showing the following information shall be included along with the plan:
	4.1	Name, address and phone number of the owner
	4.2	Name, address and phone number of any agent, architect, engineer and planner
	4.3	Compete legal description of the property
	4.4	Future land use designation, current zoning and existing land use of the property and all abutting properties
	4.5	Total acreage of the property (square footage if less than two acres)
	4.6	Total # of dwelling units, by bedroom size; square footage of nonresidential uses by type of use (and/or seating, etc. as necessary to indicate the intensity)
	4.7	Number of off-street parking spaces provided (including handicapped spaces) and loading spaces and the calculation of, and basis for, the number of such spaces required by the Land Development Regulations
	4.8	Impervious surface calculations showing: the square footage and as a% of the total site for existing impervious surfaces, additional proposed impervious surfaces and the resulting proposed total impervious surfaces



Department of State / Division of Corporations / Search Records / Search by Entity Name /

Detail by Entit	y Nam	e
Florida Limited Liability		
PARK STREET OKEE		-
Filing Information		
Document Number	L22	000487870
FEI/EIN Number	92-3	3774303
Date Filed	11/1	4/2022
State	FL	
Status	ACT	IVE
Principal Address		
603 EAST FORT KING OCALA, FL 34471	STREET	-
Mailing Address		
603 EAST FORT KINC OCALA, FL 34471	STREET	-
Registered Agent Name	e & Addres	<u>55</u>
RAMSAY, ADAM 603 EAST FORT KINC OCALA, FL 34471	STREET	
Authorized Person(s) D	Detail	
Name & Address		
Title MGR		
RAMSAY, ADAM P 603 EAST FORT KINC OCALA, FL 34471	STREET	-
Annual Reports		
Report Year Fil	ed Date	
2023 04	/28/2023	
Document Images		
04/28/2023 ANNUAL REPO	DRT	View image in PDF format
11/14/2022 Florida Limited	<u>Liability</u>	View image in PDF format

Сіту о**f** Океесновее 55 SE 3rd Avenue Океесновее, FL 34974 Tele: 863-763-3372 Fax: 863-763-1686

LAND USE POWER OF ATTORNEY

Name of Property Owner	s: WILLIAM R. GRIGSE	BY, JR.
Mailing Address: 10282	2 Payne Road, Sebring,	Florida 33875
Home Telephone: N/A	Work:	Cell:
Property Address:	slow totaling approximately 16.2 acres and	located in close proximity to 975 NE Park Street, Okeechobee, Florida 34972
Parcel ID Number: Parcel #1: 2-15-37-35-0A00-00011-0000, Parcel #2: 2-15-	· · · · ·	00009-A000, Parcel #4: 2-15-37-35-0A00-00010-0000, and Parcel #5: 3-15-37-35-0210-00010-0010.
Name of Applicant: Park Street Okeechobee, LLC a	ind its successors and assigns	
Home Telephone: (321) 704 - 2840	Work:	Cell:
understood that conditions, property. Misstatements up exception or variance and a attorney may be terminated receipt by the Planning Depa	limitations and restrictions oon application or in any hea proceeding to rezone the pro- only by a written and notan artment.	s of decisions of the Planning Department. It is may be place upon the use or operation of the aring may result in the termination of any special operty to the original classification. This power of rized statement of such termination effective upon E SET THEIR HAND AND SEALS THIS <u>/3</u>
OWNER		WITNESS
STATE OF FLORIDA COUNTY OF Highlan	2ls	
notarization, this 13 day	of <u>Feb</u> , 2023	(Name of Person)
who is personally known to n	ne or produced <u>Florid</u>	a.T.D. as identification.
S A George	Public State of Florida D Stickle kmission HH 044484 09/20/2024	<u>Alongo D. Stickle</u> NOTARY PUBLIC SIGNATURE
(Rev 4/2020)	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Page 5 of 11

PARK STREET OKEECHOBEE, LLC

603 East Fort King Street Ocala, Florida 34471

VIA EMAIL

August 11, 2023

City of Okeechobee / General Services Department Attention: Patty Burnette 55 S.E. 3rd Avenue, Room 101 Okeechobee, Florida 34974

Re: Park Street Commerce Center - Statement of Interest in Property

Dear Ms. Burnette:

On behalf of Park Street Okeechobee, LLC ("Applicant"), this letter constitutes Applicant's Statement of Interest in the following property (collectively, the "Property"):

- Parcel #1: 2-15-37-35-0A00-00011-0000
- Parcel #2: 2-15-37-35-0A00-00009-0000 Portron of Lot
 Parcel #3: 2-15-37-35-0A00-00009-A000 ALL
- Parcel #4: 2-15-37-35-0A00-00010-0000 •
- Parcel #5: 3-15-37-35-0210-00010-0010

Applicant is purchasing the Property via that certain vacant land contract between Applicant (as successor-by-assignment to WGT, Inc.) and William R. Grigsby, Jr. ("Seller") dated September 2, 2022 (the "Contract"). Applicant intends to develop the Property into a commercial real estate project to be commonly known as the Park Street Commerce Center ("Project") consisting of five separate parcels (as more particularly described in Applicant's site plan and other related documents) with portions of shared common area infrastructure, including without limitation, roadways, drainage, lift station, utility lines, project signs, entrance features, outfall pipe, sewer connection, and all other real and personal property (or interest therein) intended by Applicant for the common use and enjoyment of all lot owners within the Project (collectively, "Common Area Infrastructure").

The Common Area Infrastructure will be governed by that certain Declaration of Covenants, Conditions, Easements, and Restrictions for Park Street Commerce Center ("Declaration") of which Applicant (or its assignee) shall be the "Declarant" under the Declaration. The Declaration shall include, without limitation, provisions addressing the design, permitting, construction, maintenance, and repair of the Common Area Infrastructure, and provisions for sharing of the costs thereof by the lot owners of parcels within the Project. All easements, water management system, and other rights related to the Common Area Infrastructure will be incorporated into the Declaration. Notwithstanding the foregoing, Applicant intends to dedicate certain items constituting the Common Area Infrastructure such as the roadways, lift station, sewer connection, and outfall pipe to the City of Okeechobee ("City"), the County of Okeechobee ("County"), and/or other governmental authorities, respectively.

Applicant is hereby submitting Applicant's master site plan and preliminary plat for the Project in order for Applicant to obtain approval of said site plan, including multiple individual lot site plans and related special exception applications pertinent to Applicant or as jointly submitted by Applicant and a third-party future owner / user of a specific lot within the Project ("Co-Applicant," and as further described below, if applicable), and plat from all governing authorities, including approval of the vacation of any existing recorded plat ("Site Plan Approval"). Subject to Applicant obtaining Site Plan Approval, Applicant will enter into a development agreement with the City which shall grant Applicant all rights necessary to develop the Project. More specifically, Applicant desires to enter into separate development agreements with the $C^{\dagger t}$ for the following lots within the Project in order to develop said lot for the uses described below:

Express Car Wash Facility Lot 1: .

Page 2 Park Street Commerce Center - Statement of Interest in Property

Lot 2: Culver's Restaurant with Drive-Thru •

Applicant sincerely looks forward to working with the City to develop this exciting project and making a large investment in the City's future and that of its residents, local businesses, and others in the surrounding community. Please direct all questions regarding the Project to Scott Winch at swinch@kinghux.com and/or (386) 527-6729.

Very truly yours,

PARK STREET OKEECHOBEE, LLC By: Adam Ramsay, Manager

Cc: Gary Ritter, City Administrator (via email)

Prepared By and Return to John D. Cassels, Jr. Esq. Cassels & McCall P.O. Box 968 Okeechobee, Florida 34973

Parcel ID Numbers: 2-15-37-35-0A00-00009-A000 2-15-37-35-0A00-00009-0000 2-15-37-35-0A00-00011-0000 2-15-37-35-0A00-00010-0000 3-15-37-35-0210-00010-0110 3-15-37-35-0210-00010-0040 3-16-37-35-0210-00010-0010 FILE NUM 2006011065 OR BK 00603 PG 1345 SHARON ROBERTSON, CLERK OF CIRCUIT COURT OKEECHOBEE COUNTY, FL RECORDED 06/20/2006 04:34:38 PM RECORDING FEES 18.50 DEED DOC 18,433.80 RECORDED BY R Parrish Pss 1345 - 1346; (2pss)

WARRANTY DEED

THIS WARRANTY DEED made this day of June, 2006, between **GREAT LAKES HOLDINGS, LLC**, a **Florida limited liability company**, whose mailing address is 410 SE 2nd Avenue, Okeechobee, FL 34974, hereinafter called the **GRANTOR**, to **WILLIAM R. GRIGSBY**, JR., whose mailing address 518 Bear Road, Lake Placid, FL 33852, hereinafter called the **GRANTEE**:

(Wherever used herein, the terms "Grantors" and "Grantees" include all the parties to this instrument, and the heirs, legal representatives and assigns of individuals and the successors and assigns of corporations).

WITNESSETH:

That the **GRANTOR**, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the **GRANTEE**, all that certain land situate in OKEECHOBEE County, Florida, to-wit:

PARCEL 1 (PER O.R.B. 527, PGS. 869-870):

THE WEST HALF (W ½) OF THE EAST HALF (E ½) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) LYING NORTH OF STATE ROAD NO. 70 (FORT PIERCE ROAD), IN SECTION 15, TOWNSHIP 37 SOUTH, RANGE 35 EAST, LESS THE FOLLOWING:

BEGINNING AT THE SOUTHWEST CORNER OF THE EAST HALF (E ½) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF SAID SECTION 15, RUN NORTH 594 FEET, EAST 186.3 FEET, SOUTH 594 FEET; WEST 186.3 FEET TO THE POINT OF BEGINNING.

ALSO LESS THE NORTH 50 FEET WHICH IS RESERVED FOR ROAD PURPOSES.

ALSO LESS: A STRIP OF LAND 7 FEET WIDE SITUATE ADJACENT TO AND NORTHERLY OF THE EXISTING 66 FOOT RIGHT OF WAY OF STATE ROAD 70, LYING, WITHIN THE WEST ½ OF THE EAST ½ OF SW 1/4 OF SE 1/4, SECTION 15, TOWNSHIP 37 SOUTH, RANGE 35 EAST, LESS THE WEST 186.3 FEET THEREOF; CONTAINING .02 OF AN ACRE, MORE OR LESS.

PARCEL 2 (PER O.R.B. 528, PGS. 1342-1343):

ALL OF LOTS 1 THROUGH 12, INCLUSIVE, LYING NORTH OF NORTH PARK STREET (A/K/A S.R. 70 F/K/A FORT PIERCE ROAD) AS NOW CONSTRUCTED, IN BLOCK 4, PRICE ADDITION TO OKEECHOBEE CITY, ACCORDING TO THE PLAT THEREOF RECORDED IN PLAT BOOK 2, PAGE 17, PUBLIC RECORDS OF OKEECHOBEE COUNTY, FLORIDA.

PARCEL 3 (PER O.R.B. 528, PGS. 1342-1343):

COMMENCE AT THE NORTHEAST CORNER OF THE SOUTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SECTION 15, TOWNSHIP 37 SOUTH, RANGE 35 EAST, THENCE RUN SOUTH 00°18'26" EAST ALONG THE EASTERLY LINE THEREOF, 668.71 FEET TO THE POINT OF BEGINNING: THENCE CONTINUE SOUTH 00°18'26" EAST, 276.28 FEET TO THE NORTHEAST CORNER OF PRICE ADDITION TO OKEECHOBEE CITY ACCORDING TO THE PLAT THEREOF RECORDED IN PLAT BOOK 2, PAGE 17; THENCE RUN SOUTH 89°50'34" WEST, ALONG THE NORTHERLY LINE THEREOF, 336.76 FEET TO THE NORTHWEST CORNER OF SAID PRICE ADDITION; THENCE RUN NORTH 00°17'46" WEST, ALONG THE WEST LINE OF THE EAST ONE-HALF OF THE SOUTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 15, A DISTANCE OF 273.38 FEET; THENCE RUN NORTH 89°20'47" EAST AND PARALLEL WITH THE NORTHERLY LINE OF PREVIOUSLY MENTIONED SOUTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER, A DISTANCE OF 336.71 FEET TO THE POINT OF BEGINNING.

[3593-68289.WPD]

PARCEL 4 (PER O.R.B. 528, PGS. 1342-1343):

THE ALLEY IN BLOCK 4, PRICE ADDITION TO OKEECHOBEE CITY, ACCORDING TO THE PLAT THEREOF RECORDED IN PLAT BOOK 2, PAGE 17, PUBLIC RECORDS OF OKEECHOBEE COUNTY, FLORIDA, LYING BETWEEN LOTS 1-6 AND 7-12.

PARCEL 5 (PER O.R.B 528, PGS 1342-1343:

THE STREET KNOWN AS NORTHEAST 2ND STREET (F/K/A CENTER STREET) AS IT RUNS EAST FROM NORTHEAST 12TH AVENUE TO NORTHEAST 13TH AVENUE, PARTICULARLY LOCATED NORTH OF BLOCK 4, PRICE ADDITION TO OKEECHOBEE CITY, ACCORDING TO THE PLAT THEREOF RECORDED IN PLAT BOOK 2, PAGE 17, PUBLIC RECORDS OF OKEECHOBEE COUNTY, FLORIDA.

PARCEL 6 (PER O.R.B 554, PG 338):

ALL THAT PART OF THE NORTH 668.71 FEET OF THE E ½ OF THE E ½ OF SW 1/4 OF SE 1/4 OF SECTION 15, TOWNSHIP 37 SOUTH, RANGE 35 EAST, LESS AND EXCEPT THE NORTH 50 FEET THEREOF.

Subject to those easements in favor of Florida Power and Light recorded in O.R. Book 109, Page 983 and O.R. Book 23, Page 524 and matters contained on the Plat of Price Addition to Okeechobee City recorded in Plat Book 2, Page 17, all being in the Public Records of Okeechobee County, Florida.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

* Singular and plural are interchangeable as context requires.

IN WITNESS WHEREOF, the said GRANTOR have executed this Deed the day and year first above written.

Signed, Sealed and Delivered in our presence:

UIIA YMM DI Name' (Signature) Ta Tak (Print Name)

GREAT LAKES HOLDINGS, LLC, a Florida limited iabiling company

D. Robert Willson, Managing member

STATE OF FLORIDA COUNTY OF OKEECHOBEE

Signed and sworn to (or affirmed) before me this day of June, 2006, by D. ROBERT WILLSON, as Managing Member of GREAT LAKES HOLDINGS, LLC, a Florida limited liability company, who is personally known to me.

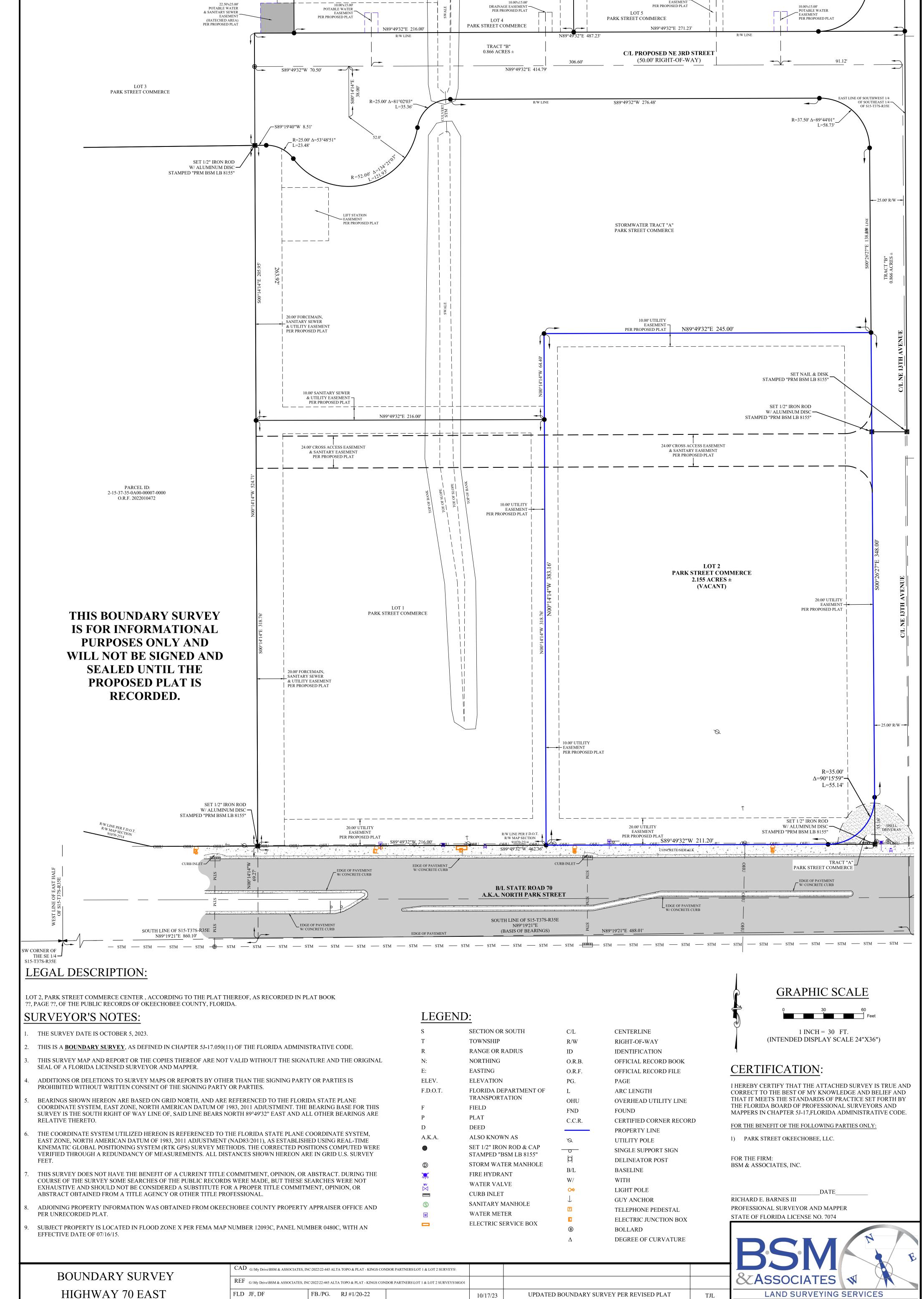


Karin Ammons My Commission DD211896 Expires July 11, 2007

NOŤAR PUB Print Name:

[3593-68289.WPD]





80 SE 31st Lane, Okeechobee, FL 34974

ricky.barnes@bsmsurvey.com

LB 8155

863.484.8324

G:\My Drive\BSM & ASSOCIATES, INC\2022\22-445 ALTA TOPO & PLAT - KINGS CONDOR PARTNERS\LOT 1 & LOT 2 SURVEYS\22-445 BND LOT 2.dwg 1 Oct 17, 2023;

OKEECHOBEE, FLORIDA 34972

OFF EGB

CKD REB

SHEET 1 OF 1

DATE 10/05/23

DWG₂₂₋₄₄₅ BND LOT 2

8/19/23

DATE:

UPDATED BOUNDARY SURVEY PER REVISED PLAT

REVISIONS:

TJL

BY:

Project Location ARCHITECT: **CIVIL ENGINEER:** WENDY MARTIN, AIA, NCARB JOSHUA COCKRIEL, P.E KIMLEY-HORN AND ASSOCIATES, INC. OLLMANN ERNEST MARTIN ARCHITECTS 12740 GRAN BAY PARKWAY WEST, SUITE 2350 200 S STATE ST. JACKSONVILLE, FLORIDA 32258 BELVIDERE, IL 61008 PHONE (904) 828-3900 PHONE (815) 544-7790

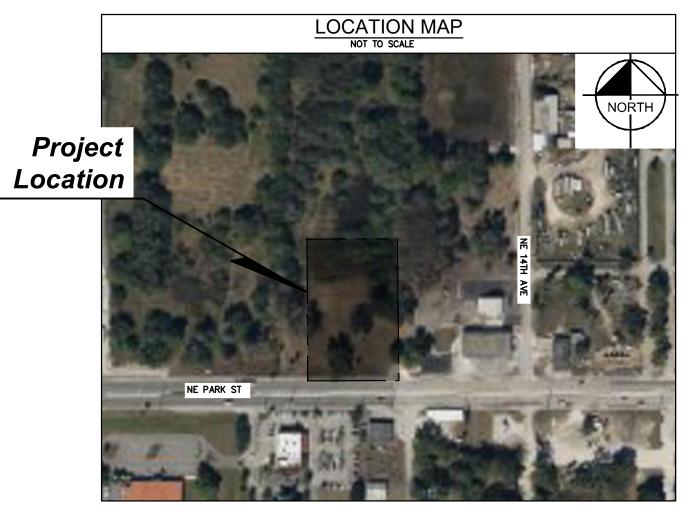
FINAL SITE PLAN/CONSTRUCTION PLANS CULVER'S OKEECHOBEE

975 NE PARK ST

FOR S&L PROPERTIES OKEECHOBEE, LLC



LOCATED IN OKEECHOBEE, FLORIDA NOV. 2023



Civil Sheet L

SHEET	NUMBER	SHEET TITLE
C-01		COVER SHEET
C-02		SIGNATURE SH
C-03		GENERAL NOTE
C-04		UTILITY NOTES
C-05		EXISTING CONE
C-06		OVERALL SITE
C-07		PAVING AND G
C-08		GRADING AND
C-09		UTILITY PLAN
C-10		SITE DETAILS
C-11		SITE DETAILS
C-12		DRAINAGE DET
C-13		BEST MANAGE
C-14		STORM WATER

SURVEYOR: BSM & ASSOCIATES, INC 80 SE 31ST LANE OKEECHOBEE, FL 34974 (863) 484 8324

CONTRACTOR: McCON BUILDING CORPORATION 1209 JOSEPH ST. DODGEVILLE, WI 53533 PHONE (608) 929-7737

GEOTECH: KSM ENGINEERING AND TESTING 11345 US HIGHWAY 1 SEBASTIAN, FL 32771 PHONE (772) 589 0712

OWNER: JEFF LIEGEL S&L PROPERTIES OKEECHOBEE, LLC 2651 KIRKING CT. PORTAGE, WI 58901 PHONE (608) 742-2898

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HEET TES IDITIONS AND DEMOLITION PLAN PLAN GEOMETRY PLAN DRAINAGE PLAN

TAILS EMENT PRACTICES POLLUTION PREVENTION PLAN

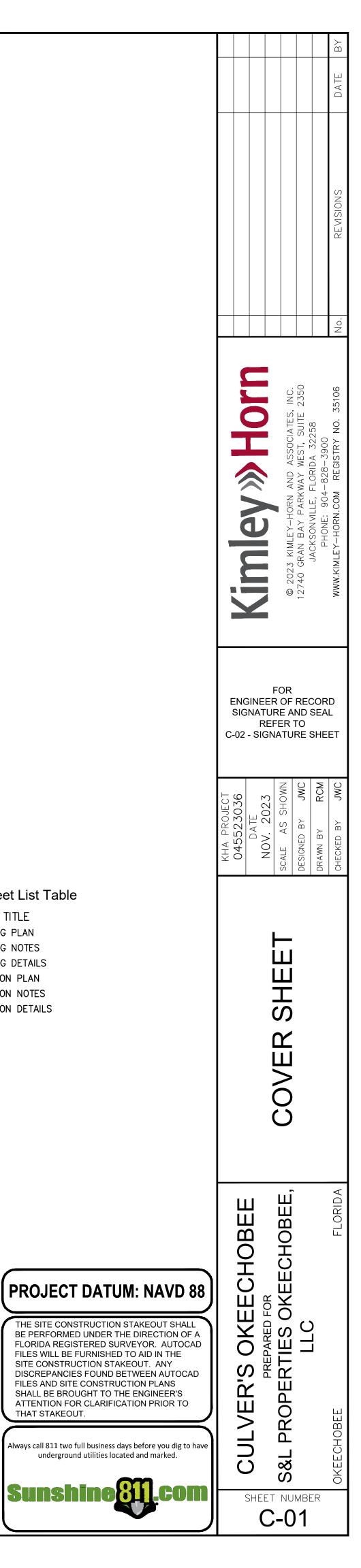
Landscape Architect Sheet List Table

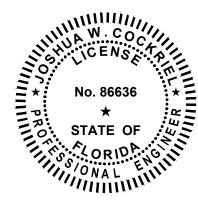
SHEET NUMBER LP-100 LP-150 LP-151 IR-100 IR-150 IR-151

SHEET TITLE PLANITNG PLAN PLANTING NOTES PLANTING DETAILS IRRIGATION PLAN IRRIGATION NOTES IRRIGATION DETAILS

THAT STAKEOUT.

Sunshine





Kimley-Horn and Associates, Inc. 12740 GRAN BAY PARKWAY WEST, SUITE 2350

JACKSONVILLE, FLORIDA 32258 CERTIFICATE OF AUTHORIZATION: 696 JOSHUA COCKRIEL, P.E. 86636

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JOSHUA COCKRIEL, P.E. ON THE DATE ADJACENT TO THE SEAL. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

Civil Sheet List Table

SHEET	NUMBER	SHEET TITLE
C-01		COVER SHEET
C-02		SIGNATURE SHEET
C-03		GENERAL NOTES
C-04		UTILITY NOTES
C-05		EXISTING CONDITIONS AND
C-06		OVERALL SITE PLAN
C-07		PAVING AND GEOMETRY PI
C-08		GRADING AND DRAINAGE F
C-09		UTILITY PLAN
C-10		SITE DETAILS
C—11		SITE DETAILS
C-12		DRAINAGE DETAILS
C-13		BEST MANAGEMENT PRACT
C-14		STORM WATER POLLUTION

D DEMOLITION PLAN

PLAN PLAN

CTICES N PREVENTION PLAN



Kimley-Horn and Associates, Inc. 12740 GRAN BAY PARKWAY WEST, SUITE 2350 JACKSONVILLE, FLORIDA 32258 CERTIFICATE OF AUTHORIZATION: REGISTRY NO. 35106 MATTHEW A. JURNEY No. LA6666993

THIS DOCUMENT HAS BEEN DIGITALLY SIGNED AND SEALED BY MATTHEW A. JURNEY ON THIS DATE ADJACENT TO THE SEAL. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

THE ABOVE NAMED PROFESSIONAL LANDSCAPE ARCHITECT SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

Landscape Architect Sheet List Table

SHEET NUMBER SHEET TITLE PLANITNG PLAN LP-100 PLANTING NOTES LP-150 LP-151 PLANTING DETAILS IR-100 IRRIGATION PLAN IR-150 IRRIGATION NOTES IR-151 IRRIGATION DETAILS

					No. REVISIONS DATE BY
	Nimiey »> Horn	© 2023 KIMLEY-HORN AND ASSOCIATES INC	12740 GRAN BAY PARKWAY WEST, SUITE 2350	JACKSONVILLE, FLORIDA 32258 PHONE: 904-828-3900	WWW.KIMLEY-HORN.COM REGISTRY NO. 35106
КНА РROJECT 045523036	DATE NOV. 2023		DESIGNED BY JWC	N BY RCM	снескер ву ЈМС
КH, 40		SIGNATURE SHEET SCALE		DRAWN	CHECK
		S&L PROPE			OKEECHOBEE FLORIDA

GENERAL NOTES:

- 1. ALL WORK AND MATERIALS SHALL BE IN COMPLETE ACCORDANCE WITH ALL RELATIVE CITY OF OKEECHOBEE, FLORIDA DEPARTMENT OF TRANSPORTATION (FDOT) AND THE CITY OF OKEECHOBEE UTILITY DEPARTMENT STANDARDS, SPECIFICATIONS, AND REQUIREMENTS.
- 2. ELEVATIONS ARE BASED ON NAVD 1988 DATUM. SURVEYS PROVIDED BY BSM & ASSOCIATES, INC, DATED NOVEMBER 11,
- 3. THE CONTRACTOR IS RESPONSIBLE FOR INSPECTING AND BECOMING FAMILIAR WITH THE SITE PRIOR TO CONSTRUCTION 2. PROTECTION OF BUILDINGS & EQUIPMENT TEMPORARY PROTECTIVE DEVICES, AS REQUIRED SHALL BE INSTALLED ADJACENT TO THE DEMOLITION WORK FOR AND BIDDING. PROTECTION OR PERSONNEL, EXISTING ADJACENT BUILDINGS, STRUCTURES AND EQUIPMENT AGAINST DUST, FALLING OR FLYING DEBRIS. ANY DAMAGE TO EXISTING STRUCTURES, FACILITIES AND/OR EQUIPMENT RESULTING FROM DEMOLITION WORK SHALL BE REPAIRED BY THE CONTRACTOR AT NO COST TO THE OWNER.
- 4. THE CONTRACTOR SHALL ADHERE TO ALL NOTES PROVIDED IN THESE CONSTRUCTION DRAWINGS.
- 5. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PERMITTED CONSTRUCTION DOCUMENTS. ANY DEVIATION FROM THE APPROVED CONSTRUCTION DOCUMENTS SHALL BE THE SOLE RESPONSIBILITY OF THE ORGANIZATION AND/OR ENTITY 3. DISPOSAL OF EXISTING EQUIPMENT & DEBRIS ALL DEBRIS AND EXISTING MATERIALS AND EQUIPMENT SHALL BE HAULED AWAY AND DISPOSED OF BY THE CONTRACTOR. RESPONSIBLE FOR THE INSTALLATION TO UPDATE/REPLACE ANY DEFICIENT MATERIAL/EQUIPMENT NECESSARY TO BRING THE FINAL PRODUCT TO THE STANDARDS OF THE PERMITTED CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL MAKE HIS OWN ARRANGEMENTS FOR OBTAINING DISPOSAL AREAS. THE CONTRACTOR SHALL TAKE EVERY PRECAUTION TO PREVENT SPILLAGE OF MATERIALS BEING HAULED IN PUBLIC STREETS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO IMMEDIATELY CLEAN UP ANY SPILLAGE WHICH MAY ACCIDENTALLY OCCUR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL/DISPOSAL OF ANY UNSUITABLE MATERIAL FROM HIS OPERATION,
- FURNISHING AND COMPACTING SUITABLE REPLACEMENT BACKFILL MATERIAL IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL REGULATIONS.
- CLEANUP THE CONTRACTOR SHALL MAINTAIN AN ORDER OF NEATNESS AND GOOD HOUSEKEEPING. TOOLS, SCAFFOLDING AND OTHER DEMOLITION EQUIPMENT MUST AT ALL TIMES BE KEPT IN A NEAT AND ORDERLY ARRANGEMENT. AT THE 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS, INCLUDING RIGHT-OF-WAY PERMITS, CONCLUSION OF THE DEMOLITION OPERATIONS, THE ENTIRE WORK AREA SHALL BE LEFT IN A CLEAN CONDITION AS AND INSURANCE REQUIRED FOR THE PROJECT. REQUIRED FOR SUBSEQUENT NEW WORK.
- 8. THE LOCATION OF ALL UTILITIES SHOWN ON THE DRAWINGS ARE FROM INFORMATION PROVIDED BY THE SURVEYOR, THE CITY OF OKEECHOBEE PUBLIC WORKS DEPARTMENT AND LIMITED FIELD OBSERVATIONS.
- 9. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONFIRM. IN THE FIELD, THE LOCATIONS AND ELEVATIONS SHOWN PRIOR TO THE COMMENCEMENT OF CONSTRUCTION.
- 10. THE CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION AND BUILDING PLACEMENT WITH ALL OTHER UTILITIES CONSTRUCTION.
- 11. CONTRACTOR SHALL PROVIDE AND MAINTAIN ADEQUATE EROSION AND TURBIDITY CONTROLS DURING AND FOLLOWING CONSTRUCTION UNTIL ALL DISTURBED AREAS HAVE BEEN STABILIZED TO AVOID ADVERSE ENVIRONMENTAL IMPACTS TO OFF-SITE PROPERTY AND DRAINAGE SYSTEMS.
- 12. THE CONTRACTOR IS RESPONSIBLE FOR THE INSTALLATION AND MAINTENANCE OF SEDIMENT AND EROSION CONTROL MEASURES PRIOR TO CONSTRUCTION. ALL SEDIMENT AND EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH STORMWATER POLLUTION PREVENTION PLAN INCLUDED HEREIN.
- 13. ALL AREAS DISTURBED BY CONSTRUCTION SHALL BE SEEDED AND MULCHED OR SODDED IN ACCORDANCE WITH THESE CONSTRUCTION PLANS AND PROJECT SPECIFICATIONS.
- 14. ALL AREAS OF DISTURBED CITY, OR STATE RIGHT-OF-WAY SHALL BE SODDED.
- 15. THE CONTRACTOR SHALL PROVIDE A TEMPORARY WATER SERVICE OR WATER TRUCK FOR WASH-DOWN OF VEHICLES LEAVING THE PROJECT SITE IF NECESSARY.
- 16. THE CONTRACTOR(S) SHALL NOTIFY ALL APPLICABLE UTILITIES COMPANIES, THE ENGINEER OF RECORD, AND THE PROPERTY OWNER 48 HOURS PRIOR TO INITIATING ANY EXCAVATION ACTIVITIES, OR AS SPECIFIED BY THE UTILITIES COMPANIES AND THE PERMITS OBTAINED FOR THE WORK.
- 17. THE ENGINEER OF RECORD SHALL BE GIVEN FORTY EIGHT HOURS (48-HR) NOTICE OF ALL MEETINGS AND OR TESTING MEASURES RELATED TO SAID PROJECT.
- 18. THERE ARE TO BE NO OPEN TRENCHES AT THE END OF EACH WORK DAY.
- 19. THE TRENCH SAFETY SYSTEM SHALL BE COMPLIANT WITH OSHA STANDARD 29 CFR SECTION 1926.650 SUBPART P. THE CONTRACTOR SHALL PREPARE AND MAINTAIN DOCUMENTATION THAT THE TRENCH SAFETY SYSTEM IS IN COMPLIANCE WITH THE STATED OSHA STANDARD.
- 20. CONSTRUCTION WARNING SIGNS ARE TO BE MOUNTED AND ERECTED BEFORE CONSTRUCTION CAN COMMENCE. THESE AND ALL TRAFFIC CONTROL DEVICES SHALL FOLLOW THE STANDARDS SET FORTH BY THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AS WELL AS FLORIDA DEPARTMENT OF TRANSPORTATION (FDOT) STANDARD INDEX.
- 21. THE CONTRACTOR IS RESPONSIBLE FOR CLEARLY IDENTIFYING THE AREA OF CONSTRUCTION AND SAFELY ROUTING ALL VEHICULAR AND PEDESTRIAN TRAFFIC AROUND THE CONSTRUCTION AREA. THE CONSTRUCTION AREA SHALL BE CLEARLY MARKED AT ALL TIMES.
- 22. THE CONTRACTOR(S) SHALL LOCATE, VERIFY, AND IDENTIFY ALL EXISTING UNDERGROUND UTILITIES SHOWN OR NOT SHOWN ON THE PLANS PRIOR TO ANY EXCAVATING ACTIVITIES.
- 23. THE CONTRACTOR SHALL VERIFY ALL UTILITY LOCATIONS PRIOR TO EXCAVATION AND TAKE ALL MEASURES NECESSARY TO PROTECT UTILITIES DURING CONSTRUCTION. SHOULD ANY UTILITY LINE OR COMPONENT BECOME DAMAGED OR REQUIRE RELOCATION THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE RESPONSIBLE UTILITY COMPANY, THE ENGINEER OF RECORD, AND THE CITY OF OKEECHOBEE.
- 24. THE CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO EXISTING UTILITIES, SURVEY MONUMENTS, MARKERS, CORNERS, AND EXISTING FEATURES IN THE AREA. ANY DAMAGE SHALL BE REPLACED/REPAIRED BY THE CONTRACTOR AT NO ADDITIONAL EXPENSE TO THE OWNER.
- 25. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO EXISTING UTILITIES CAUSED BY HIS OPERATIONS.
- 26. A COPY OF THE CONTRACTORS GENERAL LICENSE AND/OR UNDERGROUND UTILITY LICENSE SHALL BE PROVIDED TO THE OWNER AT THE TIME OF THE PRE-CONSTRUCTION CONFERENCE.
- 27. GEOTECHNICAL REPORT AND BORINGS ARE PROVIDED BY KSM ENGINEEREING AND TESTING, DATED DECEMBER 30, 2022. REFER TO DRIVEWAY & PARKING LOTS FOR SITE PREPARATION RECOMMENDATIONS.
- 28. CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT FOR PROCEDURES IN PROPERLY PREPARING THE SITE FOR CONSTRUCTION OF FOUNDATIONS, PAVEMENT AND ALL ELEMENTS RELATED TO THIS PROJECT.
- 29. ANY MATERIALS CLASSIFIED BY THE GEOTECHNICAL REPORT AS UNSUITABLE SHALL NOT BE USED AS FILL SOIL UNLESS A METHOD OF MAKING THE MATERIAL SUITABLE IS APPROVED BY THE GEOTECHNICAL REPORT ENGINEER AND THE CLIENT.
- 30. ALL UNSUITABLE MATERIALS SHALL BE REMOVED FROM THE SITE IN ACCORDANCE WITH THE GEOTECHNICAL RECOMMENDATIONS. COSTS FOR EXCAVATION AND OFFSITE HAULING AND DISPOSAL SHALL BE BORNE BY THE CONTRACTOR.
- 31. TEMPORARY CONTROL OF GROUNDWATER SHOULD FOLLOW THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT
- 32. A CITY OF OKEECHOBEE INSPECTOR SHALL BE CONTACTED 24 HOURS PRIOR TO ALL NECESSARY SITE WORK INSPECTIONS AND 5 DAYS PRIOR TO THE FINAL INSPECTION.

DEMOLITION NOTES:

1. CODES ALL CODES REGULATING DEMOLITION WORK SHALL BE COMPLIED WITH. THE CONTRACTOR SHALL PUT UP AND MAINTAIN SUCH BARRIERS AND WARNING LIGHTS, AS MAY BE NECESSARY OR REQUIRED BY CODE, TO PROTECT AND PREVENT UNAUTHORIZED PERSONNEL FROM ENTERING THE DEMOLITION WORK AREA. ALL DEMOLITION OPERATIONS SHALL COMPLY WITH THE REQUIREMENTS OF THE FEDERAL OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA) INSOFAR AS THEY APPLY TO DEMOLITION WORK TO BE PERFORMED UNDER THIS CONTRACT.

PAVING AND DRAINAGE NOTES:

- 1. ALL GRADING, PLACEMENT OF FILL, AND COMPACTION SHALL BE IN ACCORDANCE WITH THE CITY OF OKEECHOBEE STANDARD SPECIFICATIONS AND THE FLORIDA DEPARTMENT OF TRANSPORTATION (FDOT) STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST EDITION. IN THE EVENT OF A STANDARD CONFLICT, THE CITY OF OKEECHOBEE SPECIFICATIONS AND STANDARDS SHALL HAVE PRIORITY.
- 2. THE CONTRACTOR SHALL CONSTRUCT ALL DRAINAGE STRUCTURES TO THE DESIGN ELEVATIONS SHOWN AND IN COMPLIANCE WITH TYPICAL CONSTRUCTION DETAILS.
- 3. ALL PIPE LENGTHS SHOWN HEREIN ARE APPROXIMATE LENGTHS FROM CENTER TO CENTER OF THE RELATED STRUCTURES.
- 4. ALL PIPE LENGTHS ARE SCALED AND MAY REQUIRE SLIGHT FIELD ADJUSTMENTS TO FIT CONDITIONS. ALL PIPE CROSSINGS SHALL BE COMPACTED TO 95% OF THE MODIFIED PROCTOR MAX. DRY DENSITY (ASTM D1557) AT 1' LIFTS.
- 5. THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION OF THE PAVING AND DRAINAGE FACILITIES WITH ALL OTHER CONSTRUCTION. WATER AND SANITARY SEWER ARE PROVIDED AS PART OF THESE CONSTRUCTION DOCUMENTS.
- 7. THE CONTRACTOR SHALL COORDINATE ALL NOTIFICATIONS AND UTILITY LOCATION EFFORTS WITH THE UTILITY OWNERS PRIOR TO THE COMMENCEMENT OF CONSTRUCTION.

- 10. LIMEROCK BEARING RATIOS FOR SUBGRADE AT FORTY (40) AND LIMEROCK OR ALTERNATIVE BASE COURSE AT ONE HUNDRED (100). THERE WILL BE NO UNDER TOLERANCE. CRUSHED CONCRETE IS AN ACCEPTABLE ALTERNATIVE TO LIMEROCK, ONLY IF IT MEETS THE SPECIFICATIONS DEFINED IN THE GEOTECHNICAL REPORT AS "BASE COURSE" AND AS APPROVED BY THE ENGINEER OF RECORD.
- 11. ALL MATERIAL USED FOR BACKFILL SHALL BE CLASS A3 FREE DRAINING SAND.
- 12. ALL CONSTRUCTION LINES & GRADES SHALL BE ESTABLISHED AND MAINTAINED BY THE CONTRACTOR
- 13. CONTRACTOR WILL CLEAR, GRUB AND DISPOSE OF ALL DEBRIS AND SURFACE ORGANICS TO FIRM MATERIAL IN ALL EASEMENTS, ROAD RIGHT-OF-WAYS AND DETENTION AREAS. DISPOSAL SHALL BE INCLUDED IN THE CONTRACT.
- 14. CONTRACTOR SHALL BE RESPONSIBLE FOR THE PREVENTION OF DOWNSTREAM TURBIDITY/ SILTATION THROUGH THE USE OF HAY BALES, SCREENS, SILTATION BASINS, CHEMICAL FLOCCULATION AND/OR ANY OTHER SUITABLE MEANS REQUIRED TO MEET FLORIDA STREAM STANDARDS. SEED AND MULCH ALL DISTURBED AREAS, SOD AS REQUIRED TO CONTROL EROSION THROUGH FINAL INSPECTION AND TO PRODUCE A UNIFORM STAND OF GRASS THROUGHOUT.
- 15. CONTRACTOR SHALL BE RESPONSIBLE FOR BRICKING UP CURB INLETS TO FINISHED GRADE AND FURNISHING AND MAINTAINING ALL HARDWARE.
- 16. CONTRACTOR SHALL BE RESPONSIBLE FOR CONSTRUCTING CURB TRANSITIONS TO FINISH GRADE.
- 17. UNDERDRAINS SHALL BE INSTALLED IN ALL CASES WHERE THE SEASONAL HIGH WATER TABLE IS CLOSER THAN 12 INCHES BELOW THE PROPOSED SUBGRADE OF ANY ROAD, REGARDLESS OF PLANS. UNDERDRAINS SHALL BE STUBBED OUT 20' FROM ALL CURB INLETS FOR FUTURE POSSIBLE USE.
- 18. FILTER WRAP ALL DRAINAGE JOINTS TO CITY OF OKEECHOBEE SPECIFICATIONS.

- 21. THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTING ALL ADA PARKING SPACES AND ACCESSIBLE ROUTES, PURSUANT TO THE FLORIDA ACCESSIBILITY CODE FOR BUILDING CONSTRUCTION, LATEST EDITION IN EFFECT AT THE TIME THE PROJECT WAS APPROVED BY THE CITY OF OKEECHOBEE. IF THERE ARE DISCREPANCIES IN THE CONSTRUCTION DRAWINGS REGARDING GRADES ON SLOPES COMPARED TO ADA REQUIREMENTS, CONTRACTOR SHALL NOTIFY ENGINEER FOR CLARIFICATION, PRIOR TO IMPLEMENTATION. CONTRACTOR WILL BE RESPONSIBLE FOR ANY PAVEMENT, PARKING STALLS, SIDEWALKS, OR OTHER ADA ELEMENTS ASSOCIATED WITH THIS PROJECT THAT DOES NOT CONFORM TO ADA GUIDELINES AND THE INTENT OF THESE PLANS.
- 22. ALL ROADWAYS, DRIVEWAYS, AND BUILDING PAD LOCATIONS SHOULD BE PROOF ROLLED PRIOR TO IMPORTATION OF ADDITIONAL FILL OR SUBBASE MATERIAL (LIMEROCK). IN THE EVENT THAT PROPOSED COMPACTION AND DENSITIES CANNOT BE ACHIEVED, CONTRACTOR SHALL NOTIFY ENGINEER TO AID IN DETERMINING LIMITS OF UNSUITABLE MATERIALS.
- SIGNAGE NOTES:
- 1. ALL SIGNS WITHIN CITY OF OKEECHOBEE RIGHT OF WAYS MUST MEET THE CITY OF OKEECHOBEE STANDARDS.

- 1. PAVEMENT MARKINGS SHOULD BE PLACED AS SHOWN ON THE PLANS AND DETAIL SHEETS.
- 2. ANY REQUIRED TEMPORARY MARKINGS MUST BE IN PLACE BEFORE OPENING LANES OF TRAFFIC. PAY ITEMS FOR TEMPORARY PAVEMENT MARKINGS ARE TO BE INCLUDED IN THE CONTRACT.
- 3. THE REMOVAL OF EXISTING PAVEMENT MARKINGS WILL BE CONSIDERED AN INCIDENTAL ITEM WITH NO ADDITIONAL COMPENSATION PROVIDED.
- 4. ALL PERMANENT PAVEMENT MARKINGS SHALL MEET CURRENT CITY OF OKEECHOBEE SPECIFICATIONS AND/OR FDOT STANDARD SPECIFICATIONS, LATEST EDITION.
- 5. THERMOPLASTIC PAVEMENT MARKINGS ARE TO BE PLACED NO SOONER THAN 30 CALENDAR DAYS AFTER THE COMPLETION OF THE FINAL PAVEMENT LAYER.

- 7. THE CONTRACTOR SHALL USE CLASS B REFLECTIVE PAVEMENT MARKERS (RPMS) INSTALLED TO MEET CURRENT CITY OF OKEECHOBEE SPECIFICATIONS AND/OR FDOT STANDARD SPECIFICATIONS.
- 8. REFLECTIVE PAVEMENT MARKERS THAT DO NOT CONFLICT WITH PERMANENT (THERMOPLASTIC) MARKINGS SHALL BE PLACED ON ALL FINAL ASPHALTIC CONCRETE SURFACES IMMEDIATELY AFTER THE TEMPORARY PERMANENT STRIPING IS IN PLACE.
- 9. PAVEMENT MARKINGS REMOVAL; a. PAINT BLACKOUT METHOD OF PAVEMENT MARKINGS REMOVAL IS NOT ACCEPTABLE. b. GRINDING OR HYDRO BLAST METHODS SHALL BE USED ON WEATHERED ASPHALT SURFACES. C. REMOVAL ON NEW ASPHALT SURFACES SHALL BE BY HYDRO BLAST METHOD ONLY.
- 10. IN THE EVENT OF A CONFLICT BETWEEN THE SPECIFICATIONS OF THE CITY OF OKEECHOBEE AND THE SPECIFICATIONS OF THE FDOT, THE CITY OF OKEECHOBEE WILL PREVAIL.

- 6. ALL PIPE JOINTS SHALL BE PROPERLY FITTED AND SEALED PER PRODUCT MANUFACTURERS SPECIFICATIONS.
- 8. THE CONTRACTOR IS RESPONSIBLE FOR THE INSTALLATION AND MAINTENANCE OF SEDIMENT AND EROSION CONTROL MEASURES DURING CONSTRUCTION. REFER TO THE STORMWATER POLLUTION PREVENTION PLAN.
- 9. COMPACTION DENSITIES FOR ALL ROADWAY CROSSINGS ARE TO BE TAKEN IN ONE-FOOT (1') LIFTS.

- 19. A 2' STRIP OF SOD IS REQUIRED AROUND ALL DITCH BOTTOM INLETS.
- 20. ALL POND SIDESLOPES SHALL BE STABILIZED WITH ESTABLISHED GRASS AT TIME OF FINAL INSPECTION.

SIGNAGE AND PAVEMENT MARKING NOTES:

6. A BITUMINOUS REFLECTIVE PAVEMENT MARKER (RPM) ADHESIVE MEETING CURRENT CITY OF OKEECHOBEE AND/OR FDOT SPECIFICATIONS SHALL BE USED ON ASPHALT ROADWAYS.

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Vindani		© 2023 KIMLEY-HORN AND ASSOCI	12740 GRAN BAY PARKWAY WEST, 3	JACKSUNVILLE, FLURIJA 32238 PHONE: 904-828-3900	WWW.KIMLEY-HORN.COM REGISTRY
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		GENERAL NOTES			
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UTILITY NOTES:

- 1. ALL WATER, SEWER, AND REUSE WORK MUST BE PERFORMED IN ACCORDANCE WITH CITY OF OKEECHOBEE UTILITY DEPARTMENT WATER AND SEWER STANDARDS, DETAILS, AND SPECIFICATIONS AS WELL AS ALL APPLICABLE STATE AND LOCAL REGULATIONS.
- 2. ALL EQUIPMENT AND MATERIALS SHALL COMPLY WITH THE CITY OF OKEECHOBEE UTILITY DEPARTMENT STANDARDS AND SPECIFICATIONS.
- 3. WHERE IT IS NOT POSSIBLE FOR WATER AND SEWER (INCLUDING STORM) LINES TO CROSS WITH A MINIMUM OF 18 INCHES OF VERTICAL CLEARANCE, A FULL-UNCUT LENGTH OF WATER QUALITY PIPE (i.e. DR 18 AWWA C-900 FOR NEWLY INSTALLED SEWER & DR 25 AWWA C-900 WATER) WHICH IS USUALLY 20 FEET LONG WILL BE CENTERED ON THE POINT OF CROSSING. THE CONTRACTOR WILL FIELD VERIFY THE VERTICAL SEPARATION. THE MINIMUM VERTICAL SEPARATION BETWEEN WATER AND SEWER (INCLUDING STORM) PIPES WHEN 18 INCHES IS NOT POSSIBLE WILL BE 6 INCHES OUTSIDE DIAMETER TO OUTSIDE DIAMETER. IT IS PREFERABLE TO HAVE THE WATER MAIN ABOVE THE SEWER LINES AND AT LEAST 18 INCHES VERTICAL SEPARATION.
- 4. A FULL UNCUT LENGTH OF WATER MAIN PIPE (USUALLY 20 FEET) SHALL BE CENTERED AT THE POINT OF CROSSING OF ALL WATER AND SEWER (INCLUDING STORM) LINES AT THE POINT OF CROSSINGS REGARDLESS OF THE VERTICAL SEPARATIONS.
- 5. IN THE CASE WHERE SOLVENT CONTAMINATION IS FOUND IN TRENCH, WORK WILL BE STOPPED AND THE PROPER AUTHORITIES NOTIFIED. WITH THE APPROVAL OF THE CITY OF OKEECHOBEE HEALTH DEPARTMENT, DUCTILE IRON PIPE, FITTINGS AND APPROVED SOLVENT RESISTANT GASKET MATERIAL SHALL BE USED IN THE CONTAMINATE AREA. THE DUCTILE IRON PIPE WILL EXTEND AT LEAST 100 FEET BEYOND ANY DISCOVERED SOLVENT.
- 6. IN REGARD TO THE REQUEST FOR A LETTER OF RELEASE TO PLACE THE CONSTRUCTION INTO SERVICE, THE BACTERIOLOGICAL SAMPLE POINTS SHALL BE INDICATED IN RED OR PINK ON THE RECORD OR AS BUILT DRAWINGS. THE SAMPLE NUMBERS WILL CORRESPOND TO THOSE ON THE BACTERIOLOGICAL SAMPLE LAB SHEETS.
- THE RECORD OR "AS BUILT" DRAWINGS TO BE PREPARED BY THE CONTRACTOR AND SUBMITTED AT THE TIME OF THE REQUEST FOR A LETTER OF RELEASE TO PLACE THE CONSTRUCTION INTO SERVICE WILL CLEARLY DEPICT THE VERTICAL CLEARANCES BETWEEN WATER AND SEWER (INCLUDING STORM) LINES AT ALL CROSSING AND PARALLEL RUNS WHERE THE HORIZONTAL SEPARATION IS LESS THAN 10 FEET. IN ADDITION, THE CENTERING OF UNCUT LENGTHS OF PIPE (USUALLY 20 FEET) AT POINTS OF CROSSINGS WILL BE DOCUMENTED ON THE DRAWINGS AND ALL MITIGATING CONSTRUCTION MEASURES CLEARLY DEPICTED IN CASES WHERE A MINIMUM OF 18 INCHES OF VERTICAL CLEARANCE BETWEEN THE WATER AND SEWER (INCLUDING STORM) LINES IS NOT POSSIBLE.
- EXISTING UTILITIES SHOWN ON THESE PLANS HAVE BEEN LOCATED PER THE BEST MEANS AVAILABLE WITHOUT EXCAVATION. THE CONTRACTOR SHALL LOCATE ALL EXISTING UTILITIES PRIOR TO COMMENCEMENT OF CONSTRUCTION.
- VERTICAL LOCATIONS OF ALL UTILITIES (EXCLUDING EXISTING STORM SEWERS) SHOWN ON PLAN AND 9. PROFILE SHEETS HAVE BEEN ASSUMED. CONTRACTOR SHALL EXERCISE CAUTION DURING EXCAVATION NEAR EXISTING UTILITIES SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF LOCATION DIFFERS FROM THAT SHOWN ON THE PLANS BEFORE CONTINUING WITH CONSTRUCTION.
- 10. SHOULD CONDITIONS VARY FROM THOSE SHOWN ON THESE PLANS THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER PRIOR TO CONTINUING CONSTRUCTION.
- 11. ALL WATER AND SEWER CONSTRUCTION SHALL BE ACCOMPLISHED BY AN UNDERGROUND UTILITY CONTRACTOR LICENSED UNDER THE PROVISIONS OR CHAPTER 489, FLORIDA STATUTES.
- 12. ALL UNDERGROUND UTILITIES TO BE INSTALLED SHALL BE IN ACCORDANCE WITH THE CITY OF OKEECHOBEE UTILITY DEPARTMENT SPECIFICATIONS AND THE APPROVED SITE PLANS.
- 13. ALL WATERMAINS AND FORCEMAINS SHALL HAVE A MINIMUM OF 36" COVER FROM FINISHED GRADE. ALL UTILITY PIPELINES ARE DESIGNED TO FINISHED GRADE AND SHALL BE PROTECTED FROM DAMAGE UNTIL WORK IS COMPLETED.
- 14. UNSUITABLE MATERIALS UNDER WATER AND SEWER MAINS SHALL BE REMOVED AND REPLACED WITH SELECTED BACKFILL PROPERLY COMPACTED TO 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557).
- 15. ALL CURB STOPS ARE TO BE FORD BALL-TYPE WITH LOCKING CAPACITY. 1" MINIMUM.
- 16. NO CONNECTION TO EXISTING POTABLE WATER SYSTEM SHALL BE ALLOWED UNTIL ALL PROPOSED WATER LINES HAVE BEEN PRESSURE TESTED, DISINFECTED, CLEARED FOR SERVICE AND ACCEPTED FOR MAINTENANCE BY CITY OF OKEECHOBEE UTILITY DEPARTMENT AND FDEP.
- 17. CONTRACTOR IS RESPONSIBLE FOR PROPER NOTIFICATION OF INSPECTING AUTHORITIES BEFORE AND DURING CONSTRUCTION.
- 18. MECHANICAL RESTRAINTS ARE REQUIRED WHERE WATER MAINS AND FORCE MAINS ARE TERMINATED AND AT ALL BENDS. ANY VERTICAL CONFLICT CROSSING USING FITTING SHALL BE RODDED THROUGHOUT THE CROSSING. WHERE FORCE MAINS OR WATER MAINS ARE LAID WITHOUT FITTINGS, THE MAXIMUM DEFLECTION SHALL BE AS RECOMMENDED BY THE MANUFACTURER OF THE PIPE USED.
- 19. FITTINGS SHALL BE USED AT LOCATIONS INDICATED ON THE PLANS, UNLESS OTHERWISE APPROVED BY THE ENGINEER.
- 20. ALL UNDERGROUND VALVES SHALL BE INSTALLED WITH AN ADJUSTABLE CAST IRON VALVE BOX WITH TOP SET TO FINAL GRADE IN ACCORDANCE WITH THE CITY OF OKEECHOBEE UTILITY DEPARTMENT DETAILS AND SPECIFICATIONS.

NOTICE OF PROCEDURE: 21. ALL COMMERCIAL BUILDING PERMITS AND METERS TO BE PROCESSED THROUGH THE CITY OF OKEECHOBEE UTILITY DEPARTMENT WATER AND SEWER CUSTOMER SERVICE SHALL BE ACCOMPANIED BY A SET OF APPROVED CIVIL DESIGN PLANS.

- 22. ALL WATER AND SEWER TAPS TO BE PERFORMED BY UTILITY CONTRACTOR OR LICENSED MASTER PLUMBER MUST BE SCHEDULED AT LEAST 48 HOURS IN ADVANCE THROUGH YOUR CITY OF OKEECHOBEE UTILITY DEPARTMENT INSPECTOR. IN ADDITION, IT IS THE ENGINEER OF RECORD'S RESPONSIBILITY TO SECURE APPLICABLE D.E.P./CITY OF OKEECHOBEE UTILITY DEPARTMENT PERMITS IN ACCORDANCE WITH THE CITY OF OKEECHOBEE UTILITY DEPARTMENT PERMITTING PROCEDURES.
- 23. METERS TO BE INSTALLED BY THE CITY OF OKEECHOBEE UTILITY DEPARTMENT FORCES UPON APPLICATION AND PAYMENT BY LICENSED MASTER PLUMBER.
- 24. WATER AND SEWER CAPACITY FEES SHALL BE REQUIRED AT TIME OF BUILDING PERMIT APPLICATION. FEES WILL BE BASED ON TOTAL NUMBER OF PLUMBING FIXTURE UNITS SHOWN OR LISTED ON BUILDING PLANS.
- 25. A PRE-CONSTRUCTION CONFERENCE IS REQUIRED AND SHALL BE SCHEDULED 48 HOURS IN ADVANCE WITH THE CITY OF OKEECHOBEE UTILITY DEPARTMENT.
- 26. ALL ON-SITE PRIVATE WATER AND SEWER CONSTRUCTION REQUIRES A PLUMBING PERMIT WITH SCHEDULED INSPECTION PRIOR TO INSTALLATION - CONTACT CITY CHIEF PLUMBING INSPECTOR.

POTABLE WATER SYSTEM NOTES:

- 1. NOTIFY THE CITY OF OKEECHOBEE UTILITY DEPARTMENT 48 HRS. IN ADVANCE OF MAKING THE CONNECTION TO THE EXISTING WATER MAIN STUB. A CITY OF OKEECHOBEE UTILITY DEPARTMENT INSPECTOR SHALL BE PRESENT AT TIME CONNECTION IS MADE.
- 2. THE CONTRACTOR SHALL FIELD VERIFY THE LOCATIONS OF ALL EXISTING UTILITIES AND CONNECTION POINTS PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER OF RECORD IMMEDIATELY.
- 3. ALL WATER MAINS 4" THRU 12" DIAMETER SHALL BE PVC 1120-CLASS 150-DR-18 WITH PUSH-ON GASKETED JOINTS IN ACCORDANCE WITH THE CITY OF OKEECHOBEE UTILITY DEPARTMENT SPECIFICATIONS.
- 4. ALL POTABLE PVC PIPE 3 INCHES IN DIAMETER OR LESS SHALL BE LISTED AS NSF-PW AND SHALL BE SO MARKED.
- 5. ALL WATER LINES 2" DIAMETER ONLY SHALL BE PVC 1120-PR200-SDR21, PVC 1120-SCHEDULE 40 AND SCHEDULE 80 IN ACCORDANCE WITH LAKE CITY UTILITY DEPARTMENT SPECIFICATIONS. ALL 2" AND SMALLER WATER SERVICE LINES SHALL BE POLYETHYLENE PIPE.
- 6. ALL NEW AND RELOCATED WATER MAIN PIPE, FITTINGS, VALVES, AND FIRE HYDRANTS SHALL BE IN CONFORMANCE WITH APPLICABLE AMERICAN WATER WORKS ASSOCIATION (AWWA) AND THE CITY OF OKEECHOBEE UTILITY DEPARTMENT STANDARDS.
- 7. ALL NEW AND RELOCATED WATER MAIN PIPE AND FITTINGS WILL COMPLY WITH THE LATEST FDEP AND AWWA STANDARDS FOR LEAD CONTENT.
- 8. ALL PACKING AND JOINTING MATERIALS USED IN THE JOINTS OF NEW AND RELOCATED WATER MAIN PIPE SHALL BE IN CONFORMANCE WITH APPLICABLE AWWA AND THE CITY OF OKEECHOBEE UTILITY DEPARTMENT STANDARDS.
- 9. ALL NEW AND RELOCATED WATER MAINS AND APPURTENANCES SHALL BE INSTALLED IN ACCORDANCE WITH AWWA AND THE CITY OF OKEECHOBEE UTILITY DEPARTMENT STANDARDS.
- 10. ALL ROCK AND UNSUITABLY SIZED STONES (AS DESCRIBED IN APPLICABLE AWWA AND THE CITY OF OKEECHOBEE UTILITY DEPARTMENT STANDARDS AND/OR PIPE MANUFACTURER'S RECOMMENDED INSTALLATION PROCEDURES) FOUND IN TRENCHES FOR NEW AND RELOCATED WATER MAIN PIPE SHALL BE REMOVED TO A DEPTH OF AT LEAST SIX (6) INCHES BELOW THE BOTTOM OF THE PIPE. CONTINUOUS AND UNIFORM BEDDING SHALL BE PROVIDED FOR NEW AND RELOCATED WATER MAIN PIPE. THE BACKFILL MATERIAL SHALL BE TAMPED IN LAYERS AROUND THE NEW AND RELOCATED WATER MAIN PIPE AND TO A SUFFICIENT HEIGHT ABOVE SUCH PIPE TO ADEQUATELY SUPPORT AND PROTECT THE PIPE.
- 11. ALL TEES, BENDS, PLUGS, AND HYDRANTS IN NEW AND RELOCATED WATER MAINS SHALL USE RESTRAINED JOINTS PER THE CITY OF OKEECHOBEE UTILITY DEPARTMENT DETAILS TO PREVENT MOVEMENT.
- 12. ALL NEW AND RELOCATED WATER MAINS SHALL BE DISINFECTED IN ACCORDANCE WITH AWWA STANDARD C651 AND RULE 62-555.345, F.A.C.
- 13. ALL NEW AND RELOCATED WATER SERVICES SHALL BE IN CONFORMANCE WITH THE STATE PLUMBING CODE.
- 14. ALL PIPE AND PIPE FITTINGS FOR NEW AND RELOCATED WATER SERVICES SHALL COMPLY WITH THE LATEST FDEP AND AWWA STANDARDS FOR LEAD CONTENT. ALL SOLDERS AND FLUX FOR NEW AND RELOCATED WATER SERVICES SHALL COMPLY WITH THE LATEST FDEP AND AWWA STANDARDS FOR LEAD CONTENT.
- 15. THE POTABLE WATER SYSTEM SHALL MAINTAIN THE MINIMUM HORIZONTAL AND VERTICAL CLEARANCES FROM THE SANITARY SEWER SYSTEM. THE MINIMUM HORIZONTAL AND VERTICAL CLEARANCES ARE DESCRIBED IN DETAIL UNDER THE STANDARD WATER/SEWER SEPARATION STATEMENT.

SANITARY SEWER SYSTEM NOTES:

- 1. THE CONTRACTOR SHALL FIELD VERIFY THE LOCATIONS OF ALL EXISTING UTILITIES AND CONNECTION POINTS PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER OF RECORD IMMEDIATELY.
- 2. SANITARY SEWER LINE SHALL BE SDR-26 PVC PIPE, CONFORMING TO ASTM D-3034. SANITARY SEWER LINE SHALL BE COLOR CODED AND CLEARLY MARKED.
- SANITARY SEWER FORCE MAIN SHALL BE C900 DR18 PVC PIPE CONFORMING TO ASTM D-1784, 3 D-2241, D-3139, AND F-477. SANITARY FORCE MAIN SHALL BE COLOR CODED GREEN AND CLEARLY MARKED.
- 4. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER OF RECORD AND THE CITY OF OKEECHOBEE UTILITY DEPARTMENT ALL VIDEO LOGS AND DEFLECTION TEST RESULTS FOR REVIEW AND APPROVAL.
- 5. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER OF RECORD AND THE CITY OF OKEECHOBEE UTILITY DEPARTMENT ALL EXFILTRATION /INFILTRATION TEST RESULTS FOR REVIEW AND APPROVAL.
- 6. THE SANITARY SEWER SYSTEM SHALL BE CONSTRUCTED IN ACCORDANCE WITH THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS. ANY DEVIATION FROM THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS SHALL BE THE SOLE RESPONSIBILITY OF THE ENTITY AUTHORIZING THE DEVIATIONS TO BRING THE SANITARY SEWER SYSTEM INTO COMPLIANCE WITH THESE CONSTRUCTION DOCUMENTS.
- THE SANITARY SEWER SYSTEM SHALL MAINTAIN THE MINIMUM HORIZONTAL AND VERTICAL CLEARANCES FROM THE POTABLE WATER SYSTEM. THE MINIMUM HORIZONTAL AND VERTICAL CLEARANCES ARE DESCRIBED IN DETAIL UNDER THE STANDARD WATER/SEWER SEPARATION STATEMENT.
- 8. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE CONSTRUCTION OF THE SANITARY SEWER SYSTEM WITH THE OTHER UTILITIES BEING INSTALLED IN THE VICINITY.
- 9. LEAKAGE TEST SHALL BE CONDUCTED SO THAT THE LEAKAGE EXFILTRATION OR INFLITRATION DOES NOT EXCEED 200 GALLONS PER INCH OF PIPE DIAMETER PER MILE PER DAY FOR ANY SECTION OF THE SYSTEM. EXFILTRATION OR INFILTRATION TESTS SHALL BE PERFORMED WITH A MINIMUM POSITIVE HEAD OF 2 FEET. AIR TESTS, AS A MINIMUM, CONFORM TO THE TEST PROCEDURE DESCRIBED IN ASTM C-828 FOR CLAY PIPE. ASTM C 924 FOR CONCRETE PIPE, ASTM F-1417 FOR PLASTIC PIPE, AND FOR OTHER MATERIALS APPROPRIATE TEST PROCEDURES.

STANDARD WATER/SEWER SEPARATION STATEMENT:

WHERE SANITARY SEWERS, FORCE MAINS AND STORM SEWERS MUST CROSS A WATER MAIN WITH LESS THAN 18 INCHES VERTICAL DISTANCE, BOTH THE SEWER AND THE WATER MAIN SHALL BE CONSTRUCTED OF DUCTILE IRON PIPE (DIP) AT THE CROSSING. (DIP IS NOT REQUIRED FOR STORM SEWERS) SUFFICIENT LENGTHS OF DIP MUST BE USED TO PROVIDE A MINIMUM SEPARATION OF 10 FEET BETWEEN ANY TWO JOINTS. ALL JOINTS ON THE WATER MAIN WITHIN 20 FEET OF THE CROSSING MUST BE MECHANICALLY RESTRAINED. A MINIMUM VERTICAL CLEARANCE OF 12 INCHES MUST BE MAINTAINED AT ALL CROSSINGS.

ALL CROSSINGS SHALL BE ARRANGED SO THAT THE SEWER PIPE JOINTS AND THE WATER MAIN PIPE JOINTS ARE EQUIDISTANT FROM THE POINT OF CROSSING (PIPES CENTERED ON THE CROSSING).

WHERE A NEW PIPE CONFLICTS WITH AN EXISTING PIPE WITH LESS THAN 18 INCHES OF VERTICAL CLEARANCE, THE NEW PIPE SHALL BE CONSTRUCTED OF DIP (EXCEPT STORM SEWERS AND SANITARY SEWER) AND THE NEW PIPE SHALL BE ARRANGED TO MEET THE CROSSING REQUIREMENTS ABOVE.

WHERE IT IS NOT POSSIBLE TO MAINTAIN A VERTICAL DISTANCE OF 18 INCHES IN PARALLEL INSTALLATIONS, THE WATER MAIN SHALL BE CONSTRUCTED OF DIP AND THE SEWER OR THE FORCE MAIN SHALL BE CONSTRUCTED OF DIP (EXCEPT STORM SEWERS) WITH A MINIMUM VERTICAL DISTANCE OF 6 INCHES. THE WATER MAIN SHOULD ALWAYS BE ABOVE THE SEWER. JOINTS ON THE WATER MAIN SHALL BE LOCATED AS FAR APART AS POSSIBLE FROM JOINTS ON THE SEWER OR FORCE MAIN (STAGGERED JOINTS).

STANDARDS AND SPECIFICATIONS.

HYDROSTATIC TESTING NOTES:

AS-BUILT AND TESTING NOTES:

- PROVIDE RESULTS TO THE OWNER:
 - ASTM F1417 FOR PLASTIC PIPES.

 - 3.4 FOR FIRE MAINS: PER FFPC AND NFPA

 - LIFT FOR EACH 10,000 SQ. FT.

TESTING.

5. OTHER DOCUMENTS TO BE PROVIDED BY CONTRACTOR:

CONSTRUCTION COST AS MAY BE REQUIRED BY JURISDICTION TABULATED CAMS DATA AS MAY BE REQUIRED BY JURISDICTION TABULATED NORTHING/EASTING AND GPS DATA AS MAY BE REQUIRED BY JURISDICTION

FDEP SANITARY SEWER TESTING REQUIREMENTS:

LEAKAGE TESTS:

DEFLECTION TEST FOR ALL FLEXIBLE PIPE:

TESTING IS REQUIRED AFTER THE FINAL BACKFILL HAS BEEN IN PLACE AT LEAST 30 DAYS TO PERMIT STABILIZATION OF THE SOIL-PIPE SYSTEM.

- NO PIPE SHALL EXCEED A DEFLECTION OF 5% 1.

1. SANITARY SEWERS (INCLUDING LATERALS), FORCE MAINS, AND STORM SEWERS SHOULD CROSS UNDER WATER MAINS WHENEVER POSSIBLE. SANITARY SEWERS, FORCE MAINS AND STORM SEWERS CROSSING WATER MAINS SHALL BE LAID TO PROVIDE A MINIMUM VERTICAL DISTANCE OF 18 INCHES BETWEEN THE INVERT OF THE UPPER PIPE AND THE CROWN OF THE LOWER PIPE WHENEVER POSSIBLE.

2. ALL UNDERGROUND WATERMAINS IN THE PROJECT WILL BE LAID TO PROVIDE A HORIZONTAL SEPARATION OF AT LEAST 3 FEET BETWEEN THE OUTSIDE OF THE WATERMAIN AND OUTSIDE OF ANY VACUUM SANITARY SEWER, STORM SEWER, STORMWATER FORCEMAIN, OR PIPELINE CONVEYING RECLAIMED WATER UNDER PART III OF CHAPTER 62-610 F.A.C. WATERMAIN SHALL BE LAID AT A MINIMUM DISTANCE OF 6 FEET BETWEEN THE OUTSIDE OF THE WATERMAIN AND THE OUTSIDE OF A GRAVITY SANITARY SEWER (3 FEET IF THE WATERMAIN BOTTOM IS LAID ATLEAST 6" ABOVE THE TOP OF THE SEWER) WATERMAIN SHALL BE LAID ATLEAST 6 FEET HORIZONTALLY (OUTSIDE TO OUTSIDE) FROM PRESSURE SANITARY SEWER, SANITARY FORCEMAINS, OR RECLAIMED WATER LINES NOT REGULATED UNDER PART III OF CHAPTER 62-610 F.A.C.

3. ALL DUCTILE IRON, HDPE, AND PVC PIPE SHALL MEET THE CITY OF OKEECHOBEE UTILITY DEPARTMENT

1. AFTER ALL PRESSURE PIPES (WATER MAINS, SERVICES, AND FORCE MAINS) ARE INSTALLED, THE JOINTS COMPLETED, AND THE TRENCH BACKFILLED, THE NEWLY LAID PIPE AND APPURTENANCES SHALL BE SUBJECTED TO A HYDROSTATIC TEST OF 150 PSI FOR WATER, 150 PSI FOR SANITARY FORCEMAIN, AND 200 PSI FOR FIRE LINE FOR A PERIOD OF AT LEAST TWO HOURS. CONTRACTOR SHALL PERFORM A 24-HOUR (MINIMUM) PRE-TEST OF ALL PRESSURE PIPING PRIOR TO SCHEDULING PRESSURE TESTS WITH THE CITY OF OKEECHOBEE UTILITY DEPARTMENT AND THE ENGINEER. THE ENGINEER, AND THE CITY OF OKEECHOBEE UTILITY DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS BEFORE A TEST IS TO BE PERFORMED. TEST SHALL BE AS SET FORTH IN AWWA STANDARD C600. ANY LEAKS DETECTED SHALL BE CORRECTED AND THE SECTION OF PIPELINE RETESTED. THE TWO HOUR TEST PERIOD SHALL BEGIN WHEN ALL JOINTS HAVE BEEN DETERMINED TO BE WATER TIGHT. LEAKAGE SHALL BE LIMITED TO THAT ALLOWANCE SET FORTH IN SECTION 4 OF AWWA STANDARD C600 LATEST EDITION. HYDROSTATIC AND LEAKAGE TEST AND BLOW-DOWN (ZEROING OF GAGE) MUST OCCUR BEFORE SAMPLING FOR BACTERIOLOGICAL TEST. THE MAXIMUM ALLOWABLE PRESSURE LOSS IS 5 PSI REGARDLESS OF THE LENGTH OF PIPE.

1. SIGNED AND SEALED AS-BUILTS SHALL BE PREPARED BY THE CONTRACTOR ACCORDING TO AGENCY CLEARANCE AND OWNER REQUIREMENTS.

TESTING SHALL BE PERFORMED BY THE CONTRACTOR ACCORDING TO ALL APPLICABLE REGULATIONS. 3. IN ADDITION TO THE REQUIREMENTS ABOVE, THE CONTRACTOR SHALL PERFORM THE FOLLOWING TESTS AND

3.1 FOR GRAVITY SANITARY SEWER: AIR TESTED BETWEEN MANHOLES AT 3.5 PSI FOR 5 MINUTES PER

3.2 FOR SANITARY FORCEMAINS: HYDROSTATIC PRESSURE TEST OF 150 PSI FOR 2 HOURS 3.3 FOR WATERMAINS AND SERVICE LATERALS: HYDROSTATIC PRESSURE TEST OF 150 PSI FOR 2 HOURS 3.5 FOR LIFT STATIONS: START-UP OBSERVATION REPORT BY PUMP MANUFACTURER.

3.6 FOR CONCRETE PAVEMENT: TEST CYLINDERS (7 DAY AND 28 DAY BREAKS) AT RATE OF 1 PER 100 C.Y. 3.7 FOR ASPHALT PAVEMENT: CORES TAKEN 3 HEAVY DUTY AND 3 LIGHT DUTY PER DAY OF ASPHALT WORK, LOCATIONS AS SPECIFIED BY OWNER. THE CORES SHALL BE TESTED FOR DENSITY AND CONFORMANCE TO THICKNESS REQUIREMENTS. CORES SHALL BE HOT PATCHED BY CONTRACTOR. 3.8 FOR SUBGRADE: DENSITY SHALL BE TESTED BY NUCLEAR METHOD. ASTM D6938. ONE TEST IN EACH 3.9 FOR BASE: DENSITY SHALL BE TESTED BY NUCLEAR METHOD, ASTM D6938. ONE TEST IN EACH LIFT

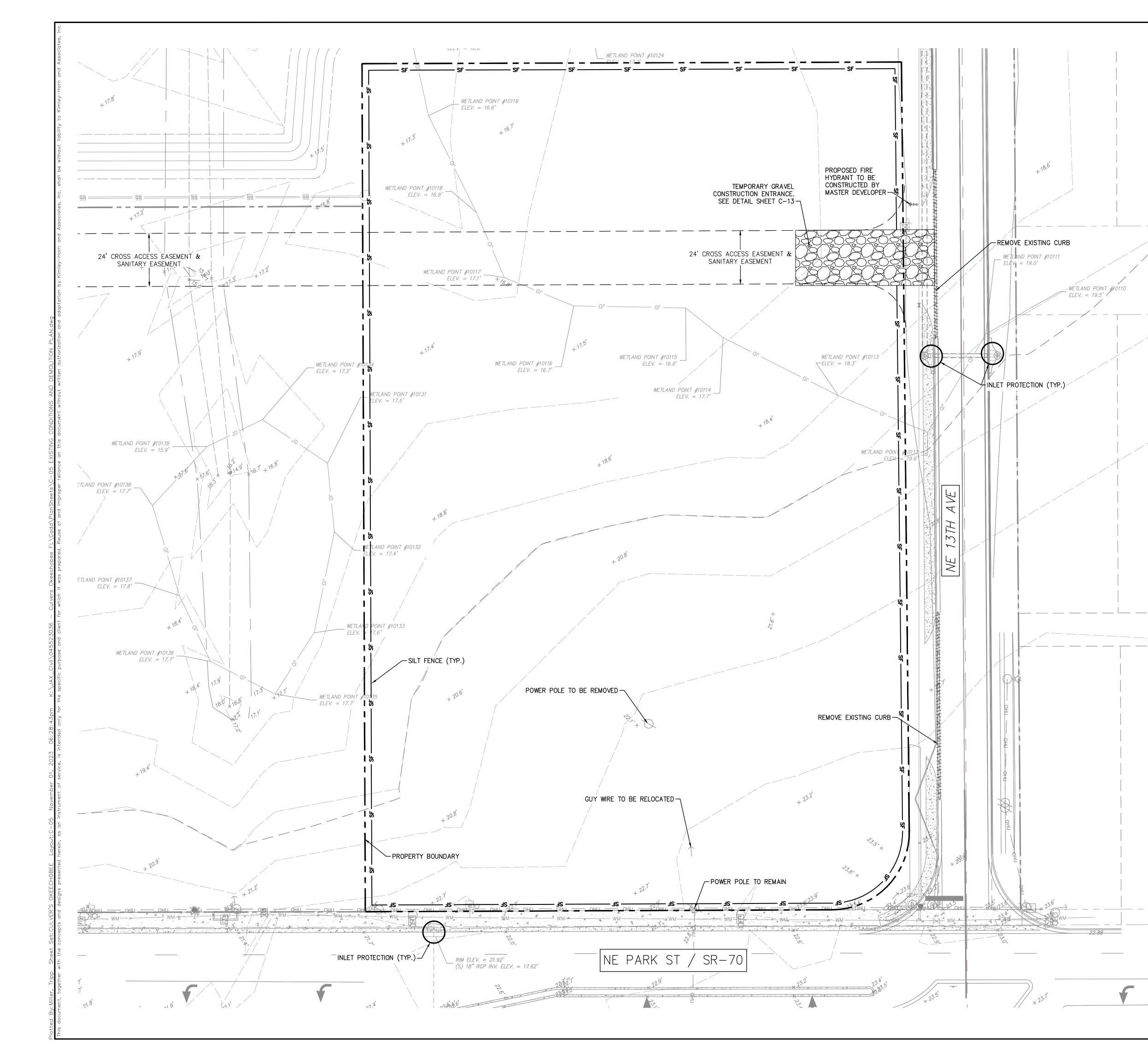
FOR EACH 20,000 SQ. FT. OF IN-PLACE BASE MATERIAL AREA. THICKNESS SHALL BE TESTED ONCE FOR EACH 20,000 SQ. FT. OF IN-PLACE MATERIAL AREA. 3.10 FOR UTILITY TRENCHES: DENSITY TEST EVERY 300', MINIMUM ON BETWEEN EACH STRUCTURE.

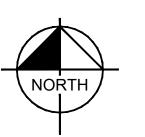
4. IN CASE OF CONFLICT OR DUPLICATION, CONTRACTOR SHALL ISSUE BID QUESTION ACCORDING TO BID REQUIREMENTS. IF NO QUESTION IS ASKED, BASE BID SHALL INCLUDE THE CONFLICTING OR DUPLICATE

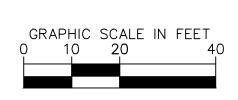
THE LEAKAGE EXFILTRATION OR INFILTRATION DOES NOT EXCEED 200 GALLONS PER INCH OF PIPE DIAMETER PER MILE PER DAY FOR ANY SECTION OF THE SYSTEM. EXFILTRATION OR INFILTRATION TESTS BE PERFORMED WITH A MINIMUM POSITIVE HEAD OF 2 FEET AIR TESTS, AS A MINIMUM, CONFORM TO THE TEST PROCEDURE DESCRIBED IN ASTM C-828 FOR CLAY PIPE, ASTM C-924 FOR CONCRETE PIPE, ASTM F-1417 FOR PLASTIC PIPE, AND FOR OTHER MATERIALS APPROPRIATE TEST PROCEDURES. [rswf 33.93, 33.94, AND 33.95]

2. A RIGID BALL OR MANDREL FOR THE DEFLECTION TEST WITH A DIAMETER NOT LESS THAN 95% OF THE BASE INSIDE DIAMETER OR AVERAGE INSIDE DIAMETER OF THE PIPE, DEPENDING ON WHICH IS SPECIFIED IN THE ASTM SPECIFICATION, INCLUDING THE APPENDIX TO WHICH THE PIPE IS MANUFACTURED. 3. PERFORM THE TEST WITHOUT PULLING DEVICES. [RSWF 33.85]

				DATE BY
				REVISIONS
				No.
Kimley» Horn	© 2023 KIMLEY-HORN AND ASSOCIATES, INC.		PHONE: 904-828-3900	WWW.KIMLEY-HORN.COM REGISTRY NO. 35106
F(ENGINEER SIGNATURI REFE C-02 - SIGNA	e Al Er 1	ND S TO	SEA	
KHA PROJECT 045523036 045523036 DATE NOV. 2023	SCALE AS SHUWN	DESIGNED BY JWC	DRAWN BY RCM	снескер ву JWC
CULVER'S OKEECHOBEE	S&L PROPERHES OREECHOBEE,			OKEECHOBEE FLORIDA
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DEMOLITION NOTES:

- 1. ALL DEMOLITION MATERIALS TO BE REMOVED FROM SITE AND PROPERLY DISPOSED OF AT AN APPROVED LANDFILL.
- 2. THE INTENT OF THE DEMOLITION PLAN IS TO DEPICT EXISTING FEATURES THAT ENCUMBER THE PROPOSED CONSTRUCTION AREA AND ARE SCHEDULED FOR REMOVAL. SOME INCIDENTAL ITEMS MAY HAVE BEEN INADVERTENTLY OMITTED FROM THE PLAN. THE CONTRACTOR IS ENCOURAGED TO THOROUGHLY INSPECT THE SITE AS WELL AS REVIEW THE PLANS AND SPECIFICATIONS PRIOR TO SUBMITTING PRICING. CONTRACTOR WILL NOT RECEIVE ADDITIONAL COMPENSATION FOR INCIDENTAL ITEMS NOT SHOWN ON THIS DEMOLITION PLAN.
- 3. THIS DEMOLITION PLAN IS BASED ON AVAILABLE UTILITY INFORMATION AND MAY OR MAY NOT BE ALL INCLUSIVE FOR THIS SITE. ANY UTILITIES ENCOUNTERED DURING DEMOLITION THAT ARE NOT DEPICTED/ADDRESSED ON THIS DRAWING SHOULD BE BROUGHT TO THE ATTENTION OF THE OWNER, ARCHITECT, AND PROJECT ENGINEER IMMEDIATELY.
- 4. ALL FEATURES IDENTIFIED ON THIS PLAN WHICH ARE LISTED TO BE DEMOLISHED ARE TO BE REMOVED FROM THE SITE. AFTER DEMOLITION IS COMPLETE THE SITE SHALL BE DELIVERED IN A CONDITION SUITABLE FOR DEVELOPMENT.
- 5. CONTRACTOR IS RESPONSIBLE FOR CONTROLLING AIRBORNE DUST AND POLLUTANTS BY USING WATER SPRINKLING OR OTHER SUITABLE MEANS OF CONTROL.
- 6. CONTRACTOR TO USE CARE IN HANDLING DEBRIS FROM SITE TO ENSURE THE SAFETY OF THE PUBLIC. HAUL ROUTE TO BE CLOSELY MONITORED FOR DEBRIS OR MATERIALS TRACKED ONTO ADJOINING ROADWAYS, SIDEWALKS, ETC. ROADWAYS AND WALKWAYS TO BE CLEARED DAILY OR AS NECESSARY TO MAINTAIN PUBLIC SAFETY.

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FOR

ENGINEER OF RECORD

SIGNATURE AND SEAL

REFER TO

C-02 - SIGNATURE SHEET

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- 7. DEWATERING SHOULD BE ANTICIPATED AND ALL COSTS, PERMITS AND EQUIPMENT FOR DEWATERING SHALL BE INCLUDED IN THE CONTRACTOR'S BID PRICE.
- 8. ALL ASPHALT TO BE REMOVED SHALL BE SAW CUT ADJACENT TO REMAINING IMPROVEMENTS.
- 9. SEE SITE AND IMPROVEMENTS PLANS FOR LIMITS AND GRADING OF RESURFACED DRIVEWAYS AND ENTRANCES.
- 10. SEE LANDSCAPE PLAN FOR TREE REMOVAL/ RELOCATION AND TREE PROTECTION.
- 11. SEE EROSION CONTROL PLAN FOR REMAINING INLET PROTECTION AND EROSION PREVENTION.
- 12. DEMOLITION CONTRACTOR SHALL CONSTRUCT EROSION CONTROL MEASURES AS SHOWN ON THIS PLAN AS NECESSARY DURING DEMOLITION OPERATIONS OR AS DIRECTED BY THE ENGINEER, CITY OR COUNTY INSPECTOR TO PROTECT ADJACENT PROPERTIES AND WATER RESOURCES FROM SOIL EROSION AND SEDIMENTATION.
- 13. VARIATIONS TO CONDITIONS OR DISCREPANCIES IN ACTUAL FIELD CONDITIONS AS THEY APPLY TO THE DEMOLITION OR SITE DEVELOPMENT WORK MUST BE BROUGHT TO THE OWNER'S REPRESENTATIVE AND ENGINEER'S ATTENTION PRIOR TO THE COMMENCEMENT OF ANY DEMOLITION.
- 14. CONTRACTOR SHALL LOCATE, IDENTIFY, PROPERLY TERMINATE, AND MARK ALL EXISTING UTILITIES WITHIN THE DEMOLITION AREA THAT ARE TO REMAIN AND PROTECT THEM FROM DAMAGE.
- 15. THE DEMOLITION CONTRACTOR MUST PROTECT ALL LANDSCAPING AND OTHER FEATURES DESIGNATED TO REMAIN AND REPLACE SUCH ITEMS IF DISTURBED DURING DEMOLITION.
- 16. CONTRACTOR SHALL CONDUCT ALL DEMOLITION OPERATIONS WITH MINIMUM INTERFERENCE TO PUBLIC AND/OR PRIVATE ACCESSES AND FACILITIES. ALL AREAS OUTSIDE OF THE PAVEMENT REMOVAL LINE MUST BE MAINTAINED FOR PUBLIC ACCESS.
- 17. THE DEMOLITION CONTRACTOR MUST PROTECT BENCH MARKS PROPERTY CORNERS, AND ALL OTHER SURVEY MONUMENTS FROM DAMAGE OR DISPLACEMENT. IF A MARKER MUST BE REMOVED, IT SHALL BE REPLACED BY A LICENSED LAND SURVEYOR AND REPLACED.
- 18. THE DEMOLITION CONTRACTOR SHALL PROVIDE TRAFFIC CONTROL MEASURES AS REQUIRED IN ACCORDANCE WITH THE U.S. DEPT. OF TRANSPORTATION "MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES" AND GOVERNING MUNICIPALITY REQUIREMENTS.
- 19. DEPRESSIONS AND VOID AREAS CAUSED BY DEMOLITION WORK ARE TO BE FILLED TO SUBGRADE AND PROPERLY COMPACTED TO THE EXISTING ELEVATION TO AVOID WATER PONDING.
- 20. THE DEMOLITION CONTRACTOR SHALL ACQUIRE ALL PERMITS AND PAY ALL ASSOCIATED FEES PERTAINING TO THE DEMOLITION AND DISPOSAL REQUIREMENTS. THE DEMOLITION CONTRACTOR IS RESPONSIBLE FOR ADHERENCE TO ALL LOCAL, STATE, AND FEDERAL REQUIREMENTS REGARDING THIS WORK. IF THERE ARE NO LOCAL, STATE OR FEDERAL REQUIREMENTS THE DEMOLITION CONTRACTOR SHALL ACQUIRE AN ENGINEERS CERTIFICATION THAT THE WORK HAS BEEN DONE IN ACCORDANCE WITH THE PLANS.
- 21. THE DEMOLITION CONTRACTOR SHALL COORDINATE THE DEMOLITION TO LEAVE ACCESS AND UTILITY SERVICE TO THE NEIGHBORING BUILDINGS AT ALL TIMES.
- 22. THE DEMOLITION CONTRACTOR SHALL FIELD LOCATE, MARK, AND PROTECT ALL EXISTING UTILITIES.
- 23. WATER SERVICE, SEWER SERVICE, POWER SERVICE AND ALL DRY UTILITIES SHALL BE MAINTAINED "IN-SERVICE" DURING THE HOURS OF BUSINESS FOR ALL EXISTING TENANTS.

PROPERTY BOUNDARY

LEGEND:

----- CURB REMOVAL

----- SILT FENCE

TEMPORARY GRAVEL CONSTRUCTION ENTRANCE ASPHALT PAVEMENT/

CONCRETE/ CURB REMOVAL

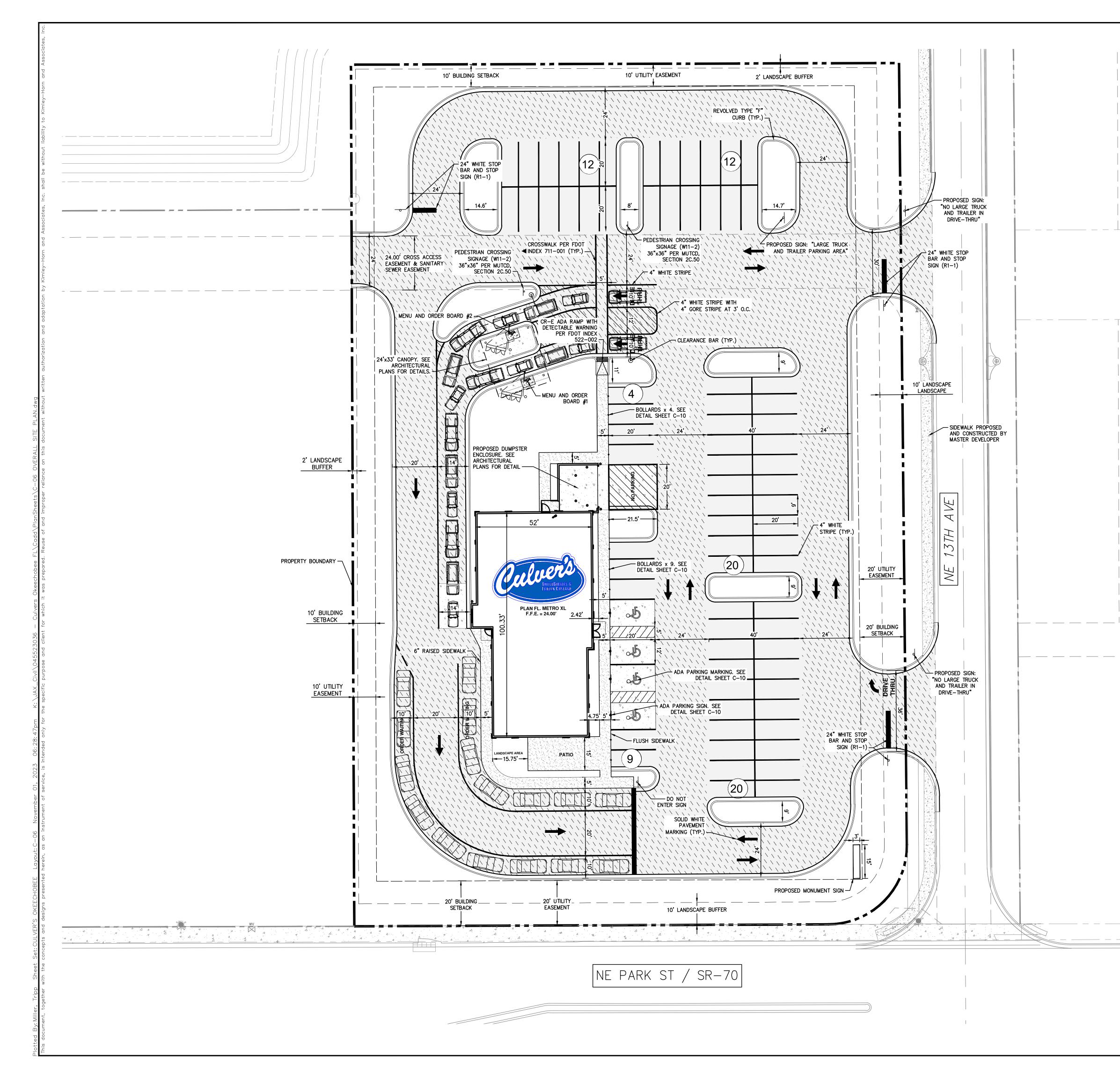
INLET PROTECTION

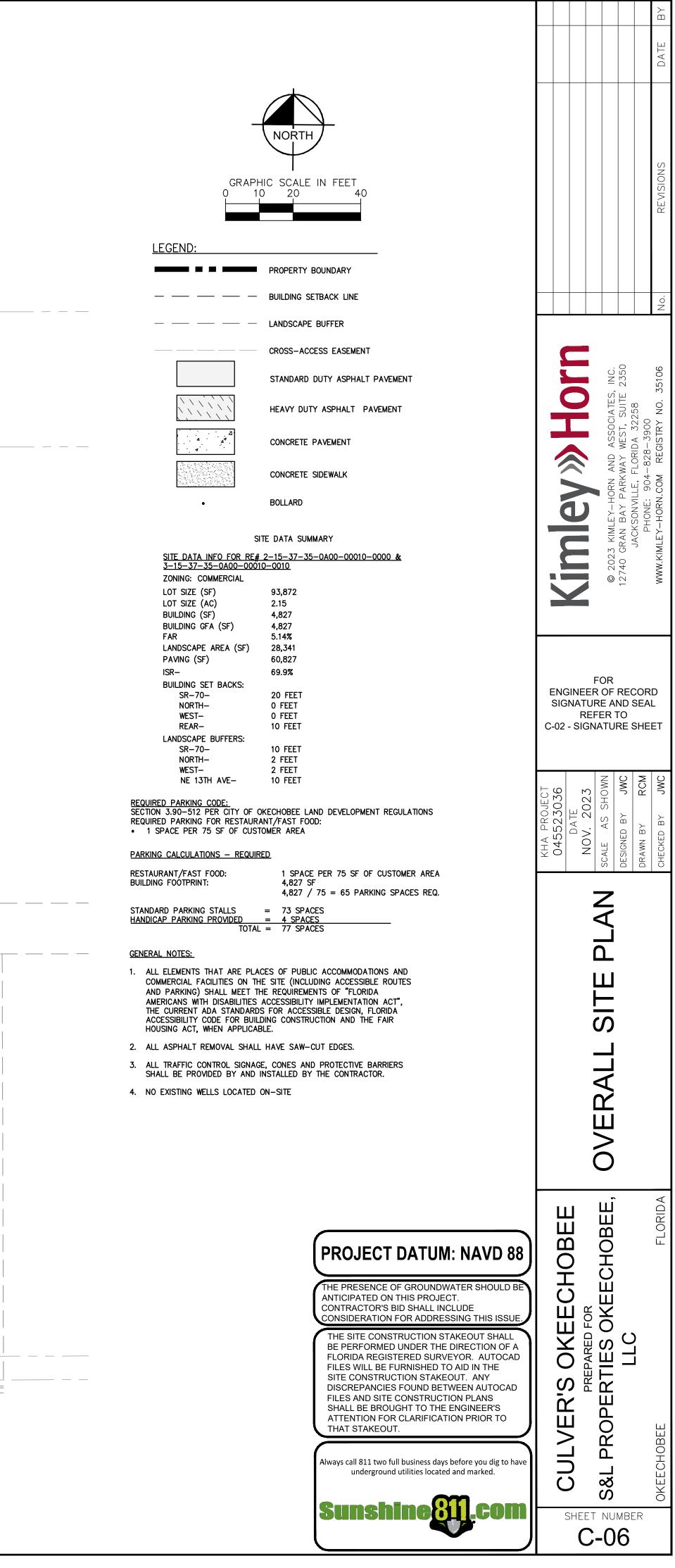
ALL ELEMENTS THAT ARE PLACES OF PUBLIC ACCOMMODATIONS AND COMMERCIAL FACILITIES ON THE SITE (INCLUDING ACCESSIBLE ROUTES AND PARKING) SHALL MEET THE REQUIREMENTS OF "FLORIDA AMERICANS WITH DISABILITIES ACCESSIBILITY IMPLEMENTATION ACT", THE CURRENT ADA STANDARDS FOR ACCESSIBLE DESIGN, FLORIDA ACCESSIBLE DESIGN, FLORIDA ACCESSIBLITY CODE FOR BUILDING CONSTRUCTION AND THE FAIR HOUSING ACT, WHEN APPLICABLE.

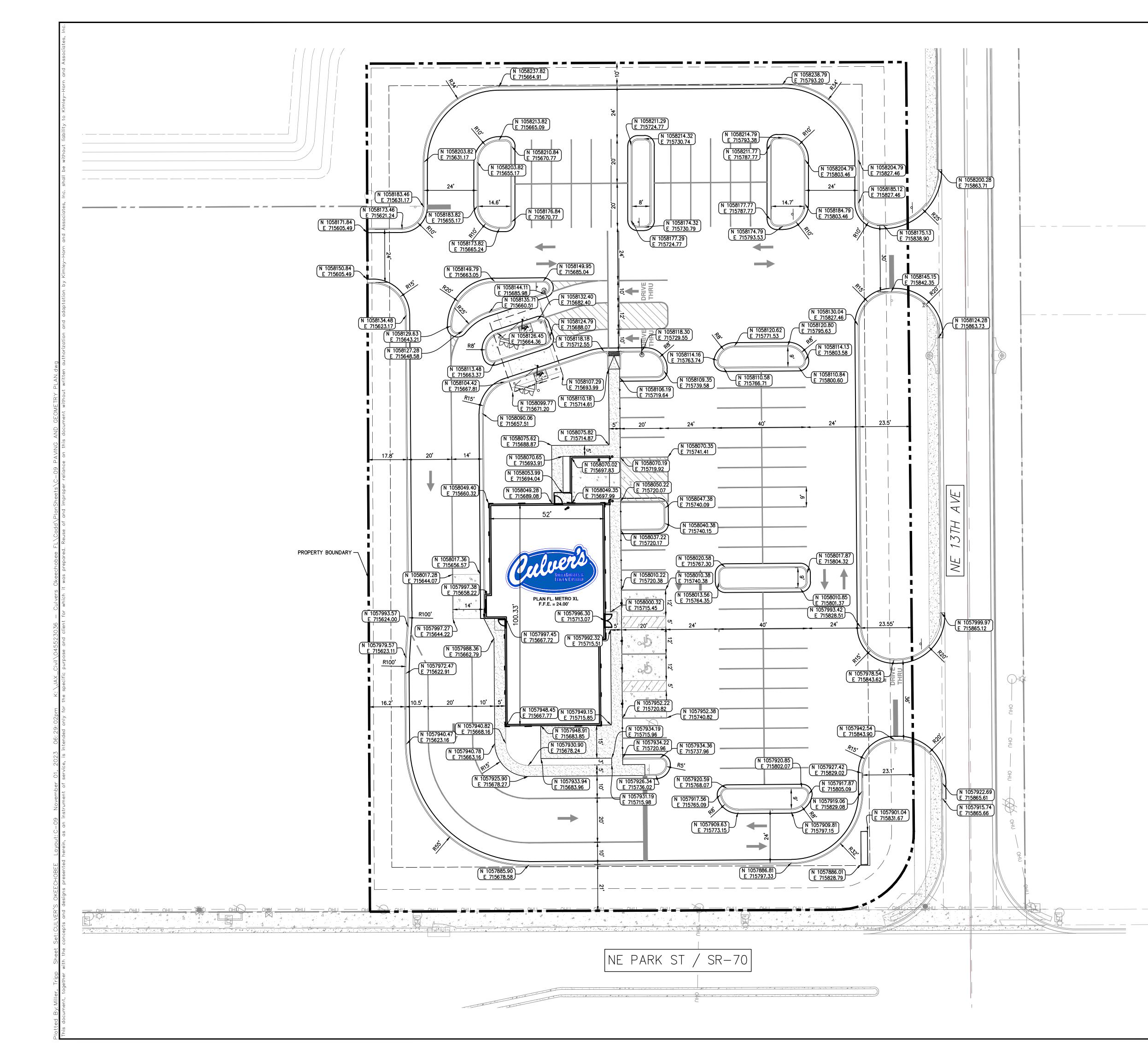
ways call 811 two full business days before you dig to hav underground utilities located and marked.

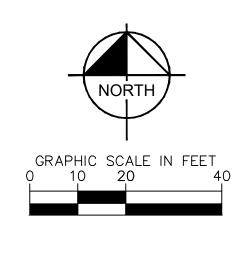
THE PRESENCE OF GROUNDWATER SHOULD BE ANTICIPATED ON THIS PROJECT. CONTRACTOR'S BID SHALL INCLUDE ALL COSTS AND METHODS FOR ALL PROJECT RELATED DEWATERING FOR THE DURATION OF CONSTRUCTION.

PROJECT DATUM: NAVD 88









LEGEND:

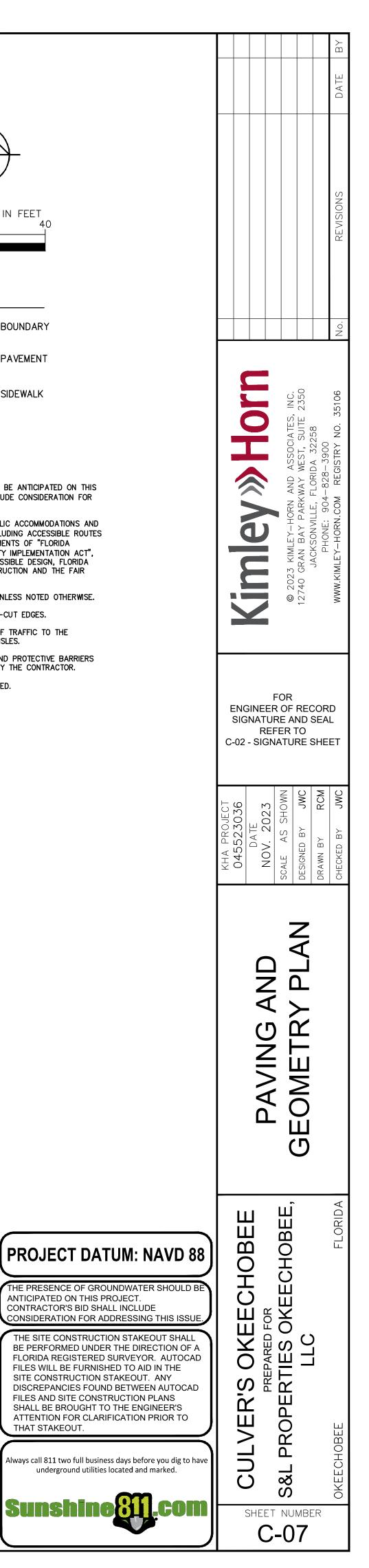
 PROPERTY BOUNDARY
CONCRETE PAVEMENT
CONCRETE SIDEWALK

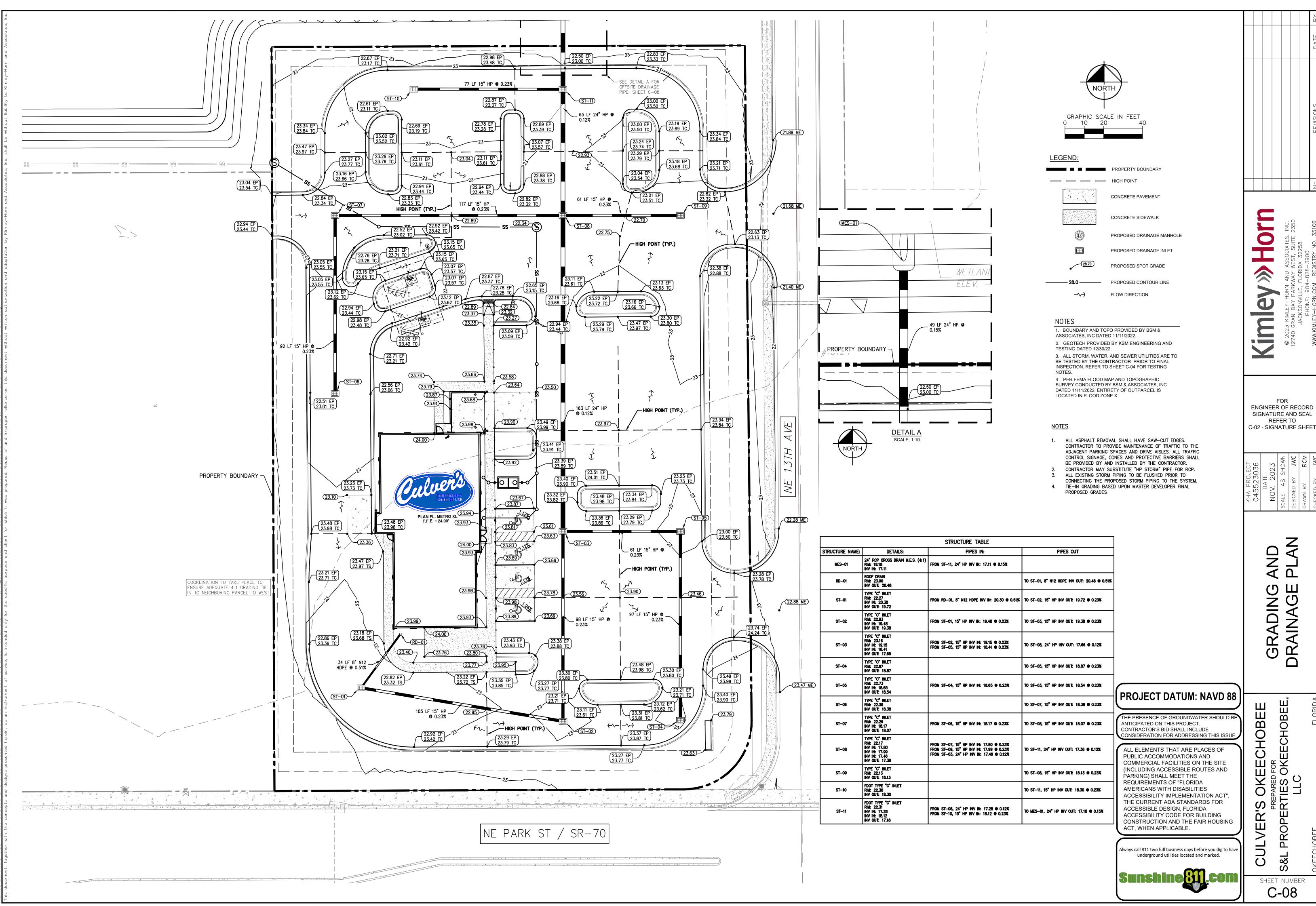
NOTES:

- 1. THE PRESENCE OF GROUNDWATER SHOULD BE ANTICIPATED ON THIS PROJECT. CONTRACTOR'S BID SHALL INCLUDE CONSIDERATION FOR ADDRESSING THIS ISSUE.
- 2. ALL ELEMENTS THAT ARE PLACES OF PUBLIC ACCOMMODATIONS AND COMMERCIAL FACILITIES ON THE SITE (INCLUDING ACCESSIBLE ROUTES AND PARKING) SHALL MEET THE REQUIREMENTS OF "FLORIDA AMERICANS WITH DISABILITIES ACCESSIBILITY IMPLEMENTATION ACT", THE CURRENT ADA STANDARDS FOR ACCESSIBLE DESIGN, FLORIDA ACCESSIBILITY CODE FOR BUILDING CONSTRUCTION AND THE FAIR HOUSING ACT, WHEN APPLICABLE.
- 3. ALL DIMENSION ARE TO FACE OF CURB. UNLESS NOTED OTHERWISE.
- 4. ALL ASPHALT REMOVAL SHALL HAVE SAW-CUT EDGES.
- 5. CONTRACTOR TO PROVIDE MAINTENANCE OF TRAFFIC TO THE ADJACENT PARKING SPACES AND DRIVE AISLES.
- 6. ALL TRAFFIC CONTROL SIGNAGE, CONES AND PROTECTIVE BARRIERS SHALL BE PROVIDED BY AND INSTALLED BY THE CONTRACTOR.

THAT STAKEOUT.

7. ALL RADII ARE 3' UNLESS OTHERWISE NOTED.





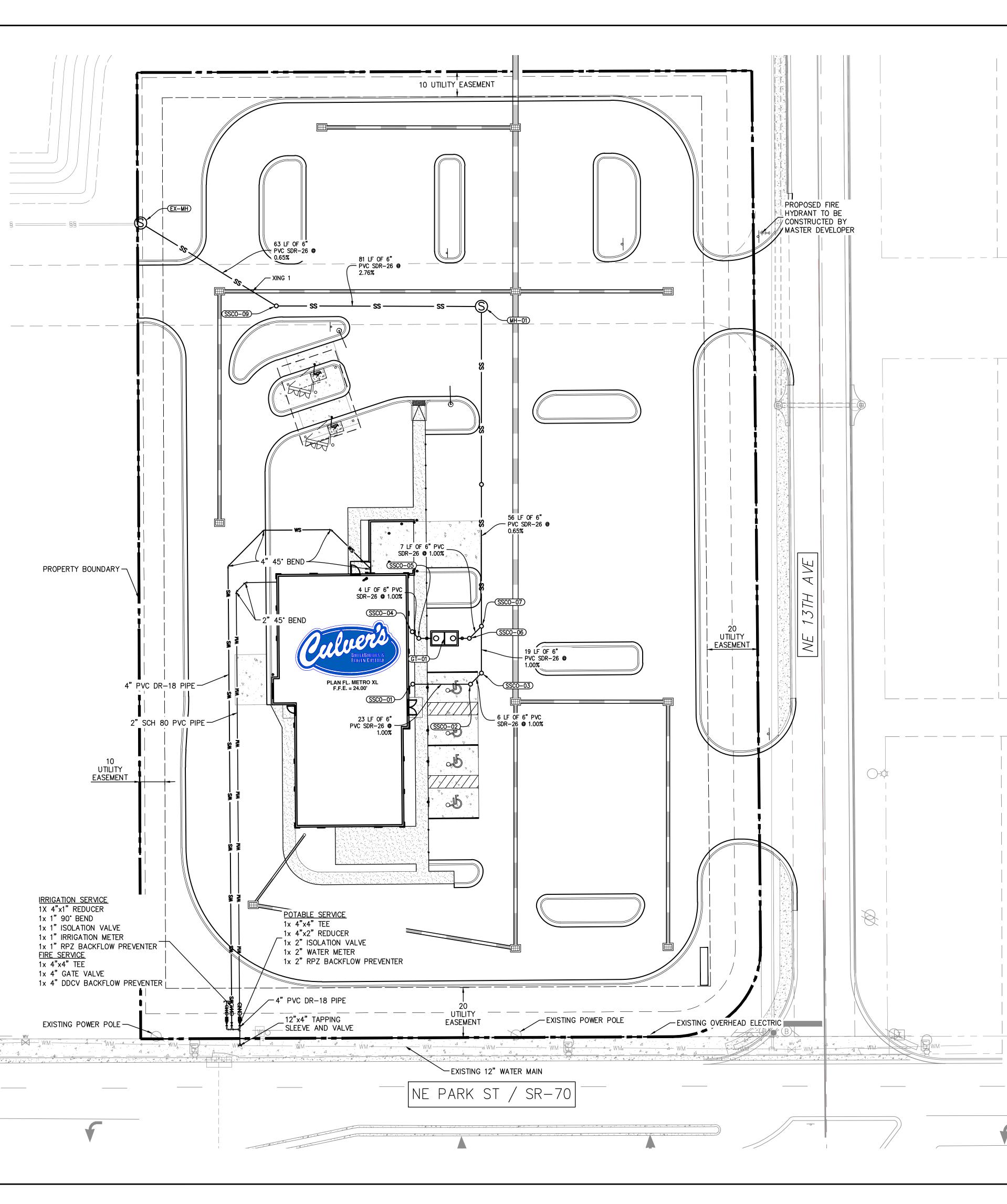
	PIPES IN:	PIPES OUT
.E.S. (4:1)	FROM ST-11, 24" HP INV IN: 17.11 • 0.15%	
		TO ST-01, 8" N12 HDPE INV OUT: 20.48 @ 0.51%
	FROM RD-01, 8" N12 HDPE INV IN: 20.30 @ 0.51%	To st-02, 15" HP INV OUT: 19.72 ● 0.23%
	FROM ST-01, 15" HP INV IN: 19.48 • 0.23%	TO ST-03, 15° HP INV OUT: 19.38 ● 0.23%
	FROM ST-02, 15" HP INV IN: 19.15 • 0.23% FROM ST-05, 15" HP INV IN: 18.41 • 0.23%	To ST-08, 24" HP INV OUT: 17.66 @ 0.12%
		TO ST-05, 15° HP INV OUT: 18.87 ● 0.23%
	FROM ST-04, 15" HP INV IN: 18.65 • 0.23%	TO ST-03, 15" HP INV OUT: 18.54 ● 0.23%
		To st−07, 15" HP INV OUT: 18.38 ● 0.23%
	FROM ST-06, 15" HP INV IN: 18.17 • 0.23%	To ST-08, 15" HP INV OUT: 18.07 ● 0.23%
	FROM ST-07, 15" HP INV IN: 17.80 • 0.23% FROM ST-09, 15" HP INV IN: 17.99 • 0.23% FROM ST-03, 24" HP INV IN: 17.46 • 0.12%	To ST-11, 24" HP INV OUT: 17.36 • 0.12%
		To ST-08, 15" HP INV OUT: 18.13 • 0.23%
		To st−11, 15" HP INV OUT: 18.30 ● 0.23%
	FROM ST-08, 24" HP INV IN: 17.28 • 0.12% FROM ST-10, 15" HP INV IN: 18.12 • 0.23%	To Mes−01, 24" HP INV OUT: 17.18 ● 0.15%

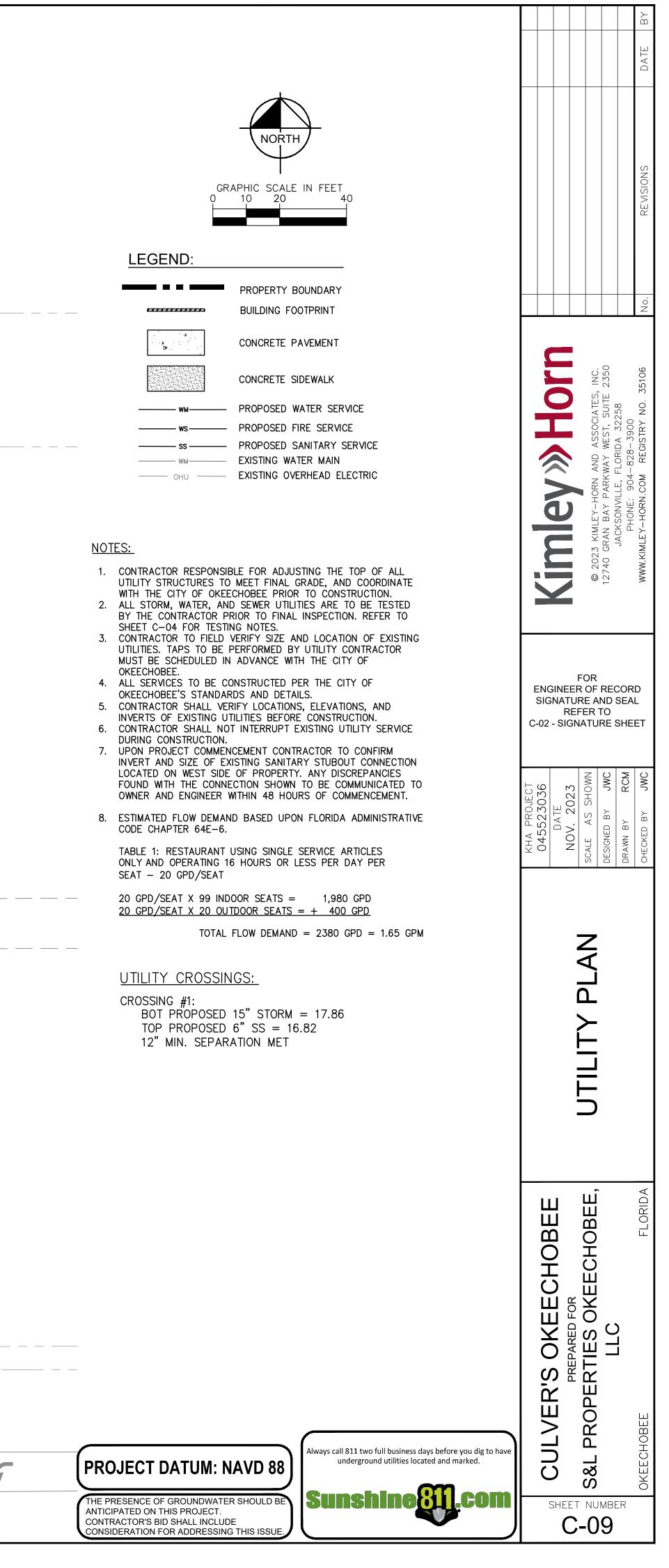
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PROJECT DATUM: NAVD 88 THE PRESENCE OF GROUNDWATER SHOULD BE ANTICIPATED ON THIS PROJECT. CONTRACTOR'S BID SHALL INCLUDE CONSIDERATION FOR ADDRESSING THIS ISSUE.	EECHOBEE FOR OKEECHOBEE,
ALL ELEMENTS THAT ARE PLACES OF PUBLIC ACCOMMODATIONS AND COMMERCIAL FACILITIES ON THE SITE (INCLUDING ACCESSIBLE ROUTES AND PARKING) SHALL MEET THE REQUIREMENTS OF "FLORIDA AMERICANS WITH DISABILITIES ACCESSIBILITY IMPLEMENTATION ACT", THE CURRENT ADA STANDARDS FOR ACCESSIBLE DESIGN, FLORIDA ACCESSIBLE DESIGN, FLORIDA ACCESSIBILITY CODE FOR BUILDING CONSTRUCTION AND THE FAIR HOUSING ACT, WHEN APPLICABLE.	CULVER'S OKEECH PREPARED FOR &L PROPERTIES OKEE
Always call 811 two full business days before you dig to have underground utilities located and marked.	CULV S&L PF

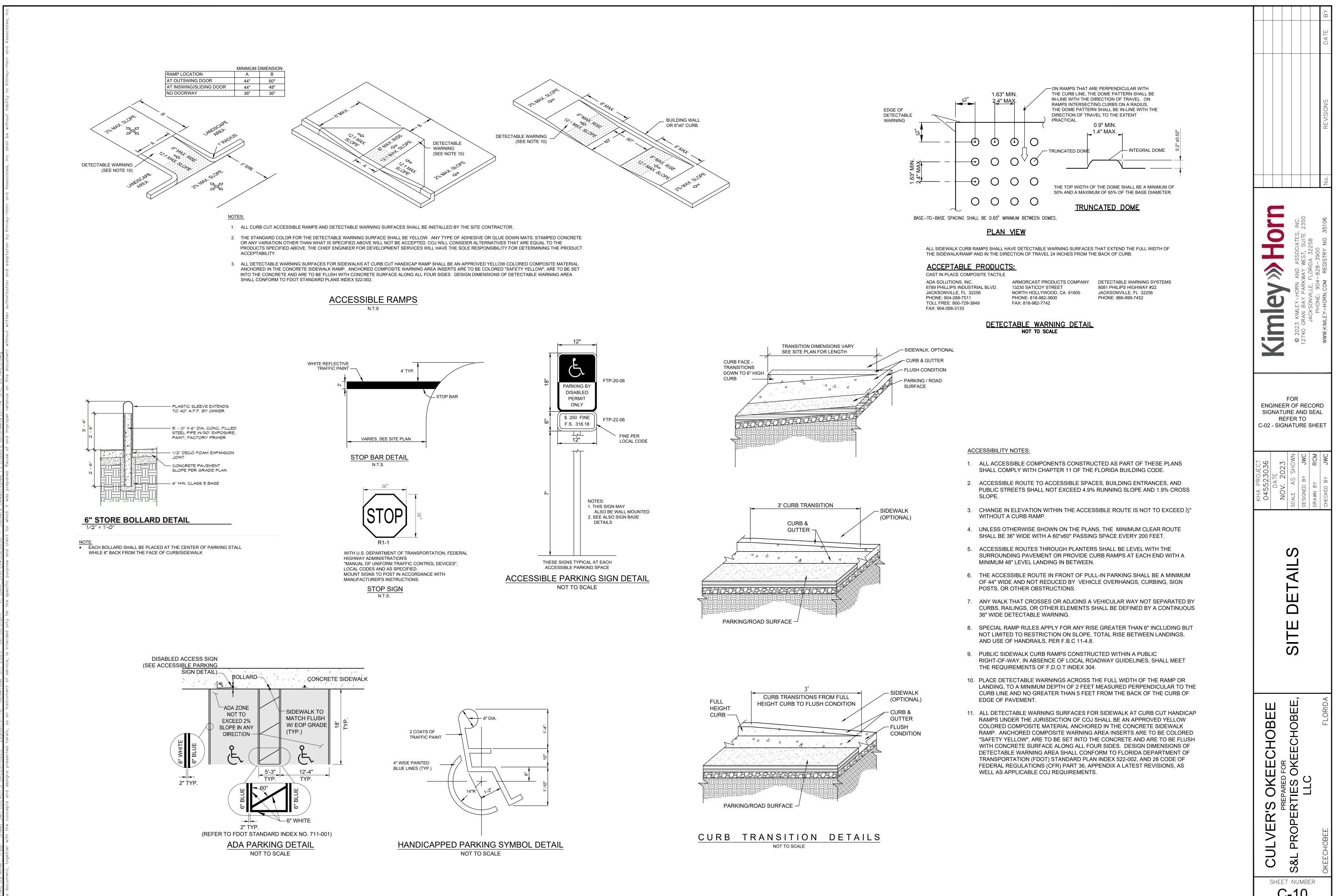
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STRUCTURE TABLE							
STRUCTURE NAME:	DETAILS:						
EX-MH	EXISTING MANHOLE RIM: 23.42 INV IN: 16.01						
GT-01	GREASE TRAP (1500 GAL. RIM: 23.73 INV IN: 20.37 INV OUT: 19.87						
MH-01	TYPE "A" MANHOLE RIM: 22.34 INV IN: 18.84 INV OUT: 18.74						
SSC0-01	CLEANOUT RIM: 23.96 INV OUT: 20.18						
SSC0-02	CLEANOUT RIM: 23.67 INV IN: 19.95 INV OUT: 19.95						
SSC0-03	CLEANOUT RIM: 23.61 INV IN: 19.89 INV OUT: 19.89						
SSC0-04	CLEANOUT RIM: 23.97 INV OUT: 20.50						
SSC0-05	CLEANOUT RIM: 23.94 INV IN: 20.46 INV OUT: 20.47						
SSC0-06	CLEANOUT RIM: 23.55 INV IN: 19.77 INV OUT: 19.77						
SSC0-07	CLEANOUT RIM: 23.41 INV IN: 19.70 INV IN: 19.70 INV OUT: 19.70						
SSC0-08	CLEANOUT RIM: 23.31 INV IN: 19.33 INV OUT: 19.33						
SSC0-09	CLEANOUT RIM: 22.50 INV IN: 16.49 INV OUT: 16.42						

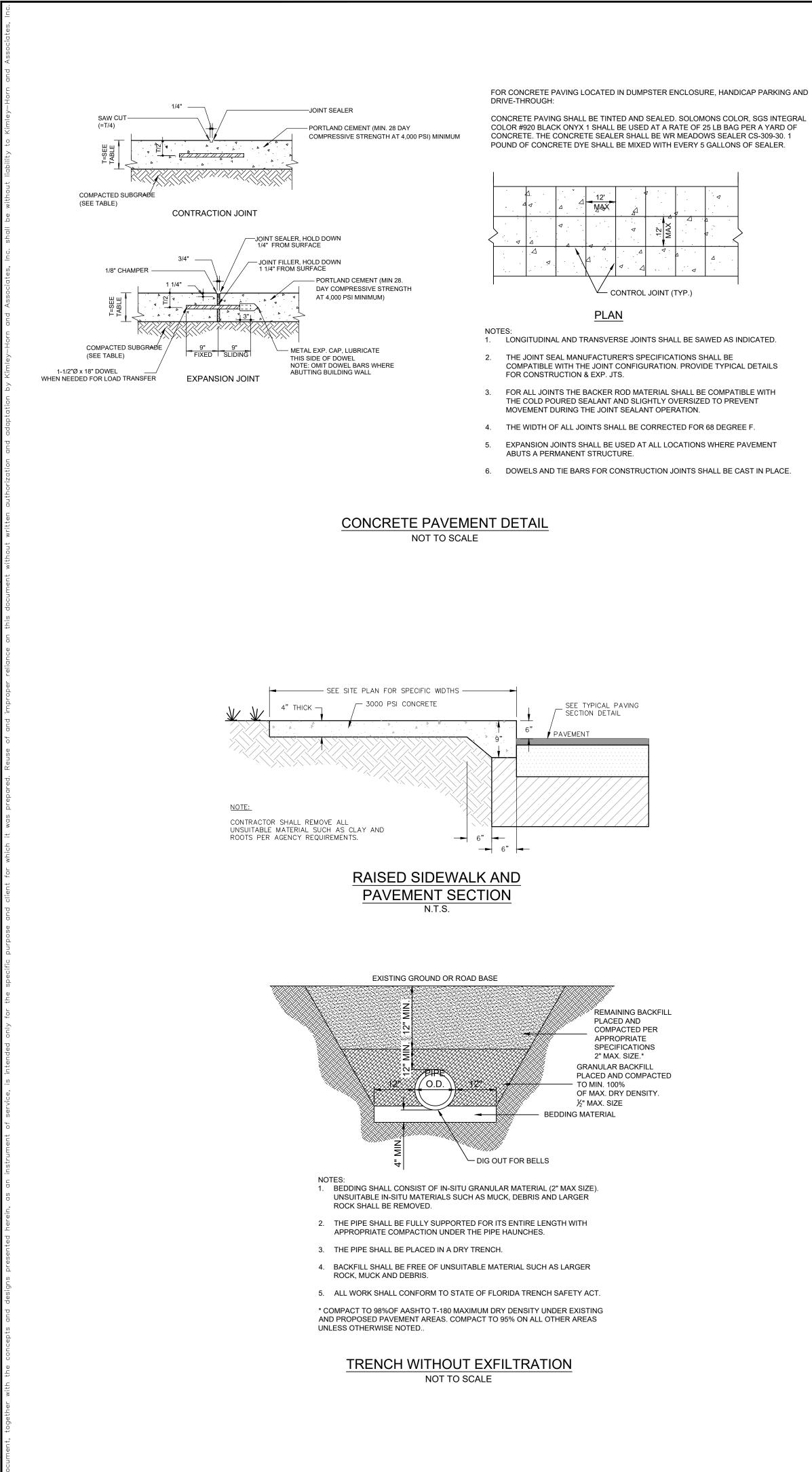
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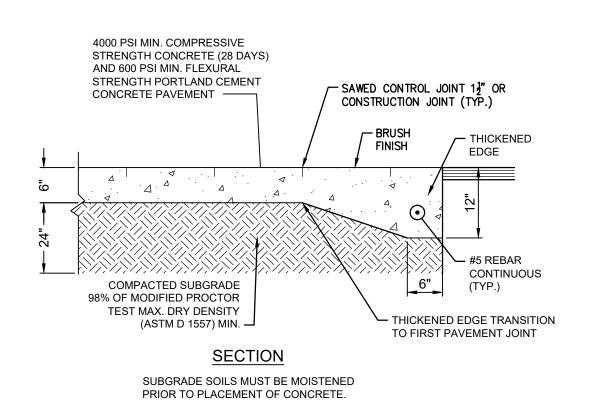


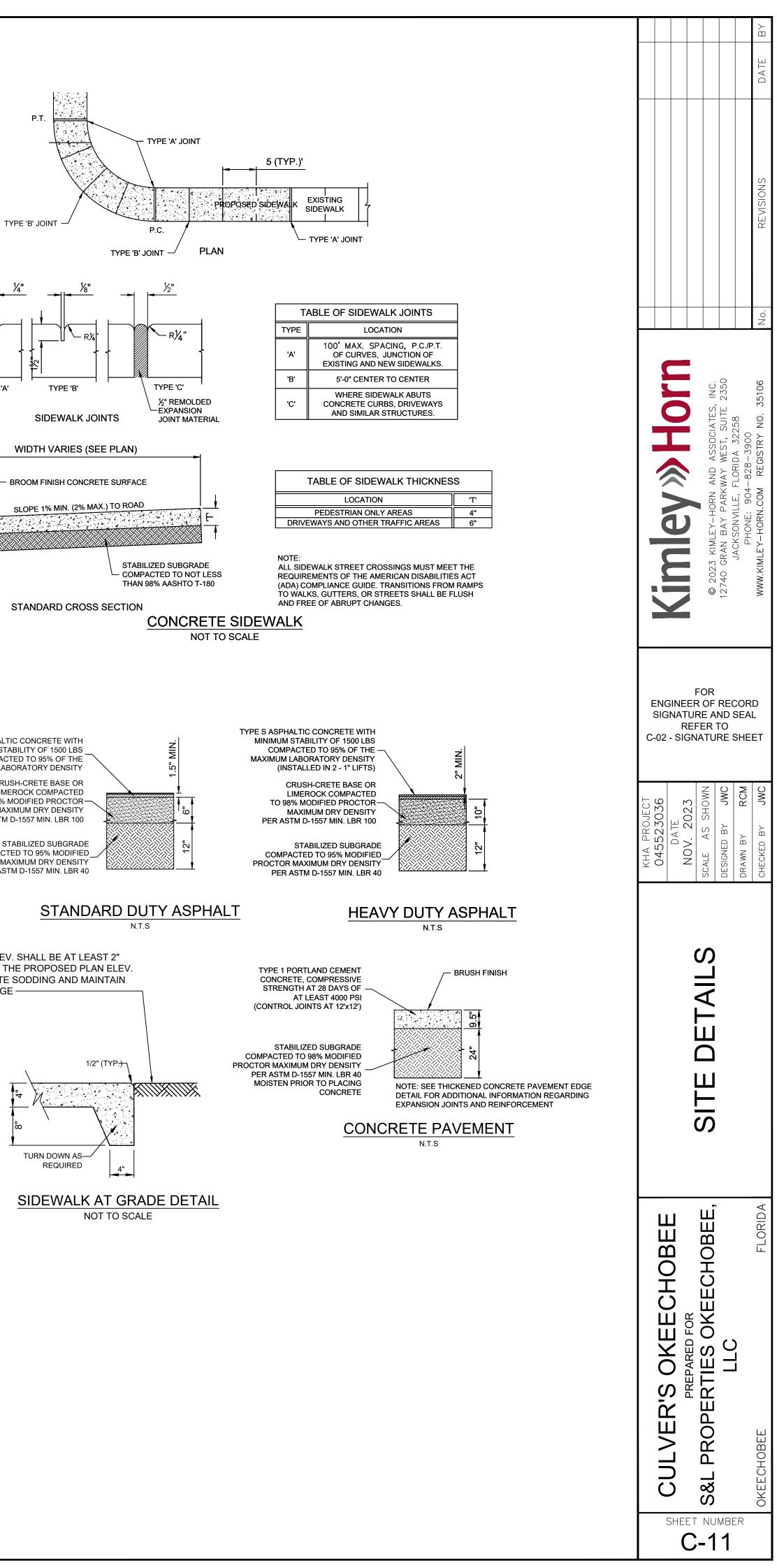




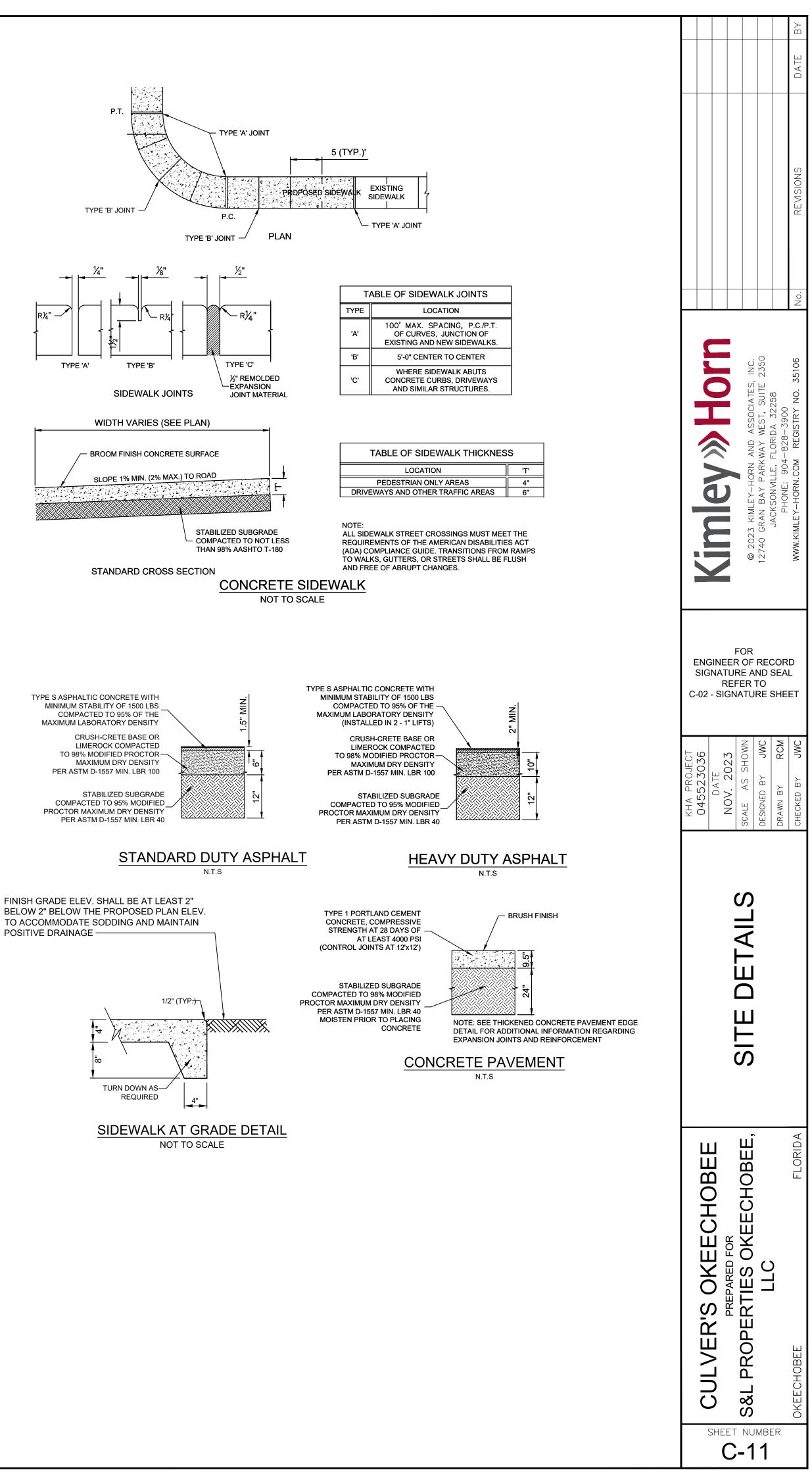
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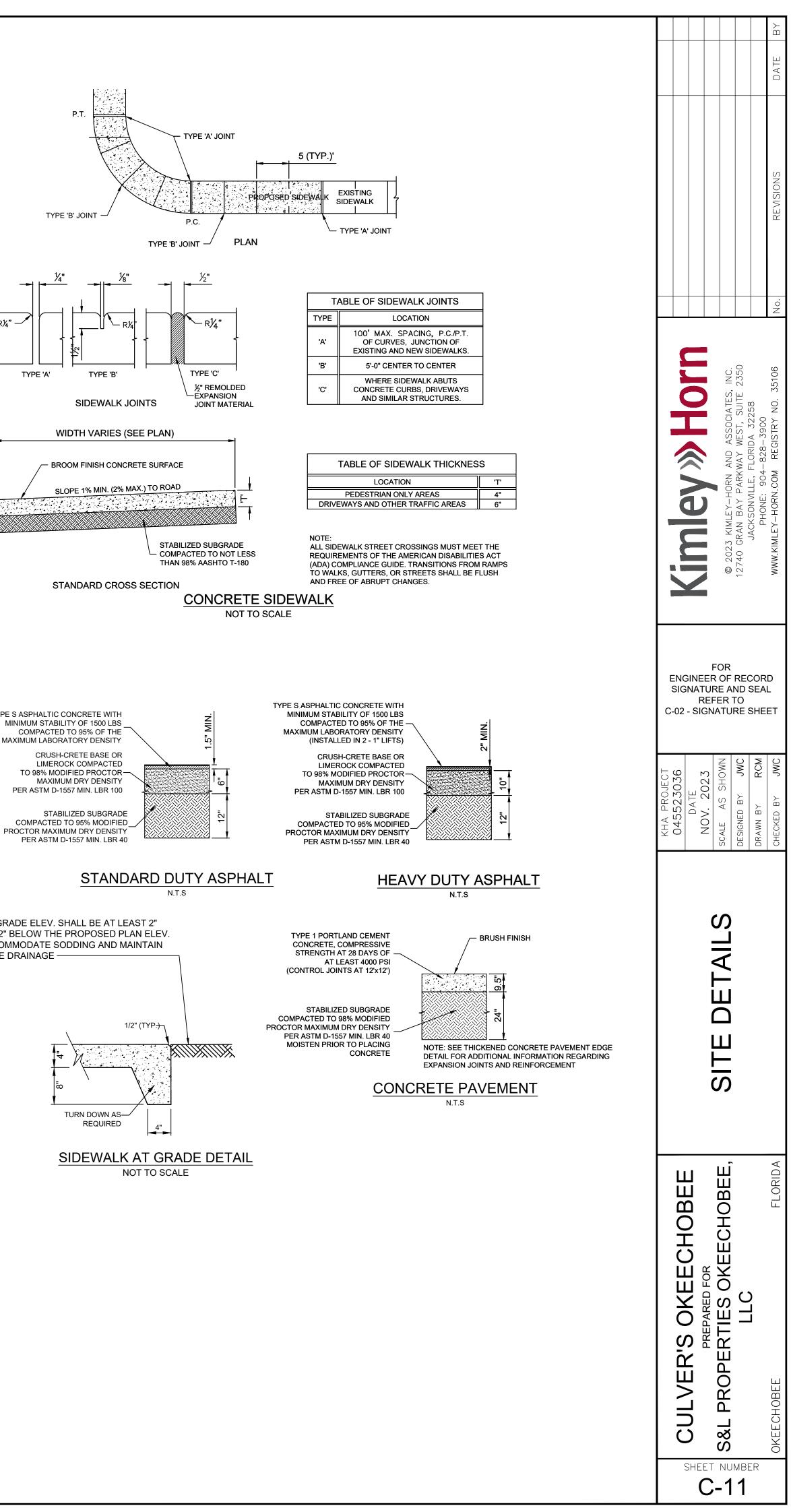


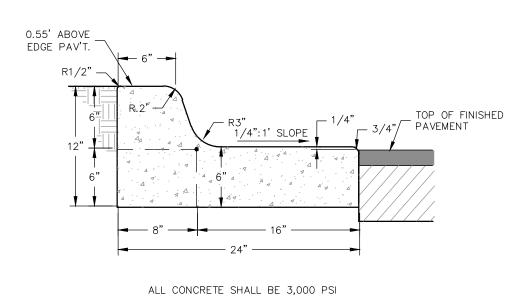




TYPE 'B' JOINT

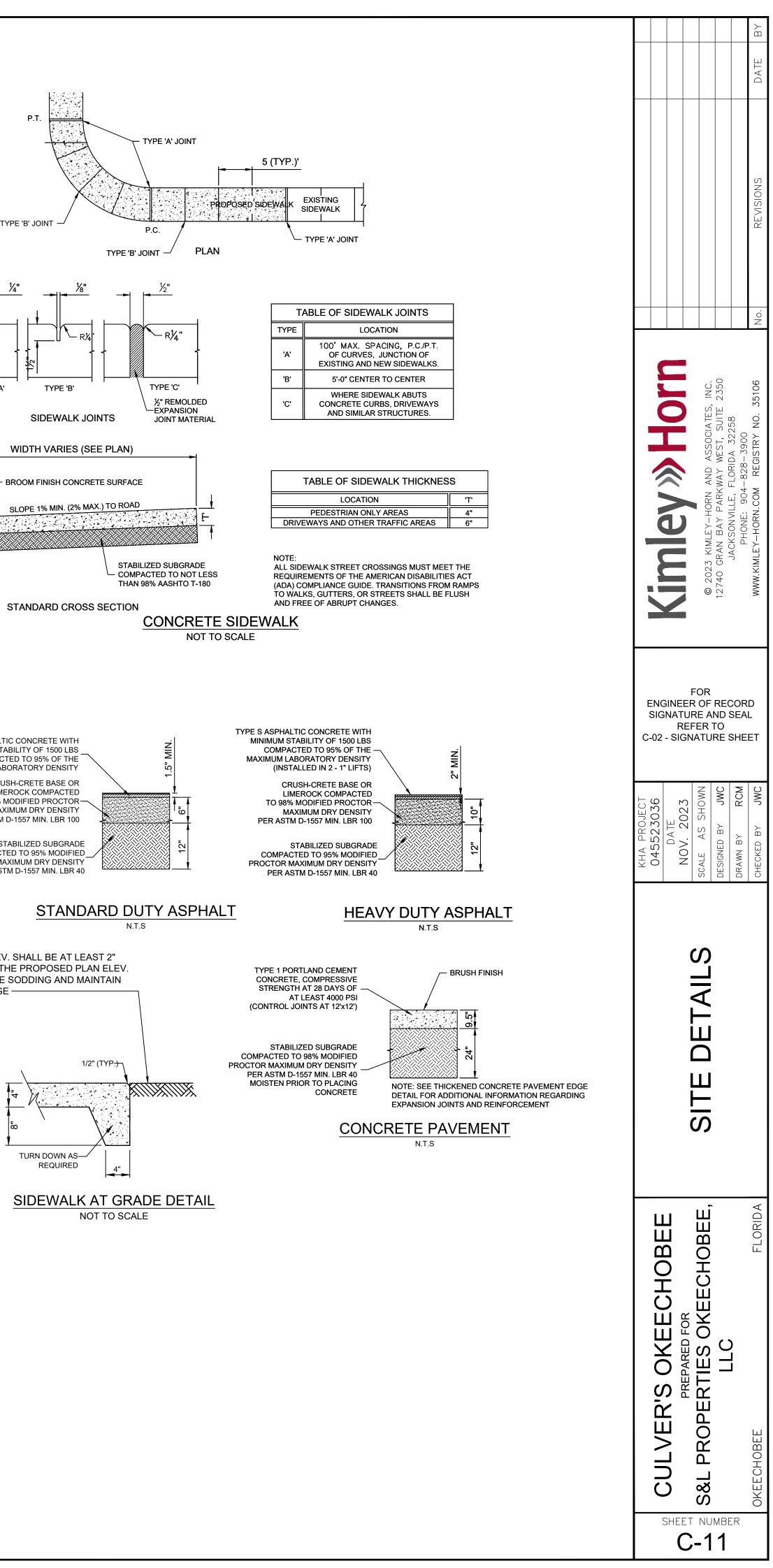


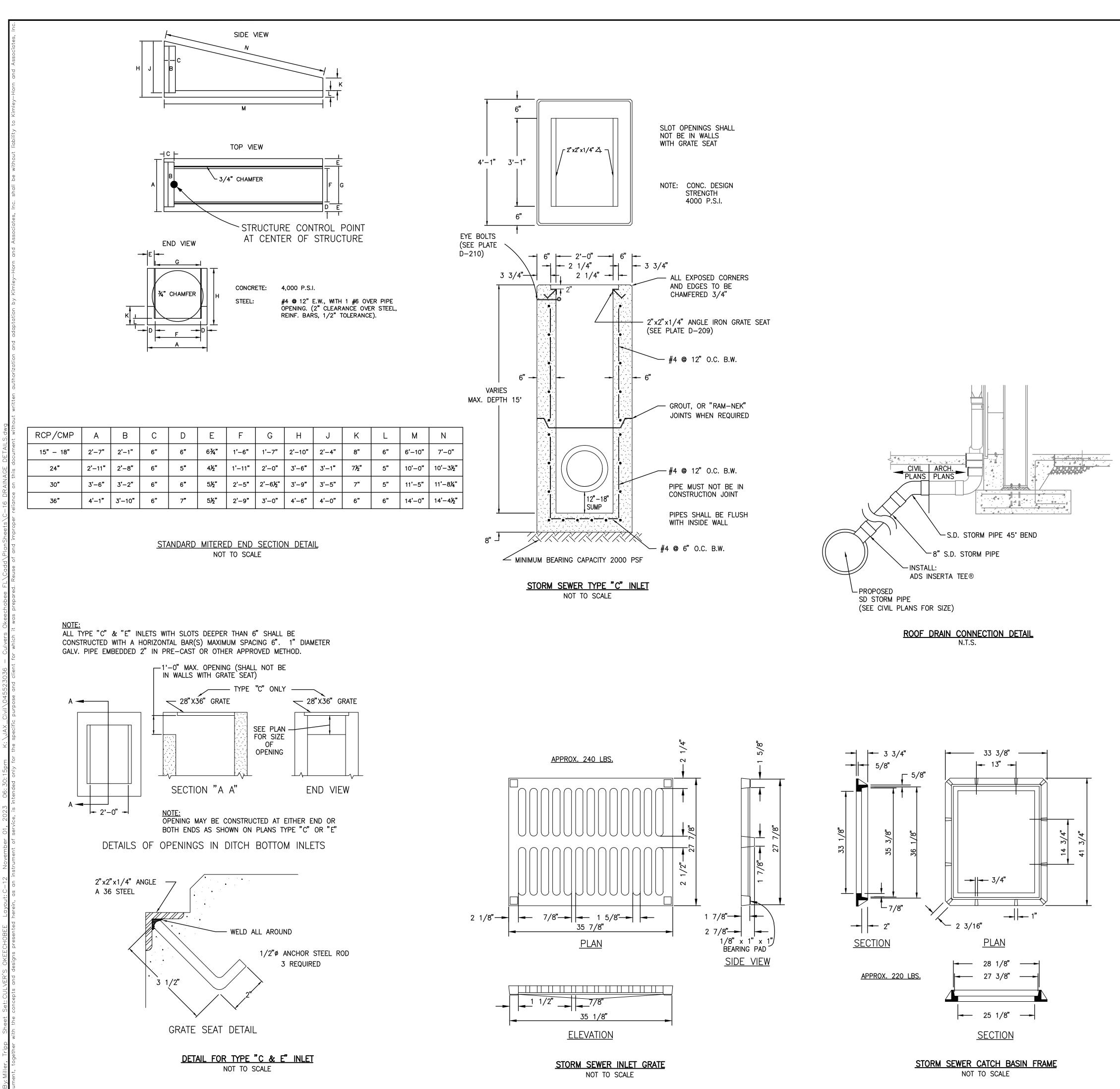




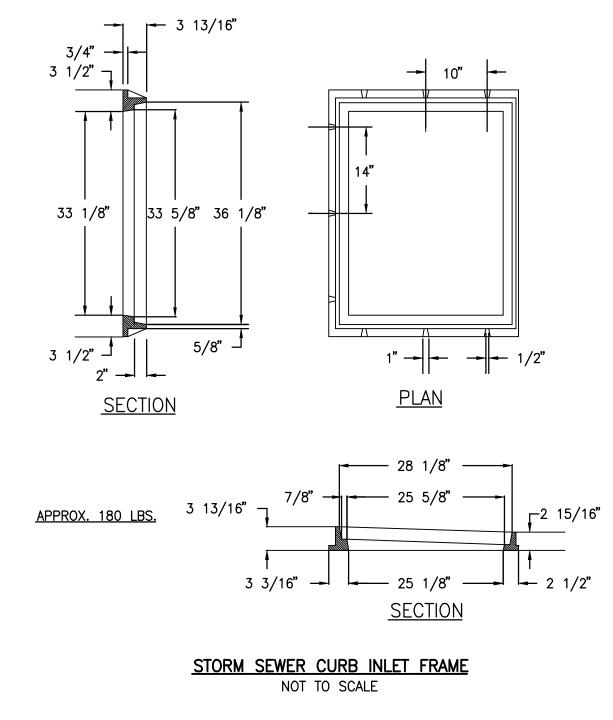
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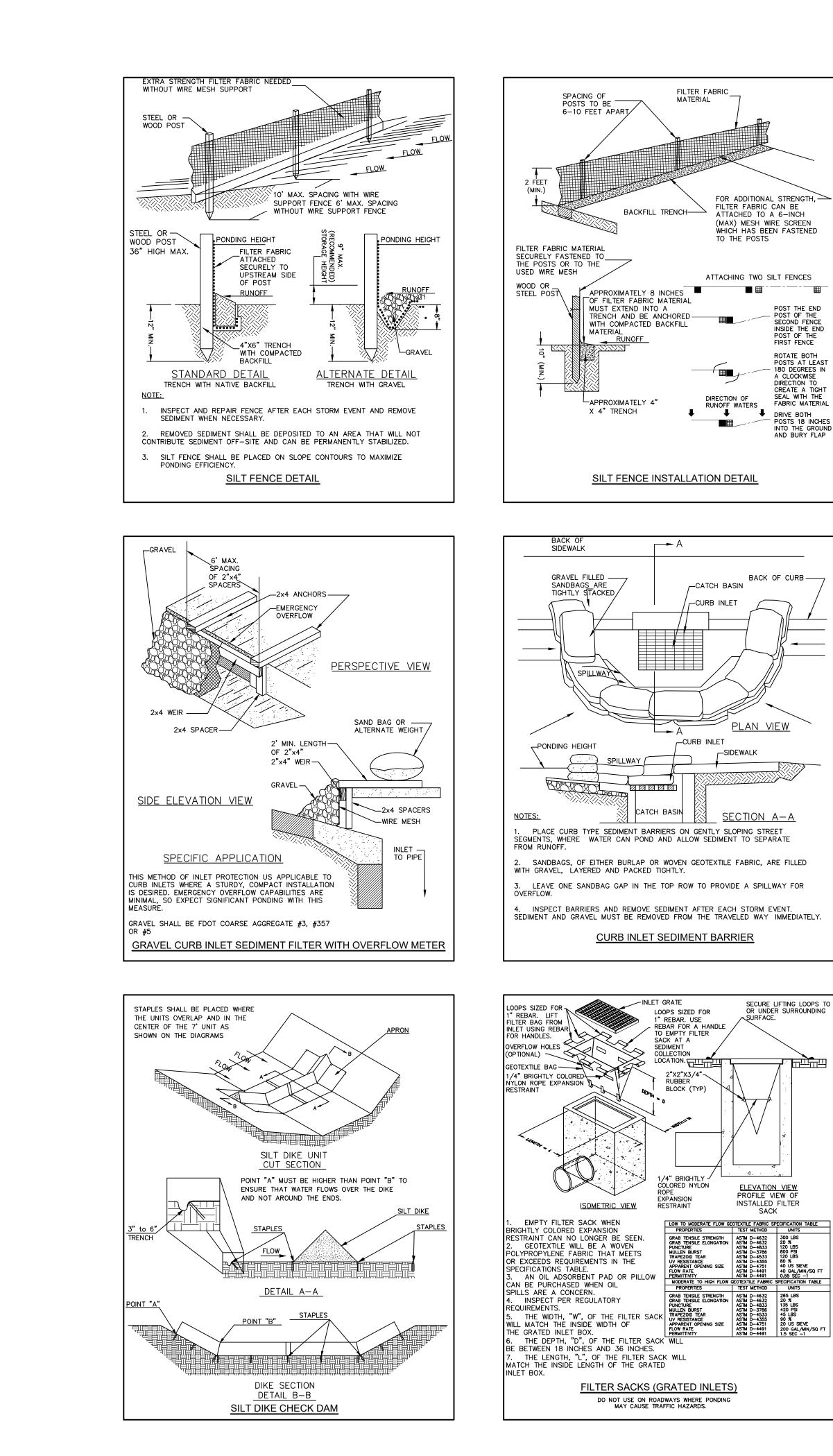
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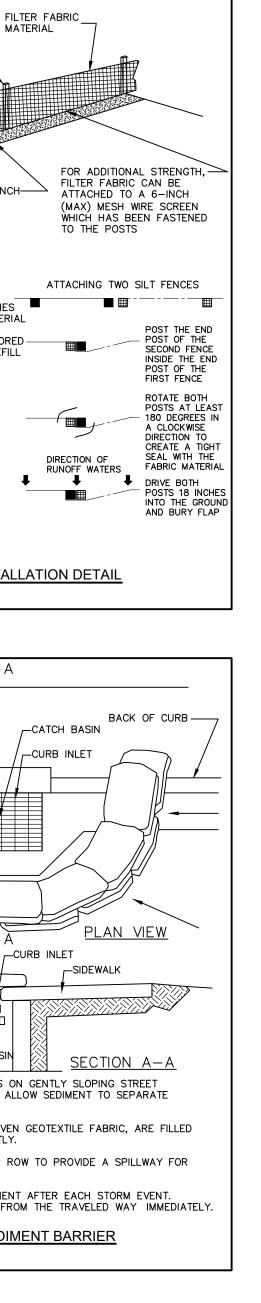


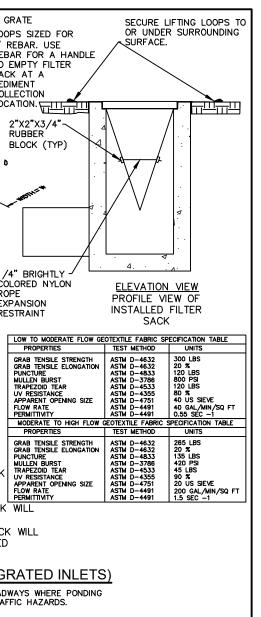


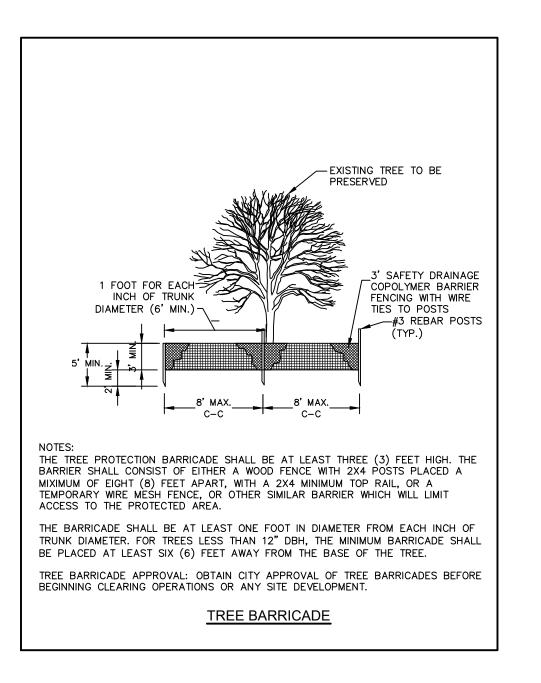
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FOR ENGINEER OF RECORD SIGNATURE AND SEAL REFER TO C-02 - SIGNATURE SHEET						
КНА РКОЈЕСТ 045523036	DATE NOV. 2023	SCALE AS SHOWN	DESIGNED BY JWC	DRAWN BY RCM	снескер ву JWC	
		DRAINAGE DETAILS				
	CULVER U CREPCECTODEE PREPARED FOR	S&L PROPERTIES OKEECHOBEE,			DKEECHOBEE FLORIDA	
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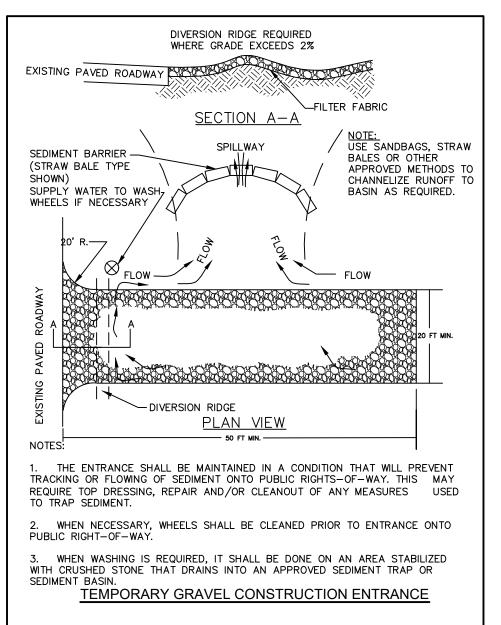


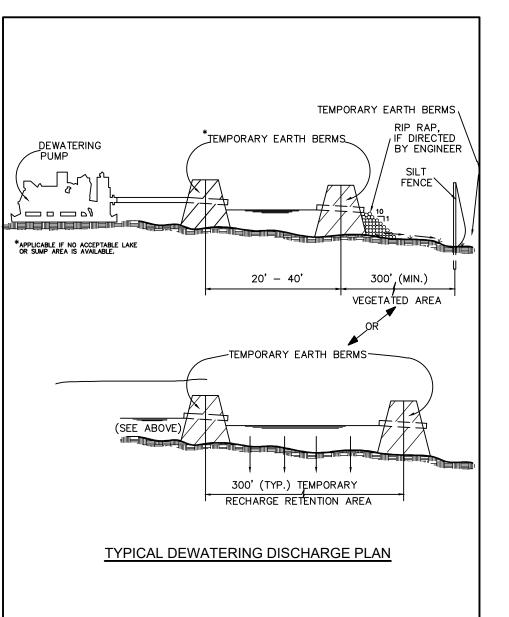


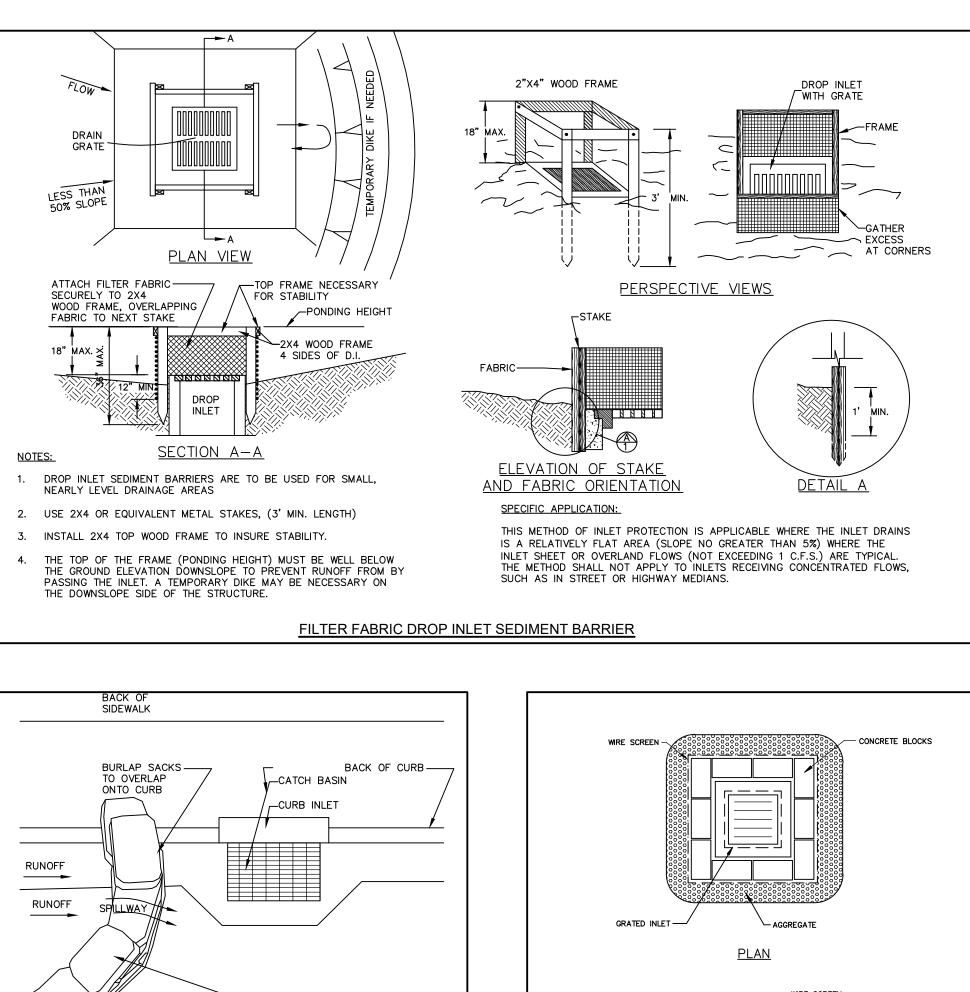


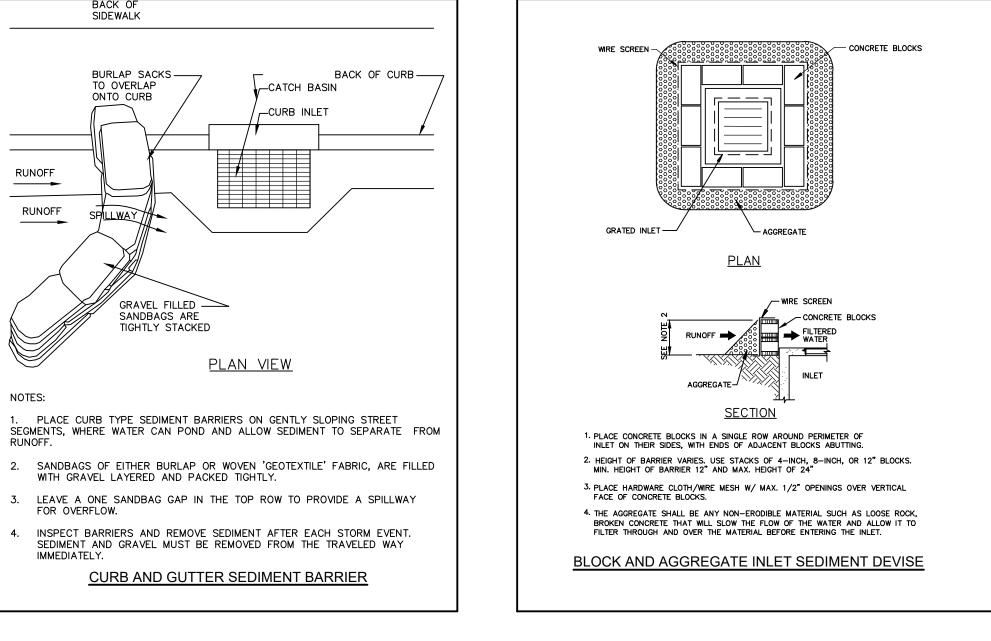


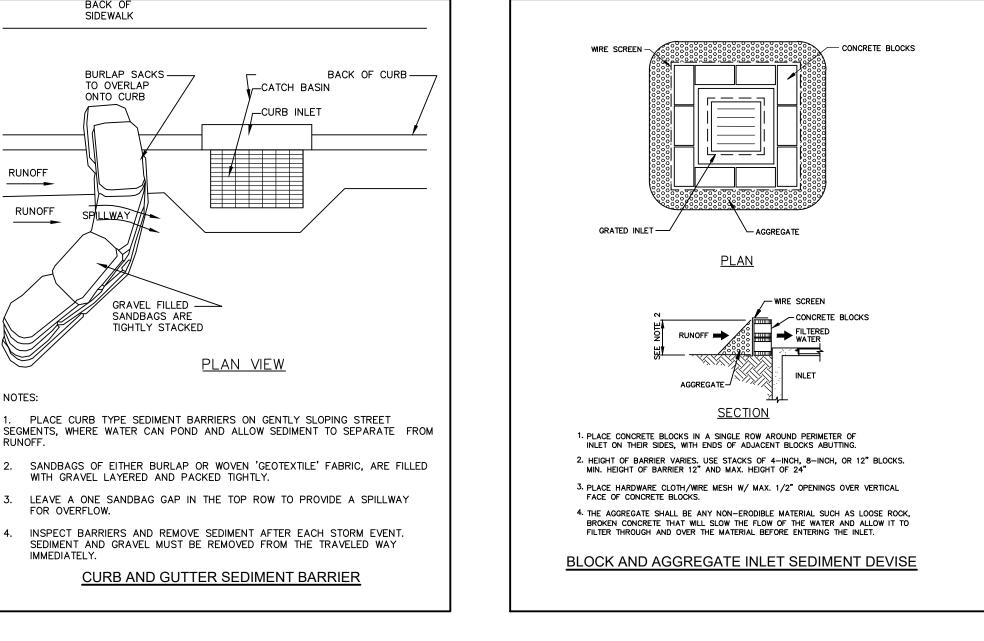


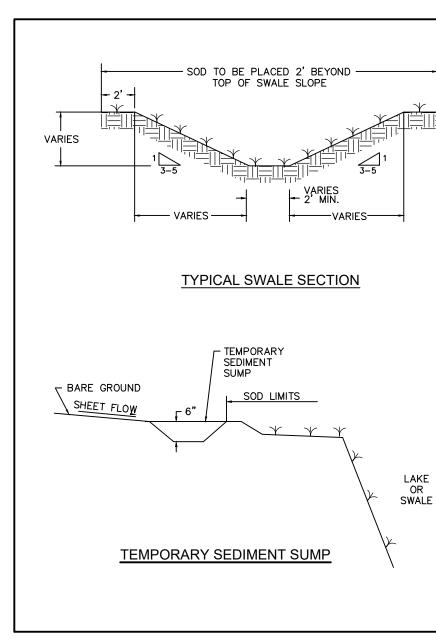


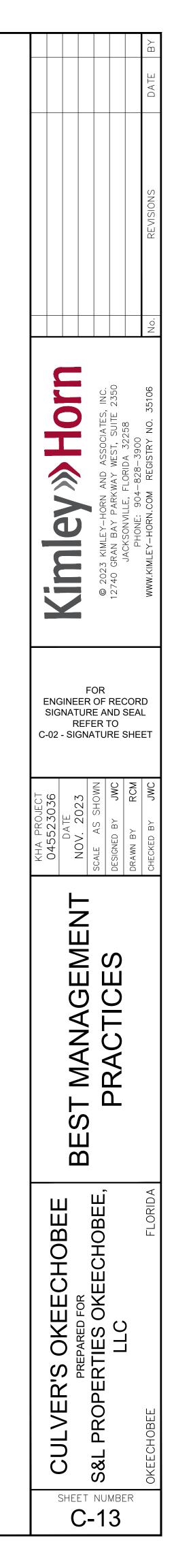












STORM WATER POLLUTION PREVENTION PLAN

SITE DESCRIPTION:

PROJECT NAME AND LOCATION:

CULVER'S OKEECHOBEE 975 NE PARK STREET OKEECHOBEE, FLORIDA 34972

*SEE COVER SHEET FOR LOCATION MAP

DEVELOPER NAME AND ADDRESS:

S&L PROPERTIES OKEECHOBEE. LLC 2651 KIRKING COURT PORTAGE, WISCONSIN 53901

PROJECT DESCRIPTION:

THE PROJECT WILL CONSIST OF PARTIAL SITE DEMOLITION. BUILDING CONSTRUCTION AND ASSOCIATED PAVING, GRADING, DRAINAGE AND UTILITY CONNECTIONS.

CONTRIBUTING DRAINAGE AREA: 2.16 ACRES

ULTIMATE RECEIVING WATERS: MASTER DEVELOPMENT POND

ACTIVITIES THAT REQUIRE EROSION CONTROL:

SITE CLEARING AND GRUBBING; PROVIDING A STABILIZED CONSTRUCTION ENTRANCE, PERIMETER, AND OTHER EROSION AND SEDIMENT CONTROLS; SITE GRADING; INSTALLATION OF STORM WATER, SANITARY SEWER, AND WATER INFRASTRUCTURE; CURB AND GUTTER, ROADWAYS, AND SIDEWALKS; LANDSCAPING.

*SEE PLANS FOR THE LOCATION OF TEMPORARY SEDIMENT BARRIERS AND OTHER EROSION CONTROL METHODS.

SUGGESTED SEQUENCE OF MAJOR ACTIVITIES:

THE ORDER OF SITE CONSTRUCTION ACTIVITIES IS AS FOLLOWS:

- PROVIDE STABILIZED CONSTRUCTION ENTRANCE
- INSTALL SILT FENCES AND OTHER EROSION CONTROL MEASURES
- CLEAR AND GRUB FOR SEDIMENT BASIN/STORMWATER POND/ EARTH DIKE
- CONSTRUCT STORMWATER PONDS, EARTH DIKE, AND SEDIMENT BASINS FINISH CLEARING AND GRUBBING OF SITE
- REMOVE AND STORE TOPSOIL
- PROVIDE INITIAL GRADING AS REQUIRED
- 8. STABILIZE ALL DISTURBED AREAS AS SOON AS POSSIBLE
- INSTALL UTILITIES, STORM SEWER, CURB AND GUTTER 9.
- 10. INSTALL BASE TO ROAD AND PARKING AREA 11. FINISH GRADING ENTIRE SITE
- 12. PROVIDE REQUIRED LANDSCAPING
- 13. CONSTRUCT PAVING
- 14. REMOVE ACCUMULATED SEDIMENT
- 15. REMOVE ANY ITEMS THAT ARE NOT REQUIRED

THE SEQUENCE OF CONSTRUCTION SHOWN ABOVE IS A GENERAL OVERVIEW AND IS INTENDED TO CONVEY THE GENERAL CONCEPTS OF THE EROSION CONTROL DESIGN AND SHOULD NOT BE RELIED UPON FOR CONSTRUCTION PURPOSES. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR DETAILED PHASING AND CONSTRUCTION SEQUENCING NECESSARY TO CONSTRUCT THE PROPOSED IMPROVEMENTS INCLUDED IN THESE PLANS. THE CONTRACTOR SHALL NOTIFY ENGINEER IN WRITING, IMMEDIATELY, PRIOR TO AND/OR DURING CONSTRUCTION IF ANY ADDITIONAL INFORMATION ON THE CONSTRUCTION SEQUENCE IS NECESSARY. CONTRACTOR IS SOLELY RESPONSIBLE FOR COMPLYING WITH THE REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION AND ALL OTHER APPLICABLE LAWS.

- FURTHERMORE: ENGINEER HAS NO CONTROL OVER OR RESPONSIBILITY FOR THE CONSTRUCTION MEANS AND METHODS OF THIS PROJECT. ANY TURBIDITY OR SEDIMENT RELEASES FROM THE PROJECT ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- THIS DOCUMENT AN ITS CONTENTS ARE PROVIDED FOR GENERAL SWPPP GUIDANCE AND FOR CONTRACTOR CONVINCE.
- CONTRACTOR IS RESPONSIBLE FOR IMPLEMENTATION AND MAINTENANCE OF THE SWPPP AT TIMES.
- CONTRACTOR SHALL BE PREPARED TO USE ANY NECESSARY METHODS FOR TURBIDITY CONTROL FROM THE SITE, INCLUDING POTENTIAL USE OF CHEMICAL DOSING/ADDITIVES (I.E. POLYACRYLAMIDE) TO ACHIEVE THE ACCEPTABLE EFFLUENT CRITERIA LEAVING THE SITE.

TEMPORARY STABILIZATION - TOPSOIL STOCK PILES AND DISTURBED PORTIONS THE FOLLOWING ITEMS ARE EXPECTED TO BE PRESENT ON THE PROJECT SITE: ASPHALT, CONCRETE, CLEANING SUPPLIES, DETERGENTS, FERTILIZERS, MASONARY OF THE SITE WHERE CONSTRUCTION ACTIVITY TEMPORARILY CEASE FOR AT LEAST 21 DAYS, SHALL BE STABILIZED WITH TEMPORARY SEED AND MULCH BLOCKS/BRICKS, METAL PIECES, PAINT, PETROLEUM BASED PRODUCTS, ROOFING WITHIN 14 DAYS OF THE LAST CONSTRUCTION ACTIVITY IN THAT AREA. THE SUPPLIES, TAR, WOOD. TEMPORARY SEED REQUIRED CAN BE FOUND IN TABLE 1.65 A OF THE FLORIDA DEVELOPMENT MANUAL. PRIOR TO SEEDING, WHERE SOILS ARE ACIDIC 2 TONS THE FOLLOWING ARE NON-STORM WATER SOURCES THAT WILL BE ENCOUNTERED OF PULVERIZED AGRICULTURAL LIMESTONE SHOULD BE ADDED PER ACRE AND AT THE SITE AND SHOULD BE DIRECTED TO THE SEDIMENT BASIN PRIOR TO 450 POUNDS OF 10-20-20 FERTILIZER SHALL BE APPLIED TO EACH ACRE. DISCHARGE: AFTER SEEDING, EACH AREA SHALL BE IMMEDIATELY MULCHED WITH STRAW OR -UNCONTAMINATED GROUNDWATER EXPOSED DURING EXCAVATION EQUIVALENT EQUAL. AREAS OF THE SITE WHICH ARE TO BE PAVED SHALL BE -WATER FROM WATER LINE FLUSHING TEMPORARILY STABILIZED BY APPLYING GEOTEXTILE AND STONE SUB-BASE -PAVEMENT WASH WATERS (WHERE NO SPILLS OR LEAKS OF TOXIC OR UNTIL BITUMINOUS PAVEMENT CAN BE APPLIED.

HAZARDOUS MATERIALS HAVE OCCURRED). SPILL PREVENTION AND CONTROL:

PERMANENT STABILIZATION - DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES PERMANENTLY CEASES SHALL BE STABILIZED WITH PERMANENT SEED NO LATER THAN 14 DAYS AFTER THE LAST CONSTRUCTION THE FOLLOWING ARE THE MATERIAL MANAGEMENT PRACTICES THAT WILL BE USED ACTIVITY. THE APPROPRIATE PERMANENT SEED MIX CAN BE FOUND IN TABLES TO REDUCE THE RISK OF SPILLS OR OTHER ACCIDENTAL EXPOSURE OF 1.66A, 1.66B AND 1.66C OF THE FLORIDA DEVELOPMENT MANUAL. PRIOR TO MATERIALS AND SUBSTANCES TO STORM WATER RUNOFF. SEEDING, 2 TONS/ACRE OF FINELY GROUND AGRICULTURAL LIMESTONE AND THE PROPER FERTILIZER BASED ON THE TYPE OF SEEDING SHALL BE APPLIED TO GOOD HOUSEKEEPING: EACH ACRE TO PROVIDE PLANT NUTRIENTS. AFTER SEEDING, EACH AREA SHALL BE MULCHED IMMEDIATELY.

STRUCTURAL PRACTICES:

EARTH DIKE - IF REQUIRED, AN EARTH DIKE SHALL BE CONSTRUCTED ALONG -ALL CONSTRUCTION MATERIALS STORED SHALL BE ORGANIZED AND IN THE THE SITE PERIMETER. A PORTION OF THE DIKE SHALL DIVERT RUN-ON PROPER CONTAINER AND IF POSSIBLE, STORED UNDER A ROOF OR PROTECTIVE AROUND THE CONSTRUCTION SITE. THE REMAINING PORTION OF THE DIKE COVER SHALL COLLECT RUNOFF FROM THE DISTURBED AREA AND DIRECT THE RUNOFF -PRODUCTS SHALL NOT BE MIXED UNLESS DIRECTED BY THE MANUFACTURER. TO THE SEDIMENT BASIN. -ALL PRODUCTS SHALL BE USED AND DISPOSED OF ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS.

SEDIMENT BASIN - A SEDIMENT BASIN SHALL BE CONSTRUCTED IN THE COMMON DRAINAGE AREA FOR THE SITE. THIS STORAGE POND SHALL HAVE A VOLUME OF 3,600 CUBIC FEET OF STORAGE FOR EACH ACRE. ALL SEDIMENT COLLECTED IN THE BASIN MUST BE REMOVED FROM THE BASIN UPON COMPLETION OF CONSTRUCTION. SEDIMENT FROM THE BASIN MAY BE USED AS FILL ON THE SITE IF IT IS SUITABLE SOIL.

WASTE DISPOSAL:

WASTE MATERIALS - ALL WASTE MATERIALS SHALL BE COLLECTED AND STORED IN A METAL DUMPSTER WITH A SECURE LID IN ACCORDANCE WITH ALL LOCAL AND STATE LAWS. ALL TRASH AND CONSTRUCTION DEBRIS FROM THE SITE SHALL BE DEPOSITED IN THE DUMPSTER. THE SUPERINTENDENT SHALL COORDINATE WITH THE LOCAL UTILITIES TO HAVE THE DUMPSTER EMPTIED AT LEAST TWICE A WEEK AND THE WASTE TAKEN TO AN APPROPRIATE LANDFILL. NO CONSTRUCTION WASTE MATERIALS SHALL BE BURIED ON SITE. THE SUPERINTENDENT SHALL ORGANIZE TRAINING FOR THE EMPLOYEES IN THE PROPER PRACTICES WHEN DEALING WITH WASTE MATERIALS. THE SUPERINTENDENT SHALL BE RESPONSIBLE FOR POSTING AND ENFORCING WASTE MATERIAL PROCEDURES.

HAZARDOUS WASTE - HAZARDOUS WASTE MATERIALS SHALL BE DISPOSED OF IN ACCORDANCE WITH ALL LOCAL AND STATE LAWS OR AS DIRECTED BY THE MANUFACTURER. THE SUPERINTENDENT SHALL ORGANIZE THE PROPER TRAINING FOR EMPLOYEES IN THE PROPER PRACTICES WHEN DEALING WITH HAZARDOUS WASTE MATERIALS. THESE PROCEDURES SHALL BE POSTED ON THE SITE. THE PERSON WHO MANAGES THE SITE SHALL BE RESPONSIBLE FOR ENFORCING THE PROCEDURES.

SANITARY WASTE - SANITARY WASTE SHALL BE COLLECTED AND DISPOSED OF IN ACCORDANCE WITH ALL LOCAL AND STATE LAWS. THE SUPERINTENDENT CLEANUP: SHALL COORDINATE WITH THE LOCAL UTILITY FOR COLLECTION OF THE -SPILL CLEANUP INFORMATION SHALL BE POSTED ON SITE TO INFORM SANITARY WASTE AT LEAST THREE TIMES A WEEK TO PREVENT SPILLAGE ONTO EMPLOYEES ABOUT CLEANUP PROCEDURES AND RESOURCES. THE SITE.

OFFSITE TRACKING:

A STABILIZED CONSTRUCTION ENTRANCE SHALL BE PROVIDED TO REDUCE -WHEN CLEANING A SPILL, THE AREA SHOULD BE WELL VENTILATED AND THE SEDIMENT TRACKING OFFSITE. THE MAJOR ROAD CONNECTED TO THE PROJECT EMPLOYEE SHALL WEAR PROPER PROTECTIVE COVERING TO PREVENT INJURY. SHALL BE CLEANED ONCE A DAY TO REMOVE ANY EXCESS MUD, DIRT OR ROCK -TOXIC SPILLS MUST BE REPORTED TO THE PROPER AUTHORITY REGARDLESS OF RESULTING FROM CONSTRUCTION TRAFFIC. ALL TRUCKS HAULING MATERIALS THE SIZE OF THE SPILL. OFFSITE SHALL BE COVERED WITH A TARPAULIN.

TIMING OF CONTROL MEASURES:

THE INSTALLATION OF SILT FENCE (AND OTHER EROSION CONTROL MEASURES), A STABILIZED ENTRANCE AND SEDIMENT BASIN SHALL OCCUR PRIOR TO CLEARING AND GRUBBING ACTIVITY. AFTER CONSTRUCTION IS COMPLETE, THE ACCUMULATED SEDIMENT SHALL BE REMOVED AND THE AREAS SHALL BE REGRADED AND PERMANENTLY STABILIZED AS SHOWN ON THE PLANS.

STABILIZATION PRACTICES:

ITEMS REQUIRING POLLUTION PREVENTION:

-SUPERINTENDENT SHALL INSPECT PROJECT AREA DAILY FOR PROPER STORAGE. USE, AND DISPOSAL OF CONSTRUCTION MATERIALS.

-STORE ONLY ENOUGH MATERIAL ON SITE FOR PROJECT COMPLETION. -ALL SUBSTANCES SHOULD BE USED BEFORE DISPOSAL OF CONTAINER.

HAZARDOUS PRODUCTS:

-MATERIALS SHOULD BE KEPT IN ORIGINAL CONTAINER WITH LABELS UNLESS THE ORIGINAL CONTAINERS CANNOT BE RESEALED. IF ORIGINAL CONTAINERS CANNOT BE USED, LABELS AND PRODUCT INFORMATION SHALL BE SAVED. -PROPER DISPOSAL PRACTICES SHALL ALWAYS BE FOLLOWED IN ACCORDANCE WITH MANUFACTURER AND LOCAL/STATE REGULATIONS.

PRODUCT SPECIFIC PRACTICES:

- -PETROLEUM PRODUCTS MUST BE STORED IN PROPER CONTAINERS AND CLEARLY LABELED. VEHICLES CONTAINING PETROLEUM PRODUCTS SHALL BE PERIODICALLY INSPECTED FOR LEAKS. PRECAUTIONS SHALL BE TAKEN TO AVOID LEAKAGE OF PETROLEUM PRODUCTS ON SITE.
- -THE MINIMUM AMOUNT OF FERTILIZER SHALL BE USED AND MIXED INTO THE SOIL IN ORDER TO LIMIT EXPOSURE TO STORM WATER. FERTILIZERS SHALL BE STORED IN A COVERED SHED. THE CONTENTS OF ANY PARTIALLY USED BAGS OF FERTILIZER SHALL BE TRANSFERRED TO A SEALABLE PLASTIC BIN TO AVOID SPILLS.

-PAINT CONTAINERS SHALL BE SEALED AND STORED WHEN NOT IN USE. EXCESS PAINT MUST BE DISPOSED OF IN AN APPROVED MANNER. -CONCRETE TRUCKS SHALL NOT BE ALLOWED TO WASH OUT OR DISCHARGE SURPLUS CONCRETE OR DRUM WASH WATER ON THE SITE.

SPILL CONTROL PRACTICES:

IN ADDITION TO THE GOOD HOUSEKEEPING AND MATERIAL MANAGEMENT PRACTICES DISCUSSED IN THE PREVIOUS SECTIONS OF THIS PLAN, THE FOLLOWING PRACTICES SHALL BE FOLLOWED FOR SPILL PREVENTION AND

- -THE FOLLOWING CLEAN-UP EQUIPMENT MUST BE KEPT ON-SITE NEAR THE MATERIAL STORAGE AREA: GLOVES, MOPS, RAGS, BROOMS, DUST PANS, SAND, SAWDUST, LIQUID ABSORBER, GOGGLES, AND TRASH CONTAINERS.
- -ALL SPILLS SHALL BE CLEANED UP AS SOON AS POSSIBLE.
- -AFTER A SPILL, THE PREVENTION PLAN SHALL BE REVIEWED AND CHANGED TO PREVENT FURTHER SIMILAR SPILLS FROM OCCURRING. THE CAUSE OF THE SPILL. MEASURES TO PREVENT IT. AND HOW TO CLEAN THE SPILL UP SHALL BE RECORDED.

-THE SUPERINTENDENT SHALL BE THE SPILL PREVENTION AND CLEANUP COORDINATOR AND IS RESPONSIBLE FOR THE DAY TO DAY SITE OPERATIONS. THE SUPERINTENDENT ALSO OVERSEES THE SPILL PREVENTION PLAN AND SHALL BE RESPONSIBLE FOR EDUCATING THE EMPLOYEES ABOUT SPILL PREVENTION AND CLEANUP PROCEDURES.

EROSION AND SEDIMENT CONTROLS:

BEST MANAGEMENT PRACTICES SHALL BE USED FOR THIS PROJECT TO CONTROL EROSION AND TURBIDITY CAUSED BY STORM WATER RUN-OFF. THE LOCATION AND DETAILS OF EROSION CONTROL METHODS ARE SHOWN ON THE PLANS. THE SUB-CONTRACTOR(S) IS/ARE RESPONSIBLE FOR PLACING AND MAINTAINING THESE CONTROL METHODS AS SHOWN ON THE PLANS OR AS REQUIRED. HE/SHE SHALL ALSO PROVIDE THE REQUIRED EROSION PROTECTION AS REQUIRED BY LOCAL, STATE AND FEDERAL LAW. OFFSITE SHALL BE COVERED WITH A TARPAULIN.

STORM WATER MANAGEMENT:

STORM WATER COLLECTION SHALL BE PROVIDED BY CURB AND GUTTER AND DRAINAGE INLETS FOR THE DEVELOPED AREAS. THE AREAS THAT ARE REGRADED BUT NOT DEVELOPED SHALL BE STABILIZED IMMEDIATELY AFTER REGRADING. THE ENTIRE SITE SHALL DRAIN TO A SWMF. THE SWMF SHALL BE IN THE AREA OF THE TEMPORARY SEDIMENT BASINS. WHEN SLOPES HAVE BEEN STABILIZED, THE SEDIMENT THAT HAS ACCUMULATED SHALL BE REMOVED FROM THE BASIN AND THE APPROPRIATE VEGETATION SHALL BE PLANTED.

MAINTENANCE AND INSPECTION PRACTICES

THE FOLLOWING ARE MAINTENANCE AND INSPECTION PRACTICES THAT SHALL BE COMPLETED BY THE CONTRACTOR:

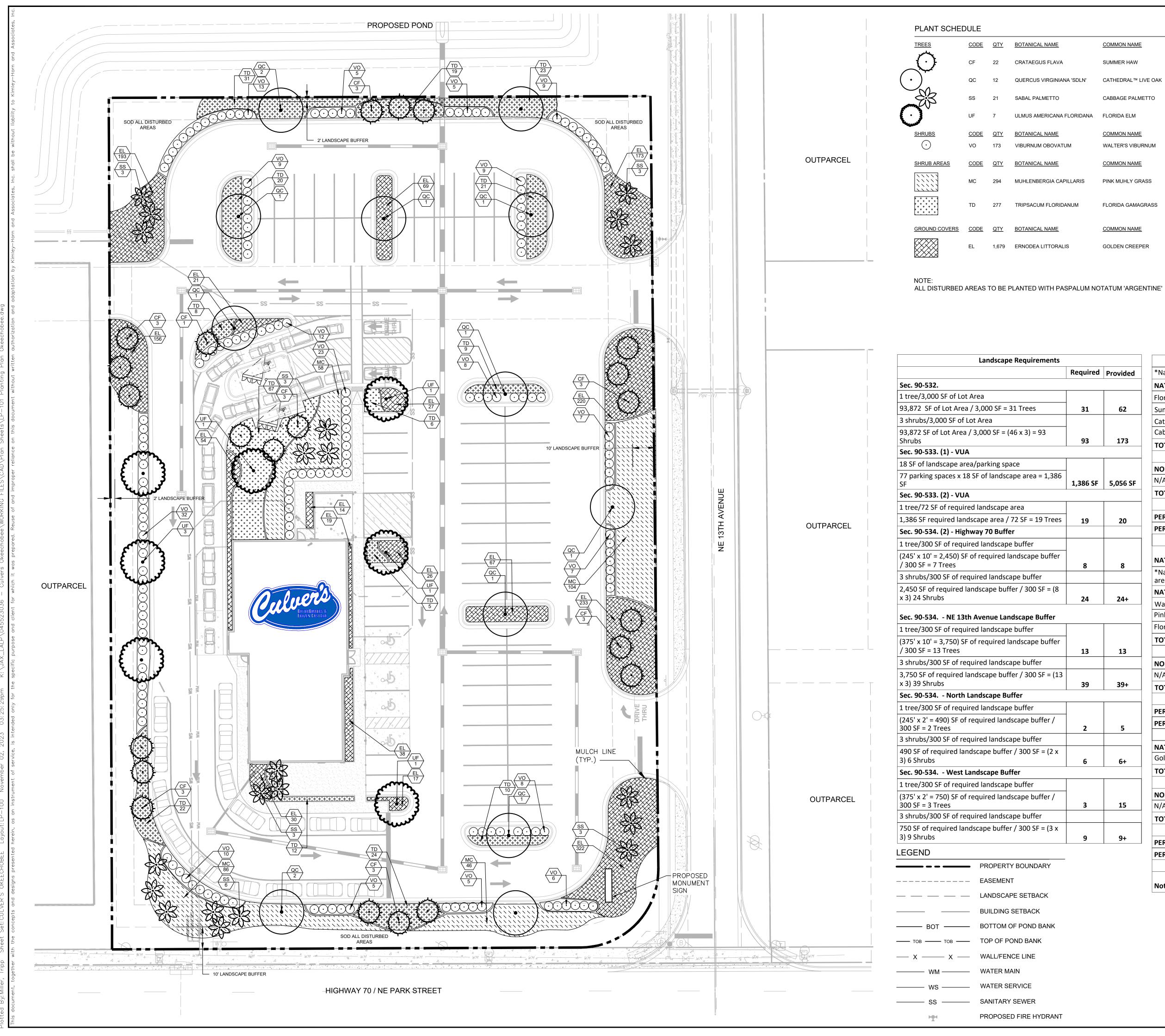
- -ALL SEDIMENT AND EROSION CONTROL METHODS SHALL BE CHECKED DAILY AND AFTER EACH SIGNIFICANT RAINFALL BY THE SUPERINTENDENT OR SOMEONE UNDER HIS/HER DIRECT SUPERVISION.
- -ALL SEDIMENT AND EROSION CONTROL METHODS SHALL BE KEPT IN GOOD CONDITION. REPAIRS MUST BE MADE WITHIN 24 HOURS OF REPORT. -THE SILT FENCE SHALL BE CLEARED OF SEDIMENT WHEN SEDIMENT MEASURES ONE-THIRD THE HEIGHT OF THE FENCE.
- -THE SILT FENCE SHALL BE INSPECTED PERIODICALLY FOR HEIGHT OF SEDIMENT AND CONDITION OF FENCE.
- -THE SEDIMENT BASINS/DITCHES SHALL BE CHECKED MONTHLY FOR DEPTH OF SEDIMENT. THEY SHALL BE CLEANED WHEN SEDIMENT REACHES 10% OF TOTAL CAPACITY AND AFTER CONSTRUCTION IS COMPLETE.
- -THE DIVERSION DIKE SHALL BE INSPECTED MONTHLY. ANY BREACHES SHALL BE PROMPTLY REPAIRED.
- -ALL SEEDING SHALL BE CHECKED FOR PROPER GROWTH AND UNIFORMITY. UNSTABALIZED AREAS SHALL BE RE-SODDED
- -A MAINTENANCE REPORT SHALL BE COMPLETED DAILY AFTER EACH INSPECTION OF THE SEDIMENT AND EROSION CONTROL METHODS. THE REPORTS SHALL BE FILED IN AN ORGANIZED MANNER AND RETAINED ON-SITE DURING CONSTRUCTION. AFTER CONSTRUCTION IS COMPLETED, THE REPORTS SHALL BE SAVED FOR AT LEAST THREE YEARS. THE REPORTS SHALL BE AVAILABLE FOR ANY AGENCY THAT HAS JURISDICTION OVER EROSION CONTROL.
- -THE SUPERINTENDENT SHALL ORGANIZE THE TRAINING FOR INSPECTION PROCEDURES AND PROPER EROSION CONTROL METHODS FOR EMPLOYEES THAT COMPLETE INSPECTIONS AND REPORTS.

CONTRACTOR'S CERTIFICATION

I CERTIFY UNDER PENALTY OF LAW THAT I UNDERSTAND THE TERMS AND CONDITIONS OF THE GENERAL NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) PERMIT THAT AUTHORIZES THE STORM WATER DISCHARGE ASSOCIATED WITH INDUSTRIAL ACTIVITY FROM THE CONSTRUCTION SITE IDENTIFIED AS PART OF THIS CERTIFICATION.

SIGNATURE/DATE	COMPANY NAME/ADDRESS	RESPONSIBILITY

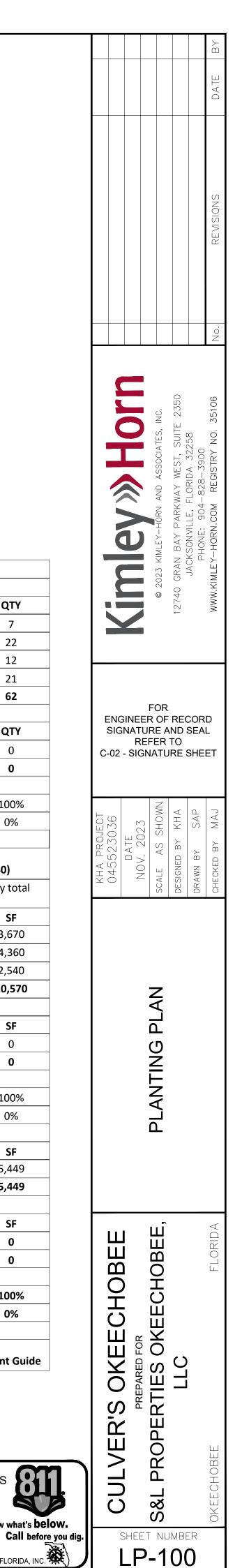
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Kimlev»Horn		© 2023 KIMLEY-HORN AND ASSOCIATES, INC.	IZ/40 GRAN BAT PARKWAT WESI, SULLE 2330 .Iacksonville florida 33358	PHONE: 904-828-3900	WWW.KIMLEY-HORN.COM REGISTRY NO. 35106
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STORM WATER 045523036	NOV.		DESIGNED		CHECKED BY



ME	COMMON NAME	CONT	CAL	HEIGHT	
AVA	SUMMER HAW	CONT	2" MIN	10` HT MIN	
INIANA 'SDLN'	CATHEDRAL™ LIVE OAK	CONT	3" CAL	10`-12` HT.	
то	CABBAGE PALMETTO	B & B		10`, 12`, 14` CT STAGGER HEIGHTS	
ANA FLORIDANA	FLORIDA ELM	CONT	3" CAL MIN	10`-12` HT.	
ME	COMMON NAME	CONT	SIZE	SPACING	
OVATUM	WALTER'S VIBURNUM	CONT.	24" X 24"	4` O.C.	
ME	COMMON NAME	CONT	SIZE		
A CAPILLARIS	PINK MUHLY GRASS	CONT.	24"X24"		48" o.c.
DRIDANUM	FLORIDA GAMAGRASS	CONT.	24"X24"		48" o.c.
ME	COMMON NAME	CONT	<u>SIZE</u>	SPACING	SPACING
DRALIS	GOLDEN CREEPER	CONT.	12"X12"	24" O.C.	24" o.c.

nts	Required	Provided	
	31	62	
	93	173	
386	1,386 SF	5,056 SF	
ees	19	20	
fer			
	8	8	
(8			
	24	24+	
er			
fer	10	12	
	13	13	
(13	39	39+	
	33		
/			
/	2	5	
2 x			
	6	6+	
/	3	15	
3 x			
, ,	9	9+	

NATIVE PLANT CALCULATIONS (PER SEC. 90.54	40)
*Native tree percentages have been calculated by quantity.	
NATIVE TREES	QTY
Florida Elm	7
Summer Haw	22
Cathedral Live Oak	12
Cabbage Palm	21
TOTAL NUMBER OF NATIVE TREES	62
NON-NATIVE TREES	QTY
N/A	0
TOTAL NUMBER OF NON-NATIVE TREES	0
PERCENTAGE OF TOTAL NATIVE TREES	100%
PERCENTAGE OF TOTAL NON-NATIVE TREES	0%
	1
NATIVE SHRUBS & GROUNDCOVER CALCULATIONS (PER SEC	C. 90.540)
*Native shrub and groundcover percentages have been calcu area.	lated by total
NATIVE SHRUBS	SF
Walter's Viburnum	3,670
Pink Muhly Grass	4,360
Florida Gamagrass	2,540
TOTAL SF OF NATIVE SHRUBS	10,570
NON-NATIVE SHRUBS	SF
N/A	0
TOTAL SF OF NON-NATIVE SHRUBS	0
PERCENTAGE OF TOTAL NATIVE SHRUBS	100%
PERCENTAGE OF TOTAL NON-NATIVE SHRUBS	0%
NATIVE GROUNDCOVER	SF
Golden Creeper	5,449
TOTAL SF OF NATIVE GROUNDCOVER	5,449
NON-NATIVE GROUNDCOVER	SF
N/A	0
TOTAL SF OF NON-NATIVE GROUNDCOVER	0
PERCENTAGE OF TOTAL NATIVE GROUNDCOVER	100%
PERCENTAGE OF TOTAL NON-NATIVE GROUNDCOVER	0%
Note: All plants chosen from the South Florida WMD Xerisc	ape Plant Guide



GRAPHIC SCALE IN FEET 0 10 20 40

CALL 2 WORKING DAYS

BEFORE YOU DIG

SUNSHINE STATE ONE CALL OF FLORIDA, INC.

Know what's **below.**

IT'S THE LAW!

DIAL 811

TYPICAL PLANTING NOTES

- A. SCOPE OF WORK
- THE WORK CONSISTS OF: OBTAINING ALL APPLICABLE PERMITS, FURNISHING ALL LABOR, MATERIALS, EQUIPMENT, TOOLS, TRANSPORTATION, AND ANY OTHER APPURTENANCES NECESSARY FOR THE COMPLETION OF THIS PROJECT AS SHOWN ON THE DRAWINGS, AS INCLUDED IN THE PLANT LIST, AND AS SPECIFIED HEREIN.
- 2. WORK SHALL INCLUDE MAINTENANCE AND WATERING OF ALL CONTRACT PLANTING AREAS UNTIL CERTIFICATION OF ACCEPTANCE BY THE OWNER.
- PROTECTION OF EXISTING STRUCTURES
- ALL EXISTING BUILDINGS, WALKS, WALLS, PAVING, PIPING, OTHER SITE CONSTRUCTION ITEMS, AND PLANTING ALREADY COMPLETED OR ESTABLISHED AND DESIGNATED TO REMAIN SHALL BE PROTECTED FROM DAMAGE BY THE CONTRACTOR UNLESS OTHERWISE SPECIFIED. ALL DAMAGE RESULTING FROM NEGLIGENCE SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE OWNER. AT NO COST TO THE OWNER.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL NECESSARY BMP DEVICES ACCORDING TO ALL REGULATORY AGENCY'S STANDARDS THROUGH THE DURATION OF ALL CONSTRUCTION ACTIVITIES.
- THE CONTRACTOR SHALL SUBMIT A DETAILED PROJECT SPECIFIC WORK ZONE TRAFFIC CONTROL PLAN UNLESS THE WORK REQUIRES NOTHING MORE THAN A DIRECT APPLICATION OF THE DEPARTMENT OF TRANSPORTATION (DOT) DESIGN STANDARDS IF A DIRECT APPLICATION OF THE STANDARD IS PROPOSED. THE CONTRACTOR SHALL SUBMIT IN WRITING A STATEMENT INDICATING THE STANDARD INDEX AND PAGE NUMBER NO LESS THAN 10 BUSINESS DAYS PRIOR TO START OF CONSTRUCTION. WHEN A DIRECT APPLICATION OF DOT STANDARD IS NOT ACCEPTABLE A PROJECT SPECIFIC WORK ZONE TRAFFIC CONTROL PLAN SHALL BE PREPARED BY A PROFESSIONAL ENGINEER WHO HAS SUCCESSFULLY COMPLETED ADVANCED TRAINING IN MAINTENANCE OF TRAFFIC, AS DEFINED BY DOT FOR APPROVAL BY THE COUNTY ENGINEER'S REPRESENTATIVE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL UTILITIES, WHETHER PUBLIC OR PRIVATE, PRIOR TO EXCAVATION. THE INFORMATION AND DATA SHOWN WITH RESPECT TO EXISTING UNDERGROUND FACILITIES AT OR CONTIGUOUS TO THE SITE IS APPROXIMATE AND BASED ON INFORMATION FURNISHED BY THE OWNER OF SUCH UNDERGROUND FACILITIES OR ON PHYSICAL APPURTENANCES OBSERVED IN THE FIELD. THE OWNER AND DESIGN PROFESSIONAL SHALL NOT BE RESPONSIBLE FOR THE ACCURACY AND COMPLETENESS OF ANY SUCH INFORMATION OR DATA. THE CONTRACTOR SHALL HAVE FULL RESPONSIBILITY FOR; REVIEWING AND CHECKING ALL SUCH INFORMATION AND DATA; LOCATING ALL UNDERGROUND FACILITIES DURING CONSTRUCTION; THE SAFETY AND PROTECTION THEREOF; REPAIRING ANY DAMAGE THERETO RESULTING FROM THE WORK. THE COST OF ALL WILL BE CONSIDERED AS HAVING BEEN INCLUDED IN THE CONTRACT PRICE. THE CONTRACTOR SHALL NOTIFY ANY AFFECTED UTILITY COMPANIES OR AGENCIES IN WRITING AT LEAST 48 HOURS PRIOR TO BEGINNING CONSTRUCTION.
- NOTIFY THE OWNER IMMEDIATE IF ANY DAMAGES OCCUR TO EXISTING TREES.

ONE (1) CUBIC FOOT

ONE (1) CUBIC FOOT

- 6. CALL ONE CALL (811) TO LOCATE UTILITIES AT LEAST 48 HOURS PRIOR TO CONSTRUCTION.
- C. PROTECTION OF EXISTING PLANT MATERIALS
 - THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL UNAUTHORIZED CUTTING OR DAMAGE TO TREES AND SHRUBS EXISTING OR OTHERWISE, CAUSED BY CARELESS EQUIPMENT OPERATION, MATERIAL STOCKPILING, ETC... THIS SHALL INCLUDE COMPACTION BY DRIVING OR PARKING INSIDE THE DRIP-LINE AND SPILLING OIL, GASOLINE, OR OTHER DELETERIOUS MATERIALS WITHIN THE DRIP-LINE. NO MATERIALS SHALL BE BURNED ON SITE.
- SEE TREE MITIGATION PLAN AND NOTES.
- MATERIALS D.
- GENERAL

MATERIAL SAMPLES LISTED BELOW SHALL BE SUBMITTED FOR APPROVAL, ON SITE OR AS DETERMINED BY THE OWNER. UPON APPROVAL, DELIVERY OF MATERIALS MAY COMMENCE.

*IN SOME INSTANCES AN IMAGE TAKEN OF THE PLANT WITH A MEASURING STICK WILL SUFFICE

ONE (1) OF EACH VARIETY* (OR TAGGED IN NURSERY)

MATERIAL	
MULCH	
TOPSOIL MIX	
PLANTS	

2. PLANT MATERIALS

- a. PLANT SPECIES AND SIZE SHALL CONFORM TO THOSE INDICATED ON THE DRAWINGS. ALL NURSERY STOCK SHALL BE IN ACCORDANCE WITH GRADES AND STANDARDS FOR NURSERY PLANTS, LATEST EDITION, PUBLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES. ALL PLANTS SHALL BE GRADE NO. 1 OR BETTER AS DETERMINED BY THE APPROPRIATE STATE DIVISION OF PLANT INDUSTRY. ALL PLANTS SHALL BE HEALTHY, VIGOROUS SOUND, WELL-BRANCHED, AND FREE OF DISEASE AND INSECTS, INSECT EGGS AND LARVAE AND SHALL HAVE ADEQUATE DOOT SYSTEMS THEES FOR DI ANITING IN DOWS SHALL BE UNIFORM IN SIZE AND SHARE ALL MATERIAL SHALL BE SUBJECT TO APPROVAL BY THE OWNER. WHERE ANY REQUIREMENTS ARE OMITTED FROM THE PLANT LIST, THE PLANTS FURNISHED SHALL BE NORMAL FOR THE VARIETY. PLANTS SHALL BE PRUNED PRIOR TO DELIVERY ONLY WITH APPROVAL FROM OWNER OR OWNER'S REPRESENTATIVE. NO SUBSTITUTIONS SHALL BE MADE WITHOUT WRITTEN PERMISSION FROM THE OWNER'S REPRESENTATIVE
- b. MEASUREMENTS: THE HEIGHT AND/OR WIDTH OF TREES SHALL BE MEASURED FROM THE GROUND OR ACROSS THE NORMAL SPREAD OF BRANCHES WITH THE PLANTS IN THEIR NORMAL POSITION. THIS MEASUREMENT SHALL NOT INCLUDE THE IMMEDIATE TERMINAL GROWTH. PLANTS LARGER IN SIZE THAN THOSE SPECIFIED IN THE PLANT LIST MAY BE USED IF APPROVED BY THE OWNER. IF THE USE OF LARGER PLANTS IS APPROVED, THE BALL OF EARTH OR SPREAD OF ROOTS SHALL BE INCREASED IN PROPORTION TO THE SIZE OF THE PLANT.
- c. INSPECTION: PLANTS SHALL BE SUBJECT TO INSPECTION AND APPROVAL AT THE PLACE OF GROWTH, OR UPON DELIVERY TO THE SITE AS DETERMINED BY THE OWNER FOR QUALITY SIZE AND VARIETY SUCH APPROVAL SHALL NOT IMPAIR THE RIGHT OF INSPECTION AND REJECTION AT THE SITE DURING PROGRESS OF THE WORK OR AFTER COMPLETION FOR SIZE AND CONDITION OF ROOT BALLS OR ROOTS, LATENT DEFECTS OR INJURIES. REJECTED PLANTS SHALL BE REMOVED IMMEDIATELY FROM THE SITE. NOTICE REQUESTING INSPECTION SHALL BE SUBMITTED IN WRITING BY THE CONTRACTOR AT LEAST ONE (1) WEEK PRIOR TO ANTICIPATED DATE.
- SOIL MIXTURE (PLANTING MEDIUM, PLANTING MIX, TOPSOIL MIX) E.
- 1. CONTRACTOR SHALL TEST EXISTING SOIL AND AMEND AS NECESSARY IN ACCORDANCE WITH THE GUIDELINES BELOW: 2. SOIL MIXTURE (PLANTING MEDIUM FOR PLANT PITS) SHALL CONSIST OF TWO PARTS OF TOPSOIL AND ONE PART SAND, AS DESCRIBED BELOW. CONTRACTOR TO SUBMIT SAMPLES AND PH TESTING RESULTS OF SOIL MIXTURE FOR OWNER'S REPRESENTATIVE APPROVAL PRIOR TO PLANT INSTALLATION OPERATIONS COMMENCE.
- a. TOPSOIL FOR USE IN PREPARING SOIL MIXTURE FOR BACKFILLING PLANT PITS SHALL BE FERTILE, FRIABLE, AND OF A LOAMY CHARACTER; REASONABLY FREE OF SUBSOIL, CLAY LUMPS, BRUSH WEEDS AND OTHER LITTER; FREE OF ROOTS, STUMPS, STONES LARGER THAN 2" IN ANY DIRECTION, AND OTHER EXTRANEOUS OR TOXIC MATTER HARMFUL TO PLANT GROWTH. IT SHALL CONTAIN THREE (3) TO FIVE (5) PERCENT DECOMPOSED ORGANIC MATTER AND HAVE A PH BETWEEN 5.5 AND 7.0.
- b. CONTRACTOR SHALL COORDINATE ALL EARTHWORK OPERATIONS WITHIN TREE PROTECTION AREAS WITH THE PROJECT ARBORIST PRIOR TO BEGINNING WORK.
- c. ALL TOPSOIL SHALL BE NATURAL, FRIABLE, FERTILE, FINE LOAMY SOIL POSSESSING CHARACTERISTICS OF REPRESENTATIVE TOPSOIL IN THE VICINITY THAT PRODUCES HEAVY GROWTH.
- d. TOPSOIL, PH RANGE OF 5.5 TO 7.0, 3-5 PERCENT ORGANIC MATERIAL, MINIMUM, FREE FROM SUBSOIL, OBJECTIONABLE WEEDS, LITTER, SODS, STIFF CLAY, STONES LARGER THAN ONE INCH IN DIAMETER, STUMPS, ROOTS, TRASH, TOXIC SUBSTANCES OR ANY OTHER MATERIAL WHICH MAY BE HARMFUL TO PLANT GROWTH.
- e. VERIFY AMOUNT STOCKPILED IF ANY, AND SUPPLY ADDITIONAL AS NEEDED FROM NATURALLY WELL-DRAINED SITES WHERE TOPSOIL OCCURS AT LEAST FOUR INCHES DEEP. DO NOT OBTAIN TOPSOIL FROM BOGS OR MARSHES.
- TREES SHALL BE PLANTED IN THE EXISTING NATIVE SOIL ON SITE, UNLESS DETERMINED TO BE UNSUITABLE AT WHICH POINT THE CONTRACTOR SHALL CONTACT OWNER'S REPRESENTATIVE TO DISCUSS ALTERNATE RECOMMENDATION PRIOR TO PLANTING.
- F. WATER
- WATER NECESSARY FOR PLANTING AND MAINTENANCE SHALL BE OF SATISFACTORY QUALITY TO SUSTAIN ADEQUATE PLANT GROWTH AND SHALL NOT CONTAIN HARMFUL, NATURAL OR MAN-MADE ELEMENTS DETRIMENTAL TO PLANTS. WATER MEETING THE ABOVE STANDARD SHALL BE OBTAINED ON THE SITE FROM THE OWNER, IF AVAILABLE, AND THE CONTRACTOR SHALL BE RESPONSIBLE TO MAKE ARRANGEMENTS FOR ITS USE BY HIS TANKS, HOSES, SPRINKLERS, ETC... IF SUCH WATER IS NOT AVAILABLE AT THE SITE, THE CONTRACTOR SHALL PROVIDE SATISFACTORY WATER FROM SOURCES OFF THE SITE AT NO ADDITIONAL COST TO THE OWNER.
- * WATERING/IRRIGATION RESTRICTIONS MAY APPLY REFER TO PROPERTY'S JURISDICTIONAL AUTHORITY.
- G. FERTILIZER
- CONTRACTOR SHALL PROVIDE FERTILIZER APPLICATION SCHEDULE TO OWNER, AS APPLICABLE TO SOIL TYPE, PLANT INSTALLATION TYPE, AND SITE'S PROPOSED USE. SUGGESTED FERTILIZER TYPES SHALL BE ORGANIC OR OTHERWISE NATURALLY-DERIVED.

* FERTILIZER RESTRICTIONS MAY APPLY - REFER TO PROPERTY'S JURISDICTIONAL AUTHORITY.

H. MULCH

- MULCH MATERIAL SHALL BE MOISTENED AT THE TIME OF APPLICATION TO PREVENT WIND DISPLACEMENT. AND APPLIED AT A MINIMUM DEPTH OF THREE (3) INCHES. CLEAR MULCH FROM EACH PLANT'S CROWN (BASE). SEE PLAN FOR MULCH SPECIFICATIONS
- PROVIDE A THREE (3) INCH MINIMUM LAYER OF SPECIFIED MULCH OVER THE ENTIRE AREA WITH PINNED WEED BARRIER FABRIC IN EACH SHRUB BED, GROUND COVER, VINE BED, AND TREE PIT (6' MINIMUM) PLANTED UNDER THIS CONTRACT. DIGGING AND HANDLING
- PROTECT ROOTS OR ROOT BALLS OF PLANTS AT ALL TIMES FROM SUN, DRYING WINDS, WATER AND FREEZING, AS NECESSARY UNTIL PLANTING. PLANT MATERIALS SHALL BE ADEQUATELY PACKED TO PREVENT DAMAGE DURING TRANSIT. TREES TRANSPORTED MORE THAN TEN (10) MILES OR WHICH ARE NOT PLANTED WITHIN THREE (3) DAYS OF DELIVERY TO THE SITE SHALL BE SPRAYED WITH AN ANTITRANSPIRANT PRODUCT ("WILTPRUF" OR EQUAL) TO MINIMIZE TRANSPIRATIONAL WATER LOSS.
- BALLED AND BURLAPPED (B&B), AND FIELD GROWN (FG) PLANTS SHALL BE DUG WITH FIRM, NATURAL BALLS OF SOIL OF SUFFICIENT SIZE TO ENCOMPASS THE FIBROUS AND FEEDING ROUTS OF THE PLANTS. NO PLANTS MOVED WITH A ROOT BALL SHALL BE PLANTED IF THE BALL IS CRACKED OR BROKEN. PLANTS SHALL NOT BE HANDLED BY STEMS.
- 3. PLANTS MARKED "BR" IN THE PLANT LIST SHALL BE DUG WITH BARE ROOTS. CARE SHALL BE EXERCISED THAT THE ROOTS DO NOT DRY OUT DURING TRANSPORTATION AND PRIOR TO PLANTING.
- PROTECTION OF PALMS: ONLY A MINIMUM OF FRONDS SHALL BE REMOVED FROM THE CROWN OF THE PALM TREES TO FACILITATE MOVING AND HANDLING. CLEAR TRUNK (CT) SHALL BE AS SPECIFIED AFTER THE MINIMUM OF FRONDS HAVE BEEN REMOVED. ALL PALMS SHALL BE BRACED PER PALM PLANTING DETAIL.
- EXCAVATION OF TREE PITS SHALL BE PERFORMED USING EXTREME CARE TO AVOID DAMAGE TO SURFACE AND SUBSURFACE ELEMENTS SUCH AS UTILITIES OR HARDSCAPE ELEMENTS, FOOTERS AND PREPARED SUB-BASES.

CONTAINER GROWN STOCK

- ALL CONTAINER GROWN MATERIAL SHALL BE HEALTHY, VIGOROUS, WELL-ROOTED PLANTS ESTABLISHED IN THE CONTAINER IN WHICH THEY ARE SOLD. THE PLANTS SHALL HAVE TOPS WHICH ARE OF GOOD QUALITY AND ARE IN A HEALTHY GROWING CONDITION.
- AN ESTABLISHED CONTAINER GROWN PLANT SHALL BE TRANSPLANTED INTO A CONTAINER AND GROWN IN THAT CONTAINER SUFFICIENTLY LONG ENOUGH FOR THE NEW FIBROUS ROOTS TO HAVE DEVELOPED SO THAT THE ROOT MASS WILL RETAIN ITS SHAPE AND HOLD TOGETHER WHEN REMOVED FROM THE CONTAINER. CONTAINER GROWN STOCK SHALL NOT BE HANDLED BY THEIR STEMS.
- 3. ROOT BOUND PLANTS ARE NOT ACCEPTABLE AND WILL BE REJECTED.
- 4. RPG="ROOTS PLUS GROWER" CONTAINER PRODUCTS SHALL BE USED WHERE SPECIFIED.

K. MATERIALS LIST

- QUANTITIES NECESSARY TO COMPLETE THE WORK ON THE DRAWINGS SHALL BE FURNISHED BY THE CONTRACTOR. QUANTITY ESTIMATES HAVE BEEN MADE CAREFULLY, BUT THE LANDSCAPE ARCHITECT OR OWNER ASSUMES NO LIABILITY FOR OMISSIONS OR ERRORS. SHOULD A DISCREPANCY OCCUR BETWEEN THE PLANS AND THE PLANT LIST QUANTITY, THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED FOR CLARIFICATION PRIOR TO BIDDING OR INSTALLATION.
- 2. ALL DIMENSIONS AND/OR SIZES SPECIFIED SHALL BE THE MINIMUM ACCEPTABLE SIZE.

FINE GRADING

- 1. FINE GRADING UNDER THIS CONTRACT SHALL CONSIST OF FINAL FINISHED GRADING OF LAWN AND PLANTING AREAS THAT HAVE BEEN ROUGH GRADED BY OTHERS. BERMING AS SHOWN ON THE DRAWINGS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS OTHERWISE NOTED.
- THE CONTRACTOR SHALL FINE GRADE THE LAWN AND PLANTING AREAS TO BRING THE ROUGH GRADE UP TO FINAL FINISHED GRADE ALLOWING FOR THICKNESS OF SOD AND/OR MULCH DEPTH. CONTRACTOR SHALL FINE GRADE BY HAND AND/OR WITH ALL EQUIPMENT NECESSARY INCLUDING A GRADING TRACTOR WITH FRONT-END LOADER FOR TRANSPORTING SOIL WITHIN THE SITE.
- ALL PLANTING AREAS SHALL BE GRADED AND MAINTAINED FOR POSITIVE DRAINAGE TO SURFACE/SUBSURFACE STORM DRAIN SYSTEMS. AREAS ADJACENT TO BUILDINGS SHALL SLOPE AWAY FROM THE BUILDINGS. REFER TO CIVIL ENGINEER'S PLANS FOR FINAL GRADES, IF APPLICABLE.

0 PLANTING PROCEDURES

- CLEANING UP BEFORE COMMENCING WORK: THE CONTRACTOR SHALL CLEAN WORK AND SURROUNDING AREAS OF ALL RUBBISH OR OBJECTIONABLE MATTER DAILY. ALL MORTAR, CEMENT, AND TOXIC MATERIAL SHALL BE REMOVED FROM THE SURFACE OF ALL PLANT BEDS. THESE MATERIALS SHALL NOT BE MIXED WITH THE SOIL, SHOULD THE CONTRACTOR FIND SUCH SOIL CONDITIONS BENEATH THE SOIL WHICH WILL IN ANY WAY ADVERSELY AFFECT THE PLANT GROWTH HE SHALL IMMEDIATELY CALL IT TO THE ATTENTION OF THE OWNER'S REPRESENTATIVE. FAILURE TO DO SO BEFORE PLANTING SHALL MAKE THE CORRECTIVE MEASURES THE RESPONSIBILITY OF THE CONTRACTOR.
- SUBGRADE EXCAVATION: CONTRACTOR IS RESPONSIBLE TO REMOVE ALL EXISTING AND IMPORTED LIMEROCK AND LIMEROCK SUB-BASE FROM ALL LANDSCAPE PLANTING AREAS TO A MINIMUM DEPTH OF 36" OR TO NATIVE SOIL. CONTRACTOR IS RESPONSIBLE TO BACKFILL THESE PLANTING AREAS TO ROUGH FINISHED GRADE WITH CLEAN TOPSOIL FROM AN ON-SITE SOURCE OR AN IMPORTED SOURCE. IF LIMEROCK OR OTHER ADVERSE CONDITIONS OCCUR IN PLANTED AREAS AFTER 36" DEEP EXCAVATION BY THE CONTRACTOR, AND POSITIVE DRAINAGE CAN NOT BE ACHIEVED, CONTRACTOR SHALL UTILIZE POOR DRAINAGE CONDITION PLANTING DETAIL.
- FURNISH NURSERY'S CERTIFICATE OF COMPLIANCE WITH ALL REQUIREMENTS AS SPECIFIED HEREIN. INSPECT AND SELECT PLANT MATERIALS BEFORE PLANTS ARE DUG AT NURSERY OR GROWING SITE.
- COMPLY WITH APPLICABLE FEDERAL, STATE, COUNTY, AND LOCAL REGULATIONS GOVERNING LANDSCAPE MATERIALS AND WORK. CONFORM TO ACCEPTED HORTICULTURAL PRACTICES AS USED IN THE TRADE. UPON ARRIVAL AT THE SITE, PLANTS SHALL BE THOROUGHLY WATERED AND PROPERLY MAINTAINED UNTIL PLANTED. PLANTS STORED ONSITE SHALL NOT REMAIN UNPLANTED OR APPROPRIATELY HEALED IN FOR A PERIOD EXCEEDING TWENTY-FOUR (24) HOURS. AT ALL TIMES WORKMANLIKE METHODS CUSTOMARY IN GOOD HORTICULTURAL PRACTICES SHALL BE EXERCISED.
- THE WORK SHALL BE COORDINATED WITH OTHER TRADES TO PREVENT CONFLICTS. COORDINATE PLANTING WITH IRRIGATION WORK TO ASSURE AVAILABILITY OF WATER AND PROPER LOCATION OF IRRIGATION APPURTENANCES AND PLANTS
- 6. ALL PLANTING PITS SHALL BE EXCAVATED TO SIZE AND DEPTH IN ACCORDANCE WITH THE USA STANDARD FOR NURSERY STOCK, UNLESS SHOWN OTHERWISE ON THE DRAWINGS, AND BACK FILLED WITH THE PREPARED PLANTING SOIL MIXTURE AS SPECIFIED IN SOIL MIXTURE SECTION OF THESE SPECIFICATIONS. TEST ALL TREE PITS WITH WATER BEFORE PLANTING TO ASSURE PROPER DRAINAGE PERCOLATION IS AVAILABLE. NO ALLOWANCE WILL BE MADE FOR LOST PLANTS DUE TO IMPROPER DRAINAGE. IF POOR DRAINAGE EXISTS, UTILIZE "POOR DRAINAGE CONDITION" PLANTING DETAIL. TREES SHALL BE SET PLUMB AND HELD IN POSITION UNTIL THE PLANTING MIXTURE HAS BEEN FLUSHED INTO PLACE WITH A SLOW, FULL HOSE STREAM. ALL PLANTING SHALL BE PERFORMED BY PERSONNEL FAMILIAR WITH PLANTING PROCEDURES AND UNDER THE SUPERVISION OF A QUALIFIED LANDSCAPE FOREMAN. PROPER "JETTING IN" SHALL BE ASSURED TO ELIMINATE AIR POCKETS AROUND THE ROOTS. "JET STICK" OR EQUAL IS RECOMMENDED.
- TAKE ALL NECESSARY PRECAUTIONS TO AVOID DAMAGE TO BUILDINGS AND BUILDING STRUCTURES WHILE INSTALLING
- 8. FOLLOW PLANTING SPECIFICATIONS IN PLANTING DETAILS
- 9. FILL HOLE WITH SOIL MIXTURE, MAKING CERTAIN ALL SOIL IS SATURATED. TO DO THIS, FILL HOLE WITH WATER AND ALLOW TO SOAK MINIMUM TWENTY (20) MINUTES, STIRRING IF NECESSARY TO GET SOIL THOROUGHLY WET. PACK LIGHTLY WITH FEET, ADD MORE WET SOIL MIXTURE. DO NOT COVER TOP OF BALL WITH SOIL MIXTURE. ALL BURLAP, ROPE, WIRES, BASKETS, ETC.., SHALL BE REMOVED FROM THE SIDES AND TOPS OF BALLS, BUT NO BURLAP SHALL BE PULLED FROM UNDERNEATH.
- 10. TREES SHALL BE PRUNED, AT THE DIRECTION OF THE OWNER OR OWNER'S REPRESENTATIVE, TO PRESERVE THE NATURAL CHARACTER OF THE PLANT. ALL SOFT WOOD OR SUCKER GROWTH AND ALL BROKEN OR BADLY DAMAGED BRANCHES SHALL BE REMOVED WITH A CLEAN CUT. ALL PRUNING TO BE PERFORMED BY CERTIFIED ARBORIST, IN ACCORDANCE WITH ANSI A-300.
- 11. SHRUBS AND GROUND COVER PLANTS SHALL BE EVENLY SPACED IN ACCORDANCE WITH THE DRAWINGS AND AS INDICATED ON THE PLANT LIST. MATERIALS INSTALLED SHALL MEET MINIMUM SPECIMEN REQUIREMENTS OR QUANTITIES SHOW ON PLANS. WHICHEVER IS GREATER. CULTIVATE ALL PLANTING AREAS TO A MINIMUM DEPTH OF 6", REMOVE AND DISPOSE ALL DEBRIS. MIX TOP 4" THE PLANTING SOIL MIXTURE AS SPECIFIED IN SOIL MIXTURE SECTION OF THESE SPECIFICATIONS. THOROUGHLY WATER ALL PLANTS AFTER INSTALLATION.
- 12. TREE GUYING AND BRACING SHALL BE INSTALLED BY THE CONTRACTOR IN ACCORDANCE WITH THE PLANS TO ENSURE STABILITY AND MAINTAIN TREES IN AN UPRIGHT POSITION. IF THE CONTRACTOR AND OWNER DECIDE TO WAIVE THE TREE GUYING AND BRACING. THE OWNER SHALL NOTIFY THE PROJECT LANDSCAPE ARCHITECT IN WRITING AND AGREE TO INDEMNIFY AND HOLD HARMLESS THE PROJECT LANDSCAPE ARCHITECT IN THE EVENT UNSUPPORTED TREES PLANTED UNDER THIS CONTRACT FALL AND DAMAGE PERSON OR PROPERTY

ALL PLANT BEDS SHALL BE KEPT FREE OF NOXIOUS WEEDS UNTIL FINAL ACCEPTANCE OF WORK.

P. LAWN SODDING

- AREA THOROUGHLY
- 4. SODDING

- 6. LAWN MAINTENANCE
- AUTHORITY.

Q. CLEANUP

- R. PLANT MATERIAL MAINTENANCE
- S. FINAL INSPECTION AND ACCEPTANCE OF WORK

T. WARRANTY

- AT NO ADDITIONAL COST TO THE OWNER.
- DATE OF ACCEPTANCE.

THE WORK CONSISTS OF LAWN BED PREPARATION, SOIL PREPARATION, AND SODDING COMPLETE, IN STRICT ACCORDANCE WITH THE SPECIFICATIONS AND THE APPLICABLE DRAWINGS TO PRODUCE A TURF GRASS LAWN ACCEPTABLE TO THE

2. ALL AREAS THAT ARE TO BE SODDED SHALL BE CLEARED OF ANY ROUGH GRASS, WEEDS, AND DEBRIS BY MEANS OF A SOD CUTTER TO A DEPTH OF THREE (3) INCHES, AND THE GROUND BROUGHT TO AN EVEN GRADE. THE ENTIRE SURFACE SHALL BE ROLLED WITH A ROLLER WEIGHING NOT MORE THAN ONE-HUNDRED (100) POUNDS PER FOOT OF WIDTH. DURING THE ROLLING. ALL DEPRESSIONS CAUSED BY SETTLEMENT SHALL BE FILLED WITH ADDITIONAL SOIL, AND THE SURFACE SHALL BE REGRADED AND ROLLED UNTIL PRESENTING A SMOOTH AND EVEN FINISH TO THE REQUIRED GRADE.

PREPARE LOOSE BED FOUR (4) INCHES DEEP. RAKE UNTIL ALL BUMPS AND DEPRESSIONS ARE REMOVED. WET PREPARED

a. THE CONTRACTOR SHALL SOD ALL AREAS THAT ARE NOT PAVED OR PLANTED AS DESIGNATED ON THE DRAWINGS WITHIN THE CONTRACT LIMITS, UNLESS SPECIFICALLY NOTED OTHERWISE.

b. THE SOD SHALL BE CERTIFIED TO MEET APPROPRIATE STATE PLANT BOARD SPECIFICATIONS, ABSOLUTELY TRUE TO VARIETAL TYPE, AND FREE FROM WEEDS, FUNGUS, INSECTS AND DISEASE OF ANY KIND.

c. SOD PANELS SHALL BE LAID TIGHTLY TOGETHER SO AS TO MAKE A SOLID SODDED LAWN AREA. SOD SHALL BE LAID UNIFORMLY AGAINST THE EDGES OF ALL CURBS AND OTHER HARDSCAPE ELEMENTS, PAVED AND PLANTED AREAS. IMMEDIATELY FOLLOWING SOD LAYING, THE LAWN AREAS SHALL BE ROLLED WITH A LAWN ROLLER CUSTOMARILY USED FOR SUCH PURPOSES, AND THEN THOROUGHLY IRRIGATED. IF, IN THE OPINION OF THE OWNER, TOP-DRESSING IS NECESSARY AFTER ROLLING TO FILL THE VOIDS BETWEEN THE SOD PANELS AND TO EVEN OUT INCONSISTENCIES IN THE SOD, CLEAN SAND, AS APPROVED BY THE OWNER'S REPRESENTATIVE, SHALL BE UNIFORMLY SPREAD OVER THE ENTIRE SURFACE OF THE SOD AND THOROUGHLY WATERED IN. FERTILIZE INSTALLED SOD AS ALLOWED BY PROPERTY'S JURISDICTIONAL AUTHORITY.

DURING DELIVERY, PRIOR TO, AND DURING THE PLANTING OF THE LAWN AREAS, THE SOD PANELS SHALL AT ALL TIMES BE PROTECTED FROM EXCESSIVE DRYING AND UNNECESSARY EXPOSURE OF THE ROOTS TO THE SUN. ALL SOD SHALL BE STACKED SO AS NOT TO BE DAMAGED BY SWEATING OR EXCESSIVE HEAT AND MOISTURE.

a. WITHIN THE CONTRACT LIMITS, THE CONTRACTOR SHALL PRODUCE A DENSE, WELL ESTABLISHED LAWN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REPAIR AND RE-SODDING OF ALL ERODED, SUNKEN OR BARE SPOTS (LARGER THAN 12"X12") UNTIL CERTIFICATION OF ACCEPTANCE BY THE OWNER'S REPRESENTATIVE. REPAIRED SODDING SHALL BE ACCOMPLISHED AS IN THE ORIGINAL WORK (INCLUDING REGRADING IF NECESSARY).

b. CONTRACTOR RESPONSIBLE FOR ESTABLISHING AND MAINTAINING SOD/LAWN UNTIL ACCEPTANCE BY THE OWNER'S REPRESENTATIVE. PRIOR TO AND UPON ACCEPTANCE, CONTRACTOR TO PROVIDE WATERING/IRRIGATION SCHEDULE TO OWNER. OBSERVE ALL APPLICABLE WATERING RESTRICTIONS AS SET FORTH BY THE PROPERTY'S JURISDICTIONAL

UPON COMPLETION OF ALL PLANTING WORK AND BEFORE FINAL ACCEPTANCE, THE CONTRACTOR SHALL REMOVE ALL MATERIAL, EQUIPMENT, AND DEBRIS RESULTING FROM HIS WORK. ALL PAVED AREAS SHALL BE CLEANED AND THE SITE LEFT IN A NEAT AND ACCEPTABLE CONDITION AS APPROVED BY THE OWNER'S REPRESENTATIVE.

ALL PLANTS AND PLANTING INCLUDED UNDER THIS CONTRACT SHALL BE MAINTAINED BY WATERING, CULTIVATING, SPRAYING, AND ALL OTHER OPERATIONS (SUCH AS RE-STAKING OR REPAIRING TREE GUY SUPPORTS NECESSARY) TO ENSURE A HEALTHY PLANT CONDITION BY THE CONTRACTOR OR UNTIL CERTIFICATION OF ACCEPTANCE BY THE OWNER'S REPRESENTATIVE.

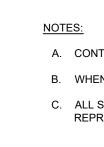
FINAL INSPECTION AT THE END OF THE WARRANTY PERIOD SHALL BE ON PLANTING, CONSTRUCTION AND ALL OTHER INCIDENTAL WORK PERTAINING TO THIS CONTRACT. ANY REPLACEMENT AT THIS TIME SHALL BE SUBJECT TO THE SAME ONE (1) YEAR WARRANTY (OR AS SPECIFIED BY THE LANDSCAPE ARCHITECT OR OWNER IN WRITING) BEGINNING WITH THE TIME OF REPLACEMENT AND ENDING WITH THE SAME INSPECTION AND ACCEPTANCE HEREIN DESCRIBED.

THE LIFE AND SATISFACTORY CONDITION OF ALL PLANT MATERIAL INSTALLED (INCLUDING SOD) BY THE LANDSCAPE CONTRACTOR SHALL BE WARRANTED BY THE CONTRACTOR FOR A MINIMUM OF ONE (1) CALENDAR YEAR COMMENCING AT THE TIME OF CERTIFICATION OF ACCEPTANCE BY THE OWNER'S REPRESENTATIVE.

ANY PLANT NOT FOUND IN A HEALTHY GROWING CONDITION AT THE END OF THE WARRANTY PERIOD SHALL BE REMOVED FROM THE SITE AND REPLACED AS SOON AS WEATHER CONDITIONS PERMIT. ALL REPLACEMENTS SHALL BE PLANTS OF THE SAME KIND AND SIZE AS SPECIFIED IN THE PLANT LIST. THEY SHALL BE FURNISHED PLANTED AND MULCHED AS SPECIFIED

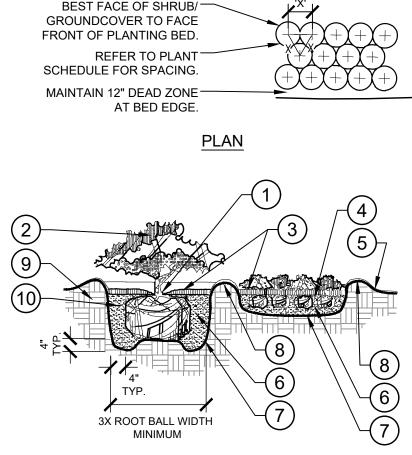
3. IN THE EVENT THE OWNER DOES NOT CONTRACT WITH THE CONTRACTOR FOR LANDSCAPE AND IRRIGATION MAINTENANCE, THE CONTRACTOR SHOULD VISIT THE PROJECT SITE PERIODICALLY DURING THE ONE (1) YEAR WARRANTY PERIOD TO EVALUATE MAINTENANCE PROCEDURES BEING PERFORMED BY THE OWNER. CONTRACTOR SHALL NOTIFY THE OWNER IN WRITING OF MAINTENANCE PROCEDURES OR CONDITIONS WHICH THREATEN VIGOROUS AND HEALTHY PLANT GROWTH. SITE VISITS SHALL BE CONDUCTED A MINIMUM OF ONCE PER MONTH FOR A PERIOD OF TWELVE (12) MONTHS FROM THE

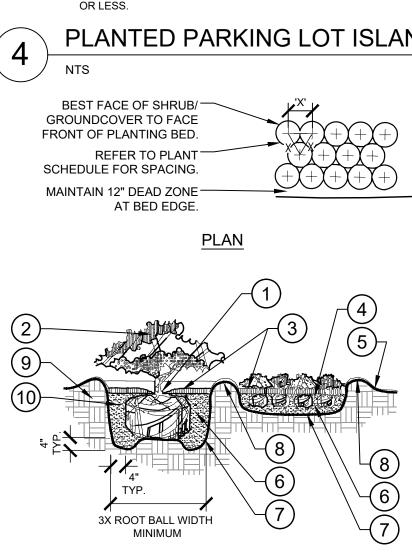
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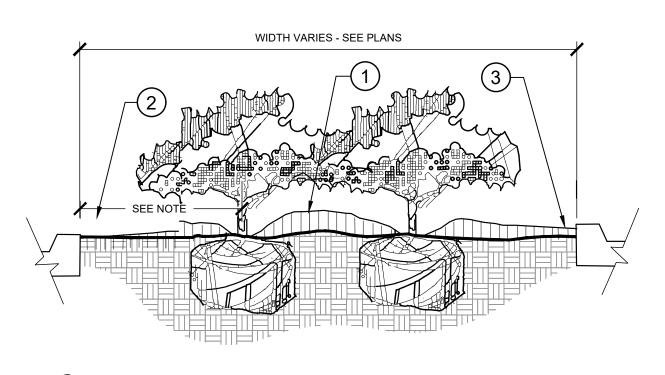
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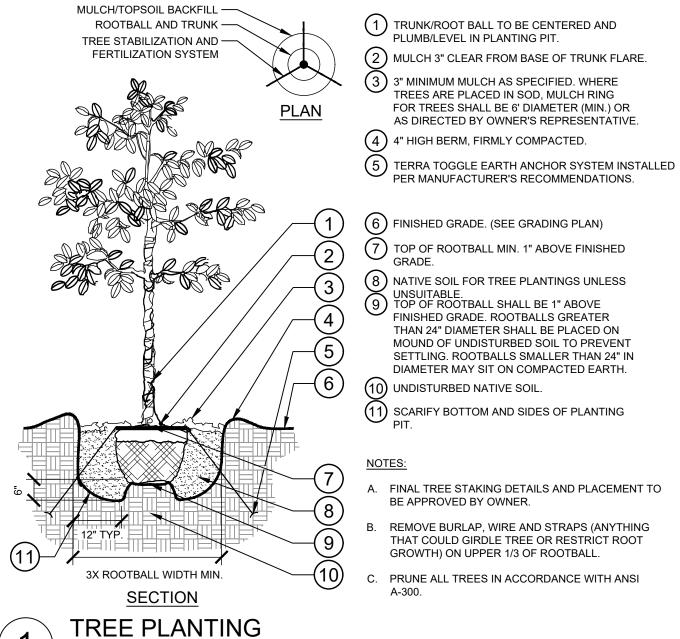


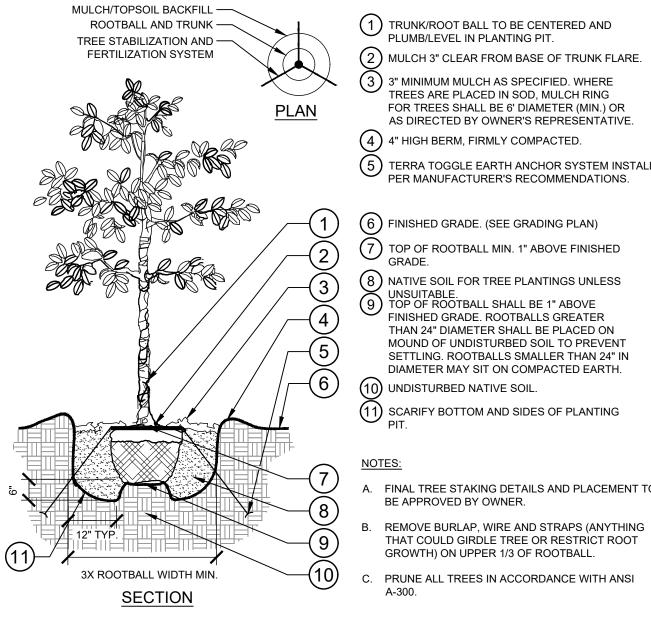




SECTION











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(1) CROWN ISLANDS @ 5:1 SLOPES (OR AS SPECIFIED ON THE LANDSCAPE PLANS). CLEAR ZONE: 36" MIN. FROM BACK OF CURB TO CENTER OF NEAREST SHRUB. CLEAR ZONE SHALL CONTAIN 3" CONTINUOUS MULCH OR TURF, SEE PLANS.

(3) 2" MIN VERTICAL CLEARANCE, TOP OF CURB TO TOP OF MULCH.

A. EXCAVATE SOIL FOLLOWING THE SHRUB/GROUNDCOVER PLANTING DETAIL AND BACKFILL WITH APPROVED PLANTING MIX.

B. PROTECT AND RETAIN ALL CURBS AND BASE. COMPACTED SUBGRADE TO REMAIN FOR STRUCTURAL SUPPORT OF CURB SYSTEM (TYP).

C. ALL ISLANDS SHALL UTILIZE POOR DRAINAGE DETAIL WHEN PERCOLATION RATES ARE 2" PER HOUR

PLANTED PARKING LOT ISLANDS/MEDIANS

(1) TOP OF SHRUB ROOTBALLS TO BE PLANTED 1" - 2" HIGH WITH SOIL MOUNDING UP TO THE TOP OF ROOTBALL.

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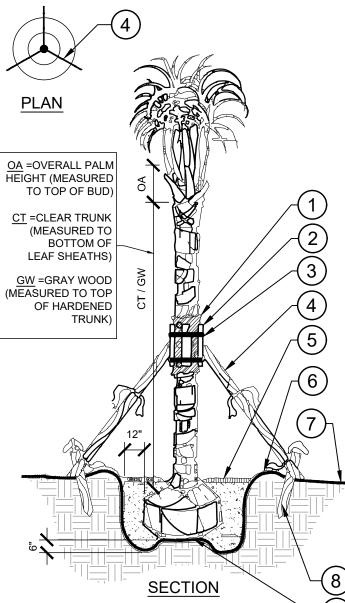
- 2) PRUNE ALL SHRUBS TO ACHIEVE A UNIFORM MASS/HEIGHT.
- (3) 3" MULCH LAYER AS SPECIFIED.
- 4 EXCAVATE ENTIRE BED SPECIFIED FOR GROUNDCOVER BED.
- 5 FINISHED GRADE (SEE GRADING PLAN).
- (6) PREPARED PLANTING SOIL AS SPECIFIED. (SEE LANDSCAPE NOTES) NOTE: WHEN GROUND-COVER'S AND SHRUBS USED IN MASSES, ENTIRE BED TO BE AMENDED WITH PLANTING SOIL MIX AS SPECIFIED.
- 7) SCARIFY OF PLANTING PIT
- SIDES AND BOTTOM. 8 4" HIGH BERM FIRMLY
- COMPACTED. (9) UNDISTURBED NATIVE SOIL.
- (10) FERTILIZER TABLETS (MAX 3"
- DEEP)

A. CONTRACTOR SHALL ASSURE PERCOLATION OF ALL PLANTING PITS PRIOR TO INSTALLATION.

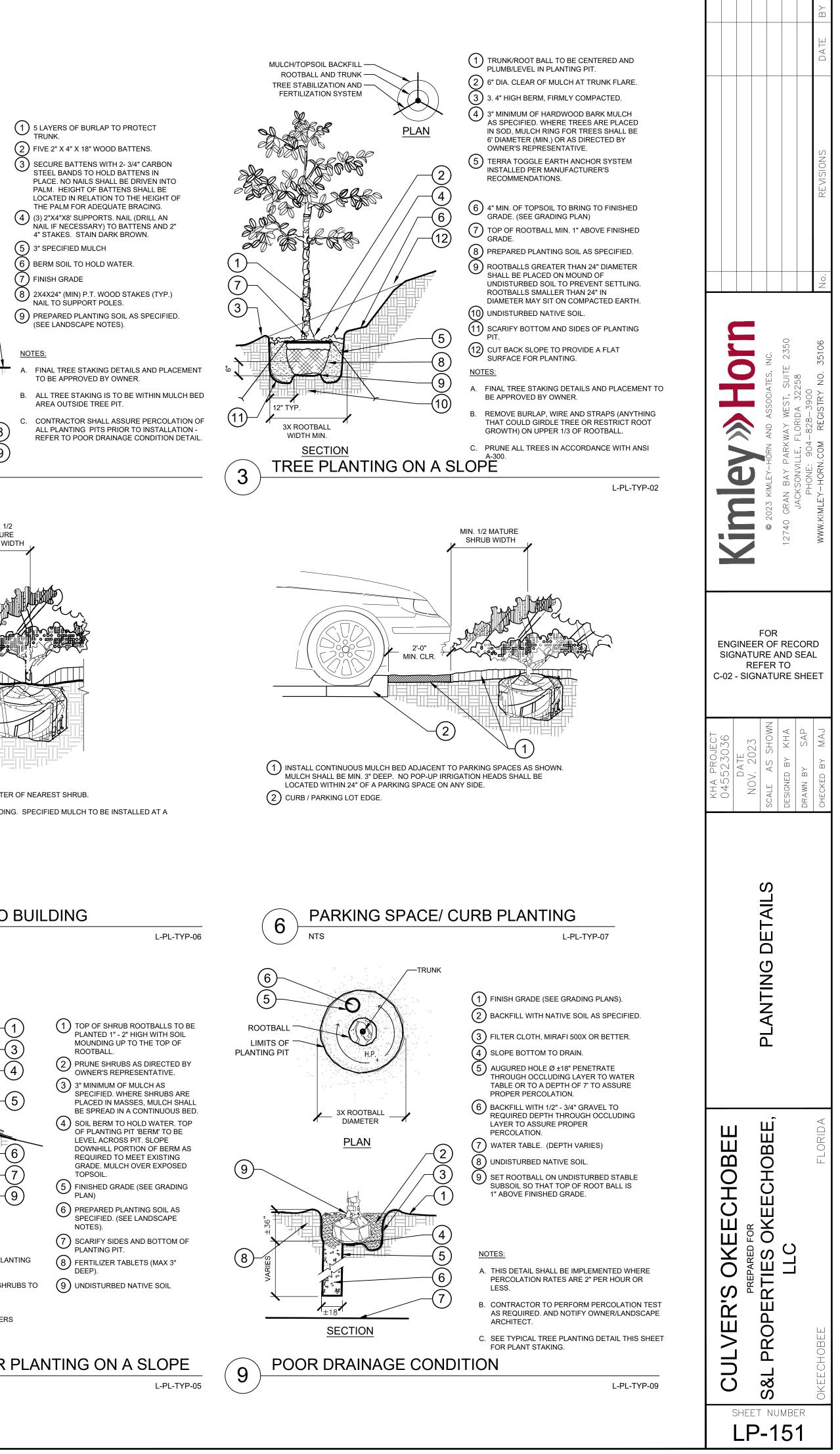
B. WHEN SHRUBS ARE PRUNED IN MASSES, PRUNE ALL SHRUBS TO ACHIEVE UNIFORM MASS / HEIGHT. C. ALL SHRUBS AND GROUNDCOVERS SHALL BE PLUMB VERTICALLY, UNLESS OTHERWISE DIRECTED BY OWNERS REPRESENTATIVE.

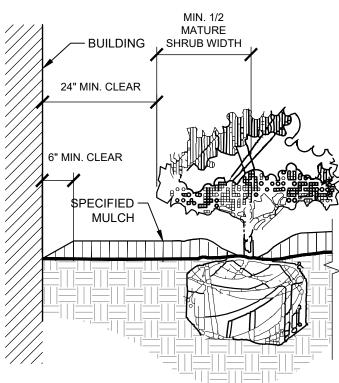
SHRUB/ GROUNDCOVER PLANTING

L-PL-TYP-03



PALM PLANTING SECTION/ PLAN





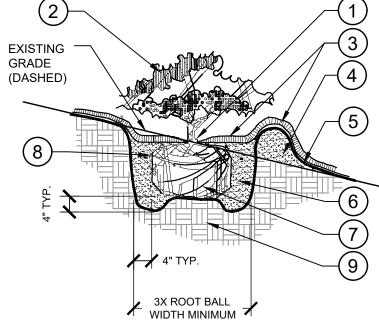
NOTES:

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NTS

- 1. CLEAR ZONE: 36" MIN. FROM BUILDING TO CENTER OF NEAREST SHRUB.
- 2. INSTALL SPECIFIED MULCH: 6" MIN. FROM BUILDING. SPECIFIED MULCH TO BE INSTALLED AT A DEPTH OF 3" (MIN.)

PLANTING ADJACENT TO BUILDING

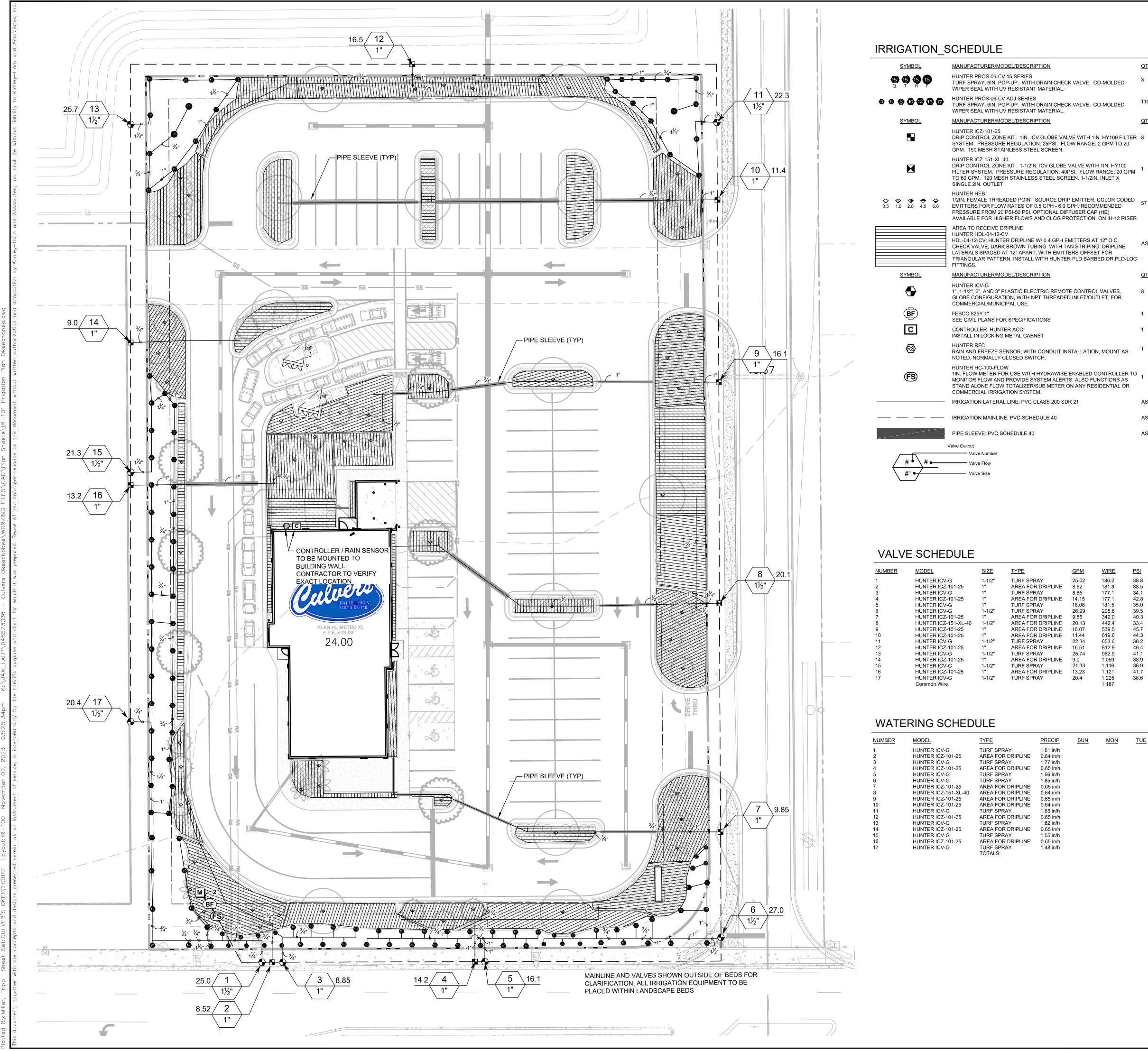


NOTES:

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- A. CONTRACTOR SHALL ASSURE PERCOLATION OF ALL PLANTING PITS PRIOR TO INSTALLATION.
- B. WHEN SHRUBS ARE PRUNED IN MASSES, PRUNE ALL SHRUBS TO ACHIEVE UNIFORM MASS / HEIGHT.
- C. ALL SHRUBS AND GROUNDCOVERS SHALL BE PLUMB VERTICALLY, UNLESS OTHERWISE DIRECTED BY OWNERS REPRESENTATIVE.

SHRUB/ GROUNDCOVER PLANTING ON A SLOPE



	<u>QTY</u>	<u>PSI</u>
/E. CO-MOLDED	3	30
/E. CO-MOLDED	119	30
	<u>QTY</u>	
TH 1IN. HY100 FILTER NGE: 2 GPM TO 20	8	
WITH 1IN. HY100 OW RANGE: 20 GPM -1/2IN. INLET X	1	
TER. COLOR CODED ECOMMENDED R CAP (HE) TION. ON IH-12 RISER	57	
RS AT 12" O.C. RIPING. DRIPLINE FFSET FOR BARBED OR PLD-LOC	AS SHOWN	
	<u>QTY</u>	

8

AS SHOWN
AS SHOWN
AS SHOWN

WIRE	<u>PSI</u>	<u>PSI @ POC</u>	PRECIP
186.2 181.8	38.8 38.5	53.3 51.3	1.61 in/h 0.64 in/h
177.1 177.1	34.1 42.8	47.0 55.8	1.77 in/h 0.65 in/h
181.5	42.8 35.0	48.1	1.56 in/h
295.6	39.5	56.0	1.85 in/h
342.0 442.4	40.3 33.4	53.7 48.4	0.65 in/h 0.64 in/h
442.4 539.5	33.4 45.7	40.4 59.7	0.65 in/h
619.6	44.3	58.0	0.64 in/h
653.6	38.2	54.8	1.65 in/h
812.9	46.4	61.3	0.65 in/h
962.9	41.1	61.0	1.62 in/h
1,059	38.9	52.7	0.65 in/h
1,116	36.9	55.0	1.55 in/h
1,121	41.7	56.3	0.65 in/h
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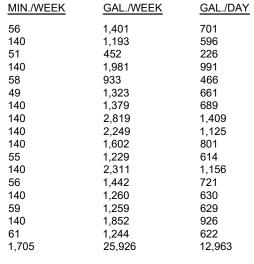
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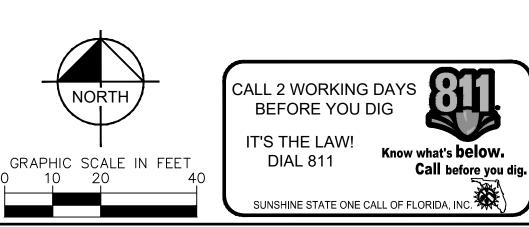
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619.6	6 44.3	58.0)
653.6	38.2	2 54.8	}
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1,116	36.9	9 55.0)
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1,225	5 38.6	56.5	5
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MON	TUE	WED	THU	<u>FRI</u>	SAT	IN./WEEK	MIN./WEEK
		28 min			28 min	1.5	56
		70 min			70 min	1.5	140
		26 min			26 min	1.5	51
		70 min			70 min	1.5	140
		29 min			29 min	1.5	58
		25 min			25 min	1.5	49
		70 min			70 min	1.5	140
		70 min			70 min	1.5	140
		70 min			70 min	1.5	140
		70 min			70 min	1.5	140
		28 min			28 min	1.5	55
		70 min			70 min	1.5	140
		28 min			28 min	1.5	56
		70 min			70 min	1.5	140
		30 min			30 min	1.5	59
		70 min			70 min	1.5	140
		31 min			31 min	1.5	61

855

1.48 in/h





					DATE BY
					REVISIONS
			220		06 No.
	NIMIEV »> TOI	© 2023 KIMLEY-HORN AND ASSOCIATES, INC.	12740 GRAN BAY PARKWAY WEST, SUITE 2350	JACKSONVILLE, FLURIDA 32258 PHONE: 904-828-3900	WWW.KIMLEY-HORN.COM REGISTRY NO. 35106
SIG	GINEE GNATU RE - SIGN	RE A FER NATU	ND TO	SEA	L
KHA PROJECT 045523036	DATE NOV. 2023	SCALE AS SHOWN	DESIGNED BY KHA	DRAWN BY SAP	снескер ву МАЈ
		RRIGATION PLAN			
		IRRIGATI			

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S&L

SHEET NUMBER

IR-100

CRITICAL ANALYSIS

Generated:	2023-10-23 15:07
P.O.C. NUMBER: 01 Water Source Information:	
FLOW AVAILABLE Water Meter Size: Flow Available	1" 37.5 GPM
PRESSURE AVAILABLE Static Pressure at POC: Elevation Change: Service Line Size: Length of Service Line: Pressure Available:	65 PSI 0.00 ft 2" <u>10 ft</u> 65 PSI
DESIGN ANALYSIS Maximum Station Flow: Flow Available at POC: Residual Flow Available:	26.99 GPM <u>37.5 GPM</u> 10.51 GPM
Critical Station: Design Pressure: Friction Loss: Fittings Loss: Elevation Loss: Loss through Valve: Pressure Req. at Critical Station: Loss for Fittings: Loss for Main Line: Loss for POC to Valve Elevation: Loss for POC to Valve Elevation: Loss for Backflow: Loss for Water Meter: Critical Station Pressure at POC: Pressure Available: Residual Pressure Available:	12 30 PSI 4.65 PSI 0.48 PSI 0 PSI 11.3 PSI 46.4 PSI 0.18 PSI 1.81 PSI 0 PSI 11.3 PSI 1.5 PSI 61.3 PSI 65 PSI 3.73 PSI

IRRIGATION SPECIFICATION AND NOTES

- 1. THE SYSTEM SHALL BE DESIGNED TO PROVIDE 100% COVERAGE. ANY CHANGES MADE IN THE LAYOUT DUE TO FIELD CONDITIONS SHALL BE IN ACCORDANCE WITH THESE STANDARDS.
- 2. VERIFY LOCATIONS OF ALL UNDERGROUND UTILITIES PRIOR TO INSTALLATION OF IRRIGATION SYSTEM. ALL UTILITIES AND STRUCTURES MAY NOT BE SHOWN ON THESE PLANS-CONTRACTOR SHALL FIELD VERIFY.
- 3. CONTRACTOR TO FIELD VERIFY ALL POINT OF CONNECTION SOURCE INFORMATION INCLUDING PSI AND GPM PRIOR TO CONSTRUCTION.
- 4. INSTALLATION OF WORK SHALL BE COORDINATED WITH OTHER CONTRACTORS IN SUCH A MANNER AS TO ALLOW FOR A SPEEDY AND ORDERLY COMPLETION OF ALL WORK ON-SITE.
- 5. IRRIGATION DESIGN IS SCHEMATIC ONLY. FULL AND COMPLETE SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW BY THE OWNER'S REPRESENTATIVE.
- 6. CONTRACTOR SHALL COORDINATE WITH THE PLANTING PLAN FOR PLANTER BED AND TREE LOCATIONS TO ENSURE ALL PLANT MATERIAL IS COVERED BY 100% HEAD-TO-HEAD IRRIGATION.
- 7. CONTRACTOR SHALL PROVIDE "AS-BUILT" DRAWINGS OF THE FINAL INSTALLATION TO OWNER AT SUBSTANTIAL COMPLETION BEFORE RECEIVING FINAL PAYMENT.
- 8. IRRIGATION CONTRACTOR TO PROVIDE POWER SUPPLY TO ELECTRIC CONTROLLERS.
- 9. IRRIGATION CONTRACTOR SHALL SECURE ANY AND ALL NECESSARY PERMITS FOR THE WORK PRIOR TO COMMENCEMENT OF HIS OPERATIONS ON-SITE. COPIES OF THE PERMITS SHALL BE SENT TO THE OWNER/GENERAL CONTRACTOR. WORK IN THE R.O.W. SHALL CONFORM TO THE STANDARDS AND SPECIFICATIONS OF LOCAL AND/OR STATE JURISDICTION.
- 10. IRRIGATION SYSTEM SHALL NOT BE INSTALLED THROUGH PRESERVED PLANT COMMUNITIES OR WITHIN WETLANDS OR THEIR ASSOCIATED BUFFERS.
- 11. LOCATE ALL IRRIGATION LINES WITHIN LANDSCAPED AREAS WHENEVER POSSIBLE. ALL LINES UNDER PAVEMENT MUST BE SLEEVED. ALL VALVES SHALL BE LOCATED WITHIN LANDSCAPED AREAS, AS SPECIFIED.
- 12. MAINLINE SHALL NOT BE LOCATED WITHOUT PRIOR APPROVAL OF THE OWNER'S REPRESENTATIVE.
- 13. CONTRACTOR TO COORDINATE LOCATION OF ALL METERS AND BACKFLOW ASSEMBLIES WITH PROJECT OWNER.
- 14. THE IRRIGATION CONTRACTOR SHALL BE DIRECTLY RESPONSIBLE FOR SLEEVING AND DIRECTIONAL BORES.
- 15. ALL SLEEVES UTILIZED BY THE IRRIGATION CONTRACTOR WHETHER INSTALLED BY HIM OR NOT, SHALL BE LOCATED ON THE "AS-BUILT" DRAWINGS. THE DEPTH BELOW FINISH GRADE, TO THE NEAREST FOOT OF EACH END OF THE SLEEVE SHALL BE NOTED AT EACH SLEEVE LOCATION ON THE "AS-BUILT" DRAWINGS. ALL SLEEVES ON PLAN FOR WALL PENETRATIONS AND UNDER SIDEWALKS SHALL BE SIZED TWO PIPE SIZES GREATER THAN THE PIPE IT CARRIES.
- 16. ALL PRESSURIZED MAINLINES AND LATERALS UNDER PAVEMENT SHALL BE WITHIN SLEEVES AS NOTED. WHERE ELECTRIC OR HYDRAULIC VALVE CONTROL LINES PASS THROUGH A SLEEVE WITH OTHER MAIN OR LATERAL LINES THEY SHALL BE CONTAINED WITHIN A SEPARATE, SMALLER CONDUIT.
- 17. SLEEVES UNDER EXISTING PAVEMENT MUST BE DIRECTIONAL BORE. OPEN CUT IS NOT AN OPTION.
- 18. NUMBER THE TOP OF ALL VALVE BOX LIDS WITH MINIMUM 1" HEIGHT BLACK LETTERS TO CORRESPOND TO AUTOMATIC AND GATE VALVE DESIGNATIONS. ALL HOSE BIBB VALVE BOXES SHALL BE LABELED IN A SIMILAR MANNER WITH THE DESIGNATION "HB". LETTER OUTSIDE OF TIME CLOCK CABINETS TO CORRESPOND WITH IRRIGATION CLOCK PROGRAM DESIGNATION.
- 19. THE IRRIGATION CONTRACTOR SHALL INSTALL A COLOR-CODED METAL DETECTABLE MARKING TAPE WHICH CLEARLY NOTES: "CAUTION: IRRIGATION LINE BURIED BELOW." THE TAPE SHALL BE INSTALLED THE FULL LENGTH OF THE IRRIGATION MAINLINE.
- 20. ALL VALVES, SPLICES WITHIN CONTROL LINES, AND QUICK COUPLERS SHALL BE LOCATED WITHIN NDS VALVE BOXES AS FOLLOWS:
- -RECTANGULAR 12"X17" HEAVY DUTY BOX. (PURPLE COVER FOR REUSE TO BE PROVIDED WHERE APPROPRIATE).
- 21. ALL UNSIZED PIPE SHALL BE 3/4" UNLESS OTHERWISE NOTED ON PLAN. SEE LATERAL PIPE SIZING REQUIREMENTS.
- 22. EACH TREE AND PALM (AS SHOWN ON THE PLANS) SHALL HAVE A MIN 24 GALLON PER HOUR FLOOD BUBBLER. LOCATE BUBBLER ON THE UPHILL SIDE OF TREES ON ALL SLOPES.
- 23. ALL IRRIGATION HEADS/DRIP TUBING SHALL BE LOCATED ONE (1) FOOT FROM BACK OF CURB WHEN NEXT TO A ROADWAY. (THIS SHALL NOT INCLUDE PARKING AREAS OR DRIVE AISLES).
- 24. LOCATE ALL VALVES IN PLANTING BEDS WITH A MINIMUM OF 3'-0" FROM BACK OF CURB OR EDGE OF PAVEMENT, UNLESS OTHERWISE NOTED. PIPE SIZES ON EITHER SIDE OF SECTION VALVES CONNECTING MAINLINE TO SECTION LATERAL SHALL BE ONE (1) PIPE SIZE LARGER THAN VALVE SIZE. WHERE MAINLINES RUN PARALLEL TO PAVEMENT OR CURBING, THE MAINLINE SHALL BE OFFSET 2'-0" FROM THE EDGE OF PAVEMENT OR CURB.
- 25. IRRIGATION ZONES SHALL BE SEPARATED FOR HIGH AND LOW WATER USE REQUIREMENTS AND OPERATED ON DIFFERENT WATERING CYCLES. BUBBLERS, DRIPLINE, AND SPRAY HEADS SHALL BE SEPARATED ON DIFFERENT VALVES. AT NO TIME SHALL MULTIPLE IRRIGATION HEAD TYPES BE LOCATED ON THE SAME VALVE.
- 26. ALL DRIP ZONES SHALL BE INSTALLED WITH A FLUSH VALVE AND AIR RELIEF VALVE. IN THE EVENT THAT A DRIP ZONE HAS MORE THAN ONE HIGH OR LOW POINT, MORE THAN ONE AIR RELIEF VALVE OR FLUSH VALVE WILL BE REQUIRED FOR THAT ZONE. DRIPLINE SHALL PROVIDE 0.9 GPH EMITTERS, 18" O.C. WITH 18" LINE SPACING AT A MINIMUM.

27. IRRIGATION CONTRACTOR TO COORDINATE WITH OWNER FOR FINAL CONTROLLER AND RAIN SENSOR LOCATIONS. THE CONTROLLER SHALL BE PLACED IN A LOCKING CABINET IN THE MOST INCONSPICUOUS AREA APPROPRIATE FOR ITS LOCATION. (INDOOR VS. OUTDOOR USE)

28. LOCATE THE AUTOMATIC RAIN SENSOR SHUTOFF DEVICE IN AN AREA THAT IS UNOBSTRUCTED BY TREES, ROOF OVERHANGS, OR ANY OTHER OVERHEAD OBJECT. THE SENSOR SHALL NOT BE PLACED WITHIN THE SPRAY ZONE OF ANY SPRINKLER HEAD, INCLUDING OFF-SITE IRRIGATION. CONTRACTOR SHALL LOCATE SENSOR WITHIN CLOSE PROXIMITY TO THE IRRIGATION CONTROLLER.

29. CONTRACTOR SHALL PERFORM HYDRO-TESTING OF MAIN LINES.

HYDRO-TESTING TO BE PERFORMED AS LISTED:

THE CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVE FORTY-EIGHT (48) HOURS IN ADVANCE OF TESTING. PRIOR TO BACKFILLING, CONTRACTOR SHALL FILL PIPING WITH WATER, IN THE PRESENCE OF THE OWNER'S REPRESENTATIVE, TAKING CARE TO PURGE THE AIR FROM IT. A SMALL, HIGH PRESSURE PUMP OR OTHER MEANS OF MAINTAINING A CONTINUOUS WATER SUPPLY SHALL BE CONNECTED TO THE PIPING AND SET SO AS TO MAINTAIN 125 PSI FOR TWO (2) HOURS WITHOUT INTERRUPTION. CONTRACTOR SHALL MAKE ANY NECESSARY CORRECTIONS AND RETEST THE SYSTEM UNTIL THE OWNER'S REPRESENTATIVE IS SATISFIED THAT THE SYSTEM IS REASONABLY SOUND.

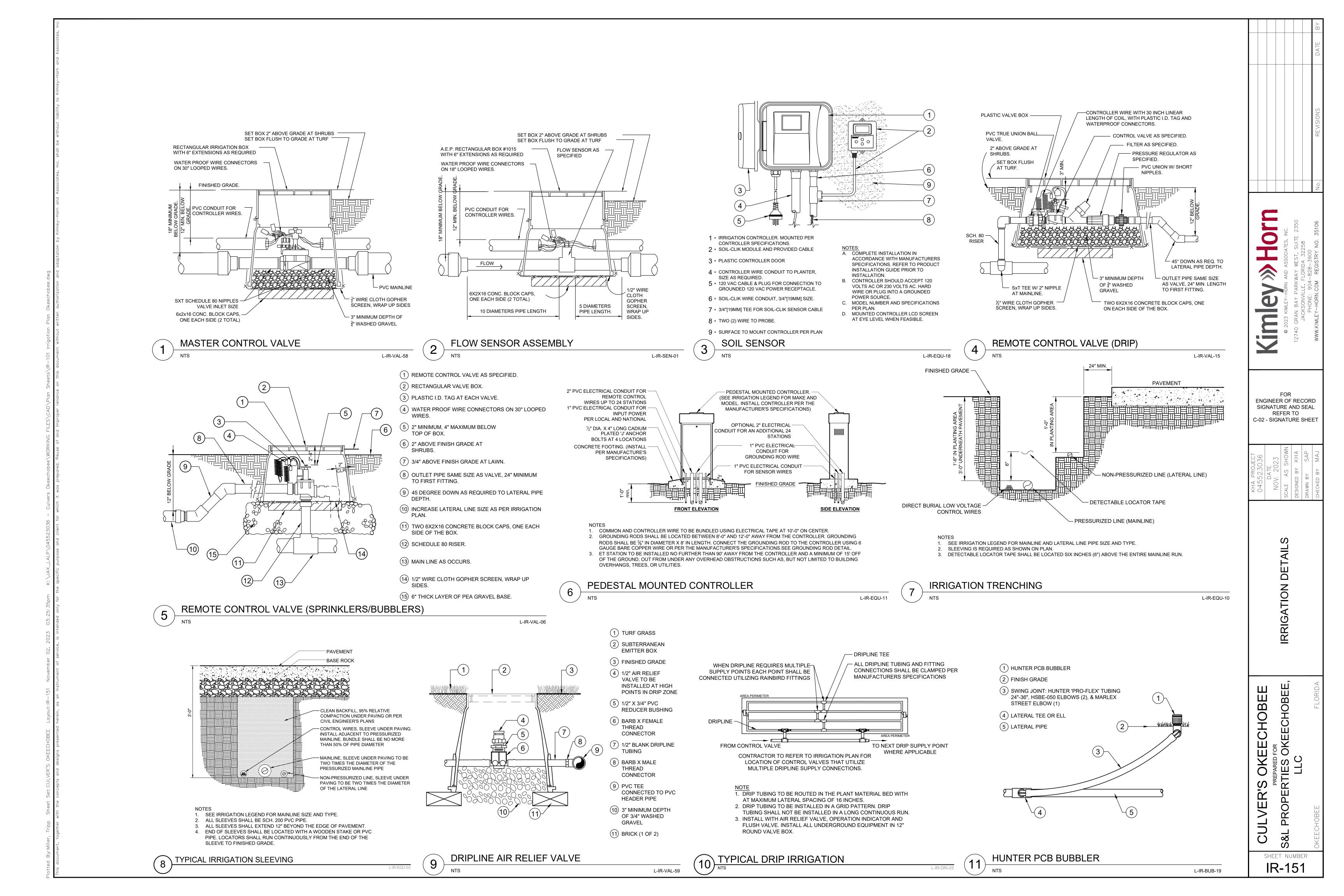
30. ALL WIRING FOR CONNECTION OF THE VALVES TO THE CONTROLLER SHALL FOLLOW MANUFACTURERS SPECIFICATIONS. IF REQUIRED, ALL WIRING FOR A TWO WIRE PATH SHALL BE WITH RED/BLUE TWISTED PAIR 14 AWG. ELECTRIC CONTROL LINES FROM THE DECODER TO THE SOLENOID VALVES SHALL BE TWISTED PAIR 18 AWG. ALL DECODERS SHALL BE GROUNDED EVERY 1,000 L.F. OR EVERY 10 DEVICES. ALL WIRE SHALL BE FURNISHED IN MINIMUM 2,500' REELS AND SPLICING SHALL BE MINIMIZED. BURY SPLICE KIT. ALL 24 VOLT WIRING SHALL BE DONE IN ACCORDANCE WITH EXISTING CODES. SPLICING SHALL BE IN VALVE BOXES OR CONTROLLERS ONLY. IRRIGATION SYSTEM CONTROL SHALL BE TWO WIRE PATH. CONTRACTOR SHALL FOLLOW ALL MANUFACTURER'S REQUIREMENTS FOR THIS INSTALLATION. TWO WIRE SYSTEM SHALL HAVE 2 -WAY COMMUNICATIONS FIELD PROGRAMMABILITY, STATION SPECIFICATIONS AND INTEGRATED SURGE PROTECTION.

31. ALL CONTROL WIRE SHALL BE INSTALLED IN A 1-1/4" ELECTRICAL CONDUIT.

32. CONTRACTOR TO MINIMIZE IRRIGATION OVERTHROW TO IMPERVIOUS AND NATURAL AREAS THROUGH FIELD ADJUSTMENTS TO INDIVIDUAL HEADS.

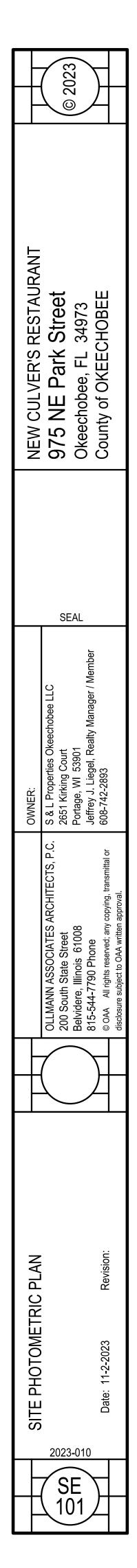
33. ALL UNIMPROVED AREAS DISTURBED DURING CONSTRUCTION SHALL BE RESTORED TO THEIR ORIGINAL CONDITION BY THE CONTRACTOR.

34. INSTALL ALL BACKFLOW PREVENTION DEVICES AND ALL PIPING BETWEEN THE POINT OF CONNECTION AND THE BACKFLOW PREVENTER AS PER LOCAL CODES. FINAL LOCATION SHALL BE DETERMINED BY THE OWNER'S AUTHORIZED REPRESENTATIVE.



	+0.3	+0.4 +0.5	+0.6	+0.6	+0.6	+0.7	+0.7	+0.7	+0.7	+0.7	+0.8	+0.7	+0.7	+0.7	+0.7	+0.7	+0.7	⁺ 0.7	+0.7	+0.7	+0.6	+0.6	+0.6	+0.6	+0.5	+ _{0.4}	+0.4
	+0.4	+D.5 +0.6	+0.8	+0.9	+0.9	+0.9	+1.0	+1.0	+1.0	+1.0	+1.0	+0.9	+ _{0.9}	+0.9	+0.9	+0.9	+1.0	+1.0	+ _{0.9}	+0.9	+0.9	+0.9	+0.8 	+0.8	+0.6	+0.5	+0.4
	+0.5	+ 0.6 + 1 ρ.8	+1.0	+1.2	+1.2	+1.3	+1.3	+1.3	+1.3	[†] 7.3	+1.2	+1.1	⁺ 1.1	+1.1	+ 1.1	+1.1	+1.2	+1.3	⁺ 1.3	⁺ 1.3	+1.3	+1.2	+1.1	+1.0 	+0.8	+0.6	+0.5
	+0.6	+p.7 +μ.9 1 μ μ	+1.2	+	+1.7	+1.7	+1.8	+1.7	+ _{1.7}	+1.5	*1.4	+1.3	+1.3	+1.3	+1.3	+1.4	+1.5	+1.7	+1.7	+1.8	+ _{1.7}	+1.6	+1.4	+1.2	+1.0	+0.8	+0.6
	+0.6	+ P .9 + I .1	+1.4	+1.8	+2.0	+2.1	+2.1	+2.0	+2.0	+1.8	+ 1 6	+ .5	+1.4	1.4	⁺ 1.5	+1.6	+1.8	+2.0	+2.1	+2.1	+2.0	+2.0	17	+1.4 	+1.2	+0.9	+0.7
	+0.6	+D.9 +1.3	+ _{1.8}	+2.0	+2.3	+2.6	+2.9	+2.4	+2.2	+2.0	+ ₁₈	+ .6	1.5	+1.5	+1.6	+1.9	+2.1	+2.3	+2.6	+ 3.0	+2.6	+2.2	+2.0	+ _{1.7} 	+1.3	+1.0	+0.7
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	+0.6	+0.9 +1.4 ■ 1.4	+2.0	+2.4	+2.6	+3.7	+4.2	+3.0	+2.5	+2.::	+ ₂₀	+1.8	1.7	+1.8	+1.9	+2.2	+2.5	+2.9	+3.3	+4.5	3.7	+2.9	+2.3	+1.9 	+1.4	0 +1.0	+0.8
	+0.6	+ 0.9 11.5	⁺ 2.2	+2.6	+2.8	+3.3	+3.7	+2.9	+2.4	+2.3	+2.0	⁺ 1.9	+1.8	+2.0	+2.2	+2.5	+2.8	+3.0	+3.5	4.2	+ _{3.5}	+2.8	+2.4	+ <u>2.0</u>	†1.5 I	+1.1	+0.8
	+0.6	⁺ 0.9 + ⁺ 1.6	+2.5	+ <u>2.7</u>	+2.8	+2.8	+2.7	+2.4	+2.2	+2.1	+2.1	† 2.0	+2.0	+2.2	+2.4	+2.7	+2.9	+ _{3.1}	+ _{3.2}	+3.4	*3.1	+2.7	+2.4	+ _{2.0} 	+1.6	⁺ 1.2	+0.8
	+0.7		+2.7	+2.9	+2.9	+2.6	+2.4	+2.2	+2.1	+2.0	+1.9	+2.0	+2.1		+2.6	+2.9	+3.1	+3.4	+3.4	+3.3	+ _{3.1}	+2.7	+2.3	+1.8	+1.5	+1.1	+0.8
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	+0.7		+2.8	+2.9	2.7	+2.2	17.6°	₹ [†] d.3	+ 1.2	+1.3	+1.5	+1.7	⁺ 2.0	EH +2.4	+2.7	+ _{3.1}	+3.9		+ _{3.7}	+3.0	+2.6	+2.3	+1.9	+1.5 	+1.2	+0.9	0.7
	+0.7	1.0 +2.2	+3.2	+3.3	+3.0	+2.3	+1,6	+1.1	+1.0	1.1	+ _{1.3}	+1.6	+1.9	+2.3	+2.6	+3.1	+3.2	4.7	- + _{3.1}	+2.9	+2.5	+2.1	+1/.17	+1.4 	+1.1	+0.8	+0.6
	+0.7	1 0. ₁ 7 1 2.1□			+3.2	+2.3	1.5		°+0.9	+1.0	+1.2	+1.5	+1.8	+2.2	+2.6	+2.8	+34	+4 1	+33	+27	+2.4	+2.0	+1.6	+ _{1.3} 	+1.0	+0.8	+0.6
1 1	+0.6	1 0.9 + 2.1	+3.1	+3.1	+2.9	+21	+1.3	+0.9	+0.8	+0.9	+1.2	⁺ 1.5	+1.8	+2.2	+2.4	+2.6	+	+ <u>3.0</u>	+ <u>-</u> 2.7	+	= +2.3	+2.0	+1.6	+1.3 	+1.0	+0.8	+0.6
	+0.5		+2.7	+2.5	+2.3	⁺ 1.8	+1.2	+0.8	+0.8	+1.0	+1.2	+1.4	+1.7	+2.0	+2.3	+2.5	2.6	2.6	2.5	2.4	= + _{2.1}	+1.8	+1.5	+1.3 	+1.0	+0.8	⁺ 0.6
31 30 <td< td=""><td>+0.3</td><td>0.4 1.2</td><td>+2.1</td><td>+1.9</td><td>+1.8</td><td>+1.4</td><td>+_{1.0}</td><td>+1.0</td><td>+1.3</td><td>+1.6</td><td>+1.6</td><td>+1.6</td><td>+1.6</td><td>+1.9</td><td>+2.2</td><td>+2.3</td><td>+2.4</td><td>+2.4</td><td>+2.3</td><td>+2.3</td><td>+2.1</td><td>+1.7</td><td>+1.4</td><td>+_{1.2} </td><td>+1.0</td><td>+0.8</td><td>⁺0.6</td></td<>	+0.3	0.4 1.2	+2.1	+1.9	+1.8	+1.4	+ _{1.0}	+1.0	+1.3	+1.6	+1.6	+1.6	+1.6	+1.9	+2.2	+2.3	+2.4	+2.4	+2.3	+2.3	+2.1	+1.7	+1.4	+ _{1.2} 	+1.0	+0.8	⁺ 0.6
	+0.2		+2.2	1.9	+1.6	+1.3	+1.3	+3.0	+5.6	+6.0	+4.0	+2.1	ARKIN	*1.8	+2.1	+2.2	+2.3	+ 2.3	+ 2.3	+2.2	+2.0	+1.7	+1.4	+1.2 	+1.0	+0.7	+0.6
	+0.2	tol2 +1.0	+1.9	+1.8	+1.8	+	+2.6 S	+76 N	+16.5 N P	+13.3 N P	+7.5 S	#5.8	+2.0	+1.8	+ _{2.1}	+2.3	+2.4	+2.4	+2.3	+2.3	+2.0	+1.7	+1.4	+1.2 	+ _{0.9}	+0.7	⁺ 0.5
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	+0.1	0.3 +0.8	+1.6	+1.5	+2.6						S	5) + 11.8	+3.3	+1.9	+2.2	+2.4	+2.7	+3.1	+2.7	+2.3	+2.1	⁺ 1.9	+1.5 	+ 1.1 	+0.8	+0.6	+0.5 Z
	+0.1		+1.7	+1.6	+2.6		<u> </u>				S	+ 11. 5)	+3.1	+1.8	+ _{2.2}	+2.5	+3.3	_ OA	+3.2	+2.5	+2.1	+1.8	+1.4	+ 1.1 	+0.8	+0.6	+0.5
	+0.1		+1.9	⁺ 1.7	+2.7) 第				GHT SIDE	↓• + 10.5	+3.0	+1.8	7.1	† 2.6	+2.7		+2.7	+2.6	2.1	+1.8	+1.4	+1.1 	+ _{0.8}	+0.6	+0.5
	+0.2		+2.1	+ _{1.8}	+4.8	• N • N	P				ENTRY-RI	5 + 10.9	+2.3	+1.8	+ _{2.2}	+2.5	+ ₃₀	+37	+ ₃₀	+24	+2.2	+ 1.8	+1.4 	+ 1.1 	+ _{0.9}	+0.6	+0.5
	+0.3		+1.9	+ _{1.9}	+2.3	+5.8	• N +132 • S			(MAIN	20.0			+2.1	+2.3	+	+	+ <u>-</u> 2.3	+2.3	+ 2.1	⁺ 1.8	+1.5	+1.2	+ _{0.9}	+0.7	⁺ 0.5
fas fu fas fus fas fa	+0.4	0,7	+2.5	+2.5	+2.5	5.4	• N 12.8 • N				N •	+9.5	+22	+16	+2.0	+2.3	2.4	2.4	2.4	2.3	+2.0	+1.7	+1.5	+1.3	+1.0	9.8	+0.6
0.5 0.9 2.2 0.2 0.1 0.2 <th0.2< th=""> <th0.2< th=""> <th0.2< th=""> <th0.2< th=""></th0.2<></th0.2<></th0.2<></th0.2<>	+0.5				+2.7	4.0	+10.9				N• N•	+10.9	+_2.3	+1.5	+1.9	+2.1	+2.2	+2.3	+2.3	+2.2	+2.0	+1.8	+ 1.6 	£.3	DRIVE THRU	+0.8	+0.6
to 4 figs	+0.5	, L' , ⊑	━╫━┼━┛		+2.7	MAILUNG		X	FRON		S) Ne	11.0			+1.7	+2.0	+2.1	+2.2	+2.2	+2.2	+2.1	+1.9	+1.7	+1.4 	+ _{1.1}	+0.9	+ _{0.7}
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	+0.4	10,9 12.3	DER		+2.3	ORDER A	• N +g.2S	N S N			N N O O	+8.3	/		+1.7	⁺ 1.9	+2.2	+2.3	+2.3	+2.3	+2.3	⁺ 2.1	+ 1.8 	+1.4	+1.2	+0.9	+ _{0.7}
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	+0.4	↓ 10.6 ↓ ↓	12 +2.0	+2.1	√+ _{1.9}	+2.0	+5.6	8.1	+9.3	† 9.2	+6.8	+3.9	1.4	1.5	+1.7	+ _{1.9}	+2.2	+2.4	+2.5	+2.5	+ _{2.4}	+2.2	ا + 1.9 /	+1.5	12 	+0.9	+ _{0.7}
$\begin{array}{c} t_{0.3} & t_{0.4} & t_{12} & t_{19} & t_{18} & t_{17} & t_{20} & t_{24} & t_{28} & t_{27} & t_{24} & t_{22} & t_{23} & t_{25} & t_{28} & t_{38} & t_{38} & t_{42} & t_{23} & t_{19} & t_{15} & t_{11} & t_{08} & t_{10} & t_{$	+0.3	F0.5	+2.1	⁺ 1.8	+1.6	+1.3	+1.4	+1.9	+2.3	+2.4	+2.0	+1.4	+1.4	+1.6	+ _{1.8}	+2.2	+2.4	+2.6	+2.9	+3.0	+2.5	+2.2	+2.0	+1.6 	+1.2	+0.9	+0.6
$\begin{array}{c} t_{0.3} \\ t_{0.2} \\ t_{0.4} \\ t_{0.2} \\ t_{0.5} \\ t_{1.0} \\ t_{1.6} \\ t_{1.0} \\$	+0.3		+2.1	- 2.0	+1.8	+1.5	+1.3	+1.3	+1.6	+1.8	+1.8	+1.8	+1.7	+1.8	+2.0	+2.3	+2.5	+2.9			+3.0	+2.2	+2.0 	+ _{1.5} 	+1.1	+0.8	+0.6
$\begin{array}{c} + 0.2 & 0.4 & 0.9 & + 1.4 & + 1.7 & + 2.0 & + 2.2 & + 2.1 & + 2.4 & + 2.9 & + 3.4 & + 3.1 & + 2.7 & + 2.4 & + 2.8 & + 2.7 & + 2.6 & + 2.5 & + 2.5 & + 2.4 & + 2.1 & + 1.9 & + 1.7 & + 1.4 & + 1.1 & + 0.8 & + 0.7 & + 0.7 & + 0.6 & + 0.9 & + 1.2 & + 1.4 & + 1.8 & + 1.9 & + 2.3 & + 2.7 & + 9.4 & + 3.1 & + 2.6 & + 2.2 & + 2.4 & + 2.2 & + 2.0 & + 2.1 & + 2.1 & + 2.0 & + 2.0 & + 1.9 & + 1.7 & + 1.4 & + 1.1 & + 0.8 & + 0.7 & + 0$	+0.3		+1.9	+2.0	+1.9	+1.8	+1.7	+2.0	+2.4	+2.8	+2.7	+2.4	+ _{2.2}	+2.2	+2.3	+2.5	AZ6	+2.8			+2.4	+2.3	+19 	+ 1.5 	+1.1	+0.8	+0.6
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	+0.2	↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓	+1.6	+1.8	+2.0	+2.0	+1.9	+2.3	+2.9	+3.3	+3.2	+2.7	+ _{2.3}	+2.4	+2.6	† 2.7	+2.7	⁺ <u>2.</u> 8	⊐+ _{3.3}	+3.4	+ _{2.7}	+2.2	+1.9	+ 1.4	+1.1	+0.8	+0.5
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	+0.2	0.4 0.9	+1.4	+1.7	+2.0	+22	+2.1	+2.4	+2.9	+34 0	+ _{3.1} B	+2.7	+2.4	+2.8	+2.7	+2.6	+2.5	2.5	>+2.5	+2.4	+2.1	1.9	+1.7	+ 1.4/ /)+ _{1.1}	+0.8	+0.5
$\frac{1}{10000000000000000000000000000000000$	+0.2	+0.3 +0.6	 + _{0.9}	 +1.2	+1.4	+1.8	+1.9	+2.3	+2.7	+ 9 .4	+3.1	+2.6	+2.2	+2.4	t22	+2.0	+2.1	+2.1	+2.0	+2.0	 + _{1.9}	 + _{1.7}	+1.4	/+1.1	+0.9	+0.7	+0.5
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	+ _{0.1}	+ _{0.1} + _{0.3}	 + _{0.4}	+0.5	+0.5	+0.5	+0.6	+0.9	+1.1	+0.8	+1.3	+1.2	+1.0	+1.0					+1.5	+1.5	+ _{1.4}	+1.3	+1.1	+0.9	+0.7	+0.6	+0.5
	+0.0	+0.1 +0.1	+0.2	+0.2	+0.3	+0.3	+0.4	+0.5	+0.6	+0.7	+0.8	+0.7	+0.7	+0.7	+0.8	+0.9	+1.0	+1.0	+ 1.0	+ 1.0	+ 1.0	+1.0	+0.9	+0.8	40.6	+0.5	+0.4
	+0.0	+0.0 +0.1	+0.2	+0.2	+0.2	+0.3	+0.4	+0.5	+0.5	ŧIĜ₽	łŴÂ	Y ^{0.7} 7	′0 ⁺ 0,7	N ^t ⁶ P	Â [™] R	K [⁺] °Š'	TRE	E ⁺ 07	+0.7	+0.7	+0.7	+0.7	+0.7	+0.6	+0.5	+0.4	+0.3
	LUN	/INAIRE S	SCHE	DULE	- BUIL	DING	MOUN	NT] /.	\sim	SITE	E PH	IOTO	OME	TRI	CP	LAN										DEL MI

SYMBOL	LABEL	CATALOGUE NUMBER	LAMP	WATTS
•	N	LDN6 40/15 LO6AR LS MVOLT GZ10 WL	LED	19
0		LITHONIA		
Ð	Р	SLIM18N	LED	18
41		RAB LIGHTING		
۵	S	OLLWU LED P1 40K 120 DDB	LED	11
		LITHONIA		

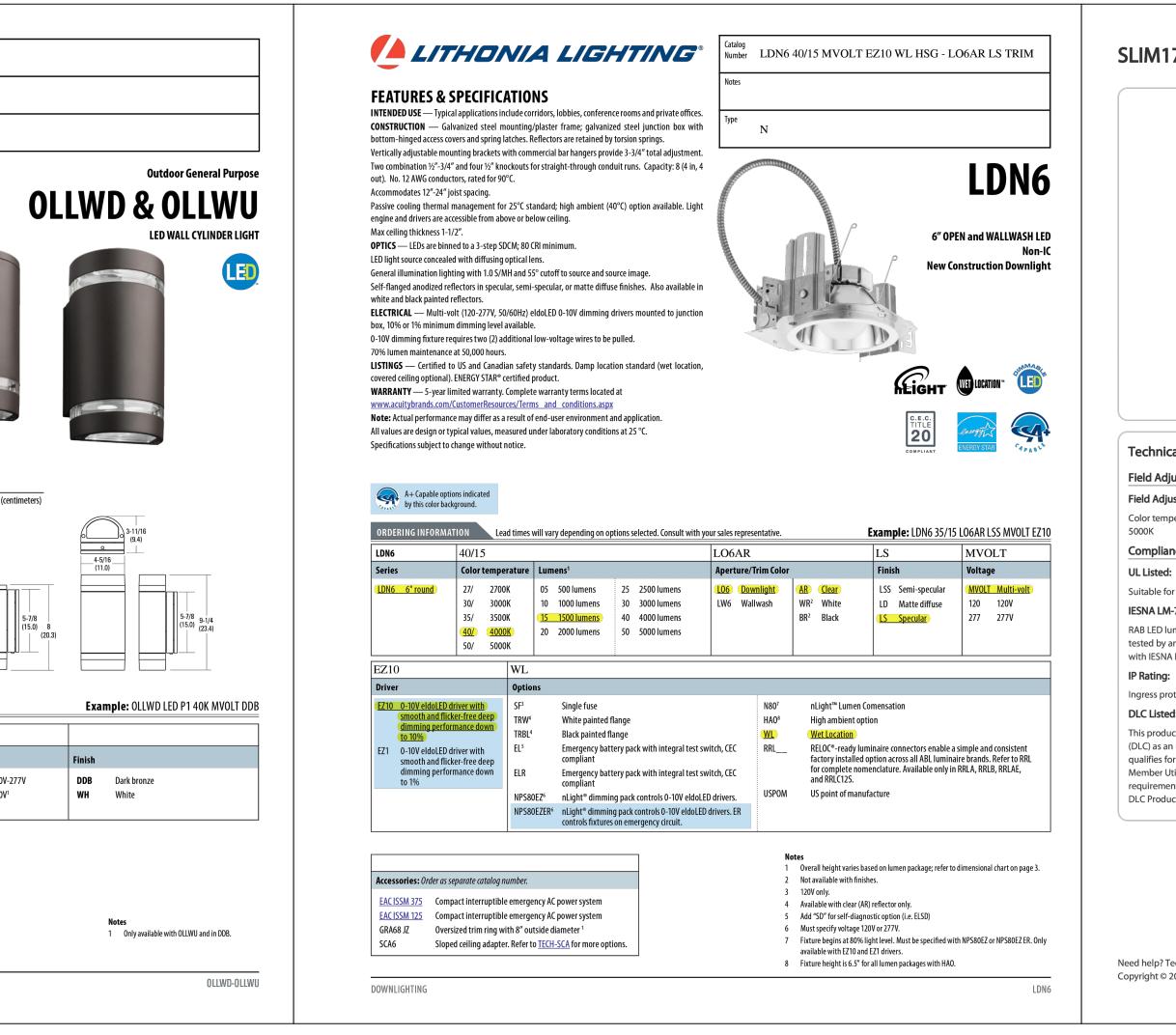


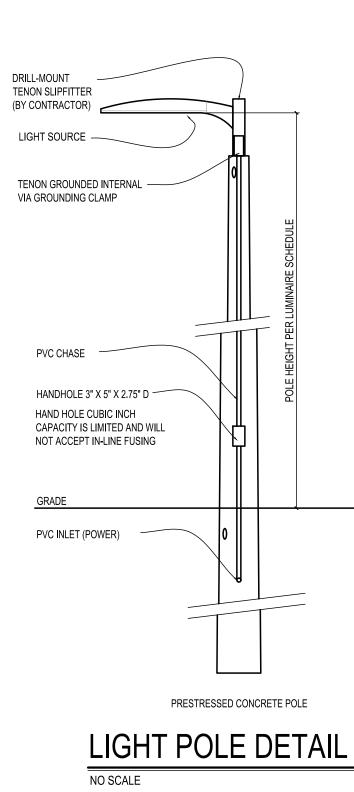
STAT	ISTIC	S			
SYMBOL	AVG	МАХ	MIN	MAX/MIN	AVG/MIN
+	2.0 fc	20.0 fc	0.0 fc	N/A	N/A

	,	E SCHEDULE - SITE LIGHT ALL BE CONCRETE	ING	PRE	IMINAF
SYMBOL	LABEL	CATALOGUE NUMBER	LAMP	WATTS	
	OA	(2) DSX1 LED P6 40K T5W MVOLT	LED	326	
		25' POLE			
	OB	DSX1 LED P6 40K T2M MVOLT	LED	163	
		25' POLE			

OLLWD LED	Downlight Up & downlight	P1	40K 4000K	MVOLT 1 120 1
Series		Performance Package	Color temperature (CCT)	Voltage
ORDERING IN	IFORMATION FO	or shortest lead times, configure produ	cts using bolded options .	
				(11.0)
				4-5/16
				3-11/16 (9.4)
	,			
	sign or typical values, me ons subject to change wit	asured under laboratory conditions a hout notice.	(2) (.	Specifications All dimensions are inche
Actual performa	nce may differ as a result	of end-user environment and applica		Specifications
		Complete warranty terms located at ces/Terms and conditions.aspx		
Tested in accorda	ince with IESNA LM-79 and	l LM-80 standards.		
LISTINGS UL Listed to U.S.	and Canadian safety stand	ards for wet locations.		aller y
Surface mounts t	to universal junction box (p	provided by others).		
1KV surge protec INSTALLATION	tion standard.			
	erature -30°C to 40°C.			
	erates on any line voltage f	from 120-277V		
Lighting Facts lal ELECTRICAL	bel on page 2 for performa	nce details.		
LUMEN MAINTEN	IANCE: The LED will delive	r 70% of its initial lumens at 50,000	hour average LED life. See	
4000K CCT LEDs. Polycarbonate le	ns protects the LED from m	noisture, dirt and other contaminants.		
OPTICS				
ADA compliant.	is using with convoluti-les	שמות שמות זה כונוכו עמוג סוטוצב UI WI		
CONSTRUCTION		istant paint in either dark bronze or wl	hite finich	
		ways and stairways for safety and secu		
INTENDED USE Provides years of	f maintenance-free illumir	nation for outdoor use in residential &	commercial applications	
	ES & SPECIFIC	ATIONS		Туре
				Notes
		VIA LIGH	ING	Number

DECORATIVE INDOOR & OUTDOOR





EPA: Length: Width: Height H1: Height H2: Weight:

DSX1 LED Shipped in NLTAIR2 PIR PIR PER PER5

> COMMERCIAL OUTDOOR

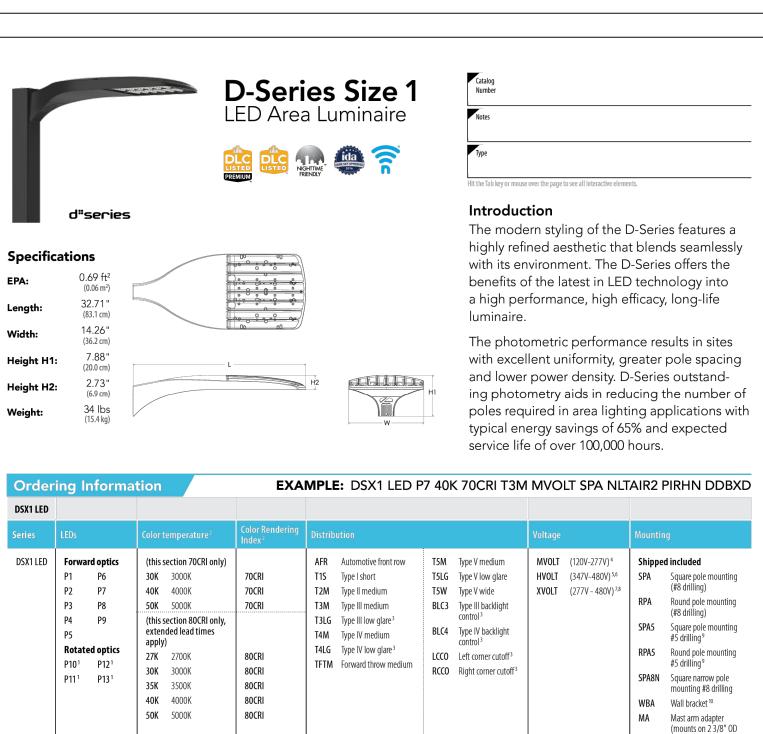
SLIM17FA15ADJ

) ()
95		Project: Prepared	4 Bar	Type:			_
Color: Bronze	Weight: 3.0 lbs	Driver Info Type (120V (208V (240V (277V (Constant Current 0.13A 0.07A 0.06A 0.05A 14.2/14/14.2W	LED Info Watts Color Temp Color Accuracy	15W 3000K/4000K/5000K 70 CRI 100,000 Hours 1761/1904/1852 lm 124/136.2/130.3 lm/W	RESTAURANT	Street
)						
nical Specifications							ark
nical Specifications Adjustability	Performance		Photocell:			VER.	La
	Performance Lifespan:		Photocell: 120-277V Integra	ated button pho	tocell included.		Л Л
Adjustability		IES LM-80	120-277V Integra		cocell included.		л Л Л
Adjustability	Lifespan: 100,000-Hour LED lifespan based on	IES LM-80	120-277V Integra Construction Cold Weather S	itarting:			5 NE P
Adjustability Adjustable: emperature selectable by 3000K, 4000K and	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations	IES LM-80	120-277V Integra Construction Cold Weather S The minimum st	itarting: arting temperatu	ıre is -40°C (-40°F)		Л Ц Л
Adjustability Adjustable: emperature selectable by 3000K, 4000K and liance	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations Wattage Equivalency:	IES LM-80	120-277V Integra Construction Cold Weather S	itarting: arting temperatu	ure is -40°C (-40°F) u re:		5 NE P
Adjustability Adjustable: emperature selectable by 3000K, 4000K and liance red:	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations Wattage Equivalency: Equivalent to 70W Metal Halide	IES LM-80	120-277V Integra Construction Cold Weather S The minimum st Maximum Amb	itarting: arting temperatu	ure is -40°C (-40°F) u re:		5 NE P
Adjustability Adjustable: emperature selectable by 3000K, 4000K and Iliance red: e for wet locations LM-79 & LM-80 Testing: D luminaires and LED components have been	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations Wattage Equivalency: Equivalent to 70W Metal Halide LED Characteristics		120-277V Integra Construction Cold Weather S The minimum st Maximum Amb Suitable for use i	itarting: arting temperatu pient Temperat n up to 50°C (12)	ure is -40°C (-40°F) ure: 2°F)		5 NE P
Adjustability Adjustable: emperature selectable by 3000K, 4000K and liance red: e for wet locations LM-79 & LM-80 Testing:	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations Wattage Equivalency: Equivalent to 70W Metal Halide LED Characteristics LEDs:		120-277V Integra Construction Cold Weather S The minimum st Maximum Amb Suitable for use i Housing:	itarting: arting temperatu pient Temperat n up to 50°C (12)	ure is -40°C (-40°F) ure: 2°F)		5 NF P
Adjustability Adjustable: emperature selectable by 3000K, 4000K and liance red: e for wet locations LM-79 & LM-80 Testing: D luminaires and LED components have been by an independent laboratory in accordance	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations Wattage Equivalency: Equivalent to 70W Metal Halide LED Characteristics LEDs: Long-life, high-efficiency, surface-mo		120-277V Integra Construction Cold Weather S The minimum st Maximum Amb Suitable for use i Housing: Precision die-cas	itarting: arting temperatu pient Temperat n up to 50°C (12)	ure is -40°C (-40°F) ure: 2°F)		5 NF P
Adjustability Adjustable: emperature selectable by 3000K, 4000K and liance ed: e for wet locations LM-79 & LM-80 Testing: D luminaires and LED components have been by an independent laboratory in accordance SNA LM-79 and LM-80	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations Wattage Equivalency: Equivalent to 70W Metal Halide LED Characteristics LEDs: Long-life, high-efficiency, surface-mod Electrical Driver: Constant Current, Class 2, 120-277V,	ount LEDs 50/60 Hz, 120V:	120-277V Integra Construction Cold Weather S The minimum st Maximum Amb Suitable for use i Housing: Precision die-cas	itarting: arting temperatu pient Temperat n up to 50°C (12)	ure is -40°C (-40°F) ure: 2°F)		5 NE P
Adjustability Adjustable: emperature selectable by 3000K, 4000K and liance red: e for wet locations LM-79 & LM-80 Testing: D luminaires and LED components have been by an independent laboratory in accordance SNA LM-79 and LM-80 ng:	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations Wattage Equivalency: Equivalent to 70W Metal Halide LED Characteristics LEDs: Long-life, high-efficiency, surface-mod Electrical Driver: Constant Current, Class 2, 120-277V, 0.13A, 208V: 0.07A, 240V: 0.06A, 2770	ount LEDs 50/60 Hz, 120V:	120-277V Integra Construction Cold Weather S The minimum st Maximum Amb Suitable for use i Housing: Precision die-cas	itarting: arting temperatu pient Temperat n up to 50°C (12)	ure is -40°C (-40°F) ure: 2°F)		5 NF P
Adjustability adjustable: emperature selectable by 3000K, 4000K and liance red: e for wet locations LM-79 & LM-80 Testing: D luminaires and LED components have been by an independent laboratory in accordance SNA LM-79 and LM-80 ng: protection rating of IP65 for dust and water	Lifespan: 100,000-Hour LED lifespan based on results and TM-21 calculations Wattage Equivalency: Equivalent to 70W Metal Halide LED Characteristics LEDs: Long-life, high-efficiency, surface-mod Electrical Driver: Constant Current, Class 2, 120-277V,	50/60 Hz, 120V: V: 0.05A ing for 0-10V e 0-10V DC	120-277V Integra Construction Cold Weather S The minimum st Maximum Amb Suitable for use i Housing: Precision die-cas	itarting: arting temperatu pient Temperat n up to 50°C (12)	ure is -40°C (-40°F) ure: 2°F)		5 NE P

RAB

Page 1 of 2

Need help? Tech help line: (888) 722-1000 Email: sales@rablighting.com Website: www.rablighting.com Copyright © 2023 RAB Lighting All Rights Reserved Note: Specifications are subject to change at any time without notice



300 3000					MA	Mast arm adapter (mounts on 2 3/8" OD horizontal tenon)	
ions			Other opti	ons	Finish (reg	uired)	
 stalled IHN nLight AIR gen 2 enabled with bi-lew ambient sensor, 8–40' mounting heig sensor enabled at 2fc: ^{11, 12, 20, 21} High/low, motion/ambient sensor, 8–4 height, ambient sensor enabled at 2fc NEMA twist-lock receptacle only (cor separate) ¹⁴ Five-pin receptacle only (controls orde 	ht, ambient FAO BL30 0'mounting BL50 1, 20, 21 trols ordered DMG	Seven-pin receptacle only (controls ordered separate) ^{14, 21} Field adjustable output ^{15, 21} Bi-level switched dimming, 30% ^{16, 21} Bi-level switched dimming, 50% ^{16, 21} 0-10v dimming wires pulled outside fixture (for use with an external control, ordered separately) ¹⁷ Dual switching ^{18, 19, 21}	Shipped i SPD20KV HS L90 R90 CCE HA Shipped s EGSR BSDB	nstalled 20KV surge protection Houseside shield (black finish standard) ²² Left rotated optics ¹ Right rotated optics ¹ Coastal Construction ²³ 50°C ambient operation ²⁴ eparately External Glare Shield (reversible, field install required, matches housing finish) Bird Spikes (field install required)	DDBXD DBLXD DNAXD DWHXD DDBTXD DBLBXD DNATXD DWHGXD		PREIMINAR
	, ,	rs, Georgia 30012 • Phone: 1-800-7 hting, Inc. All rights reserved.	705-SERV (73	378) • www.lithonia.com		DSX1-LED Rev. 03/15/23 Page 1 of 10	3





- DUMPSTER ENCLOSURE

BUILDING SIGNAGE, BY OTHERS -



SOUTH / FRONT ELEVATION



EAST / MAIN ENTRY ELEVATION

ALUMINUM ROOF LADDER w/ SECURITY SCREEN

DUMPSTER ENCLOSURE-

EXTERIOR HOLLOW METAL SHALL -BE PAINTED TO MATCH EIFS

BLUE LED STRIP LIGHT (NON-FLASHING)
 ALL TOWERS EXCEPT REAR







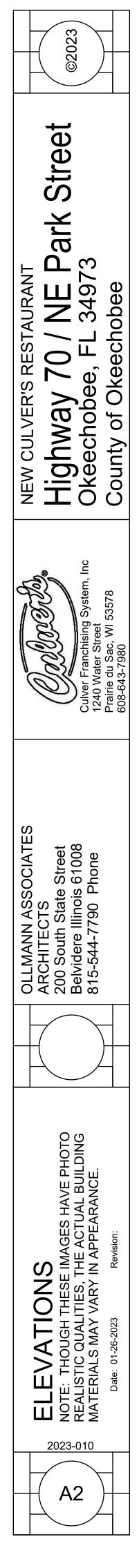
REAR RIGHT VIEW (NEC)



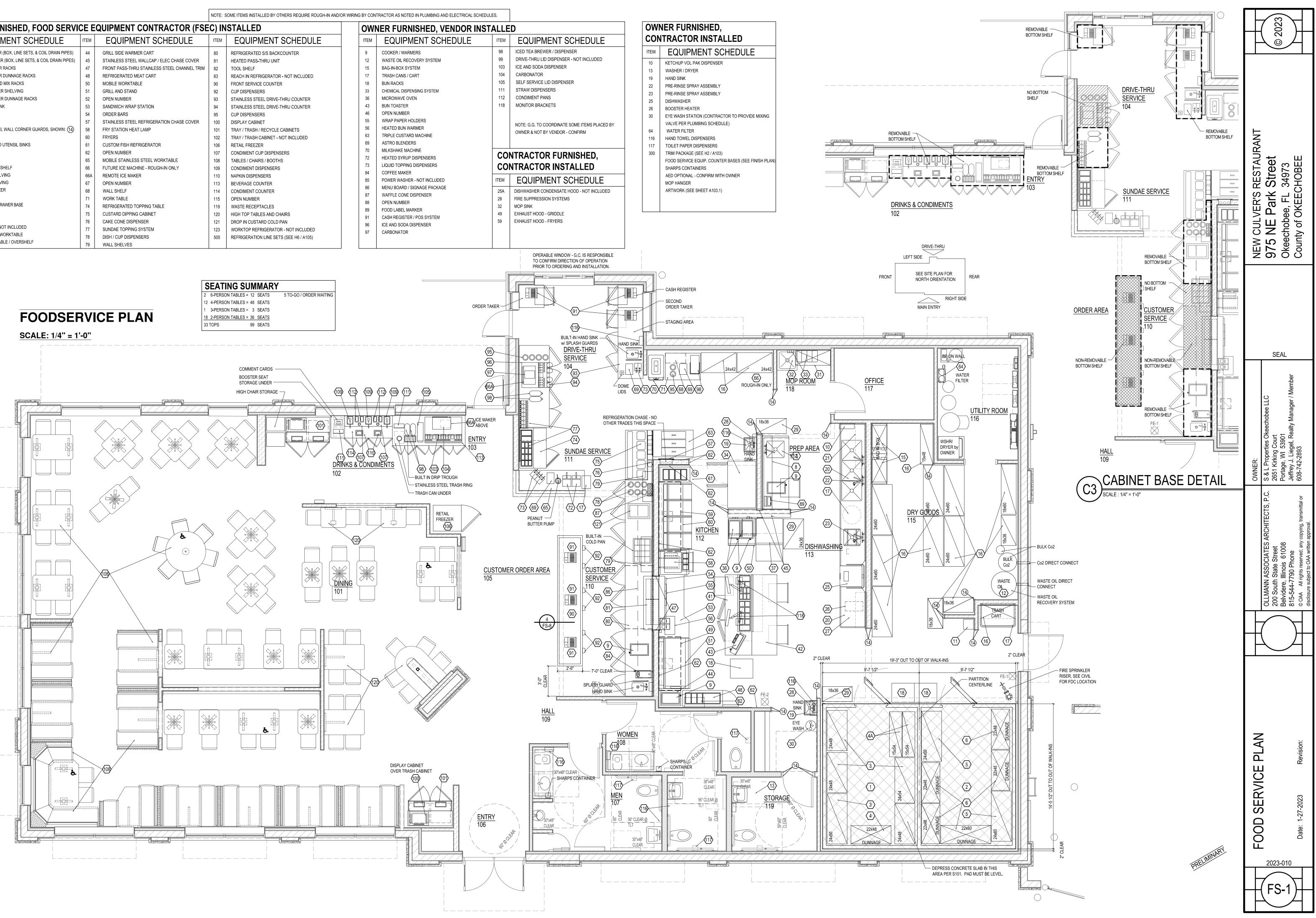


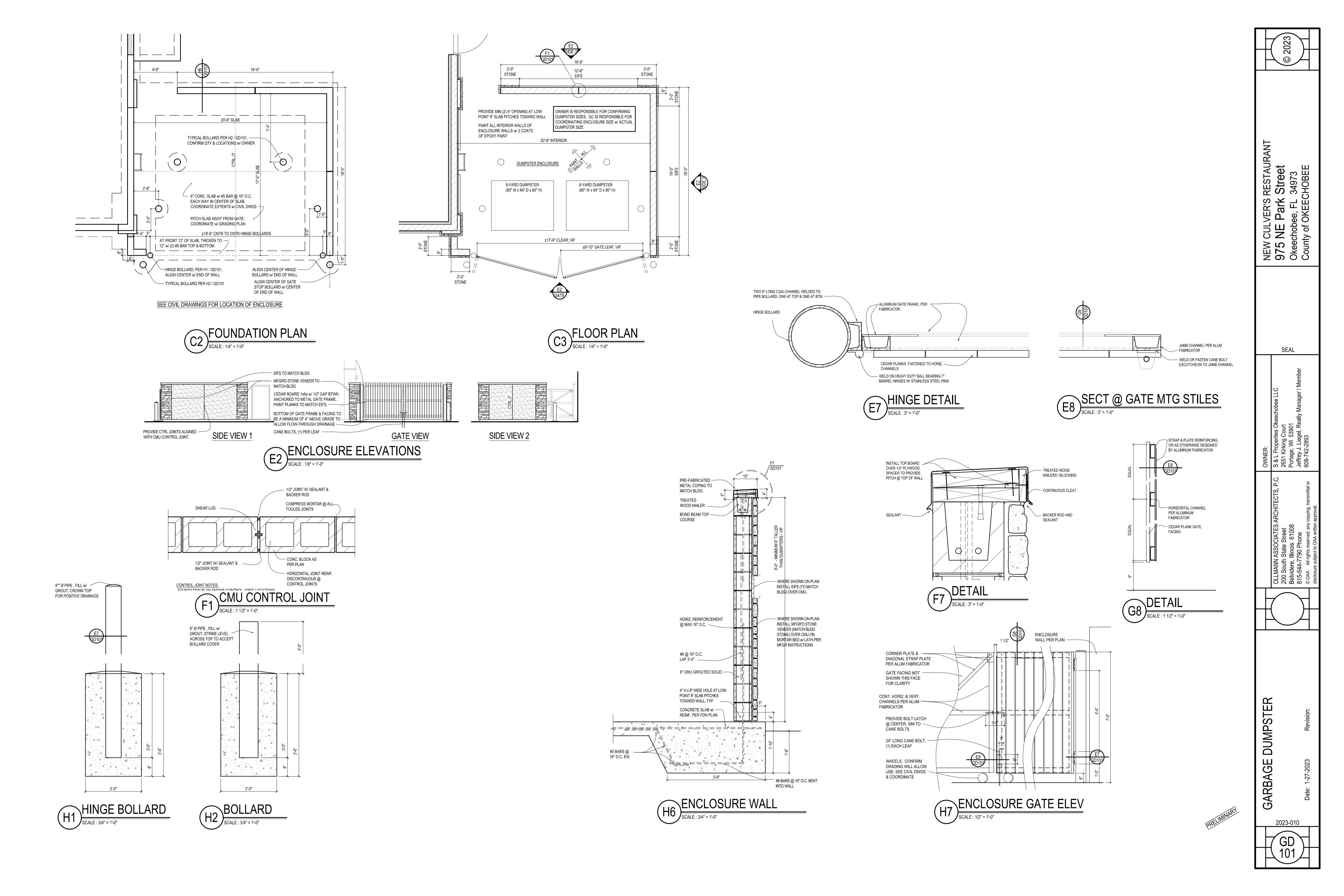


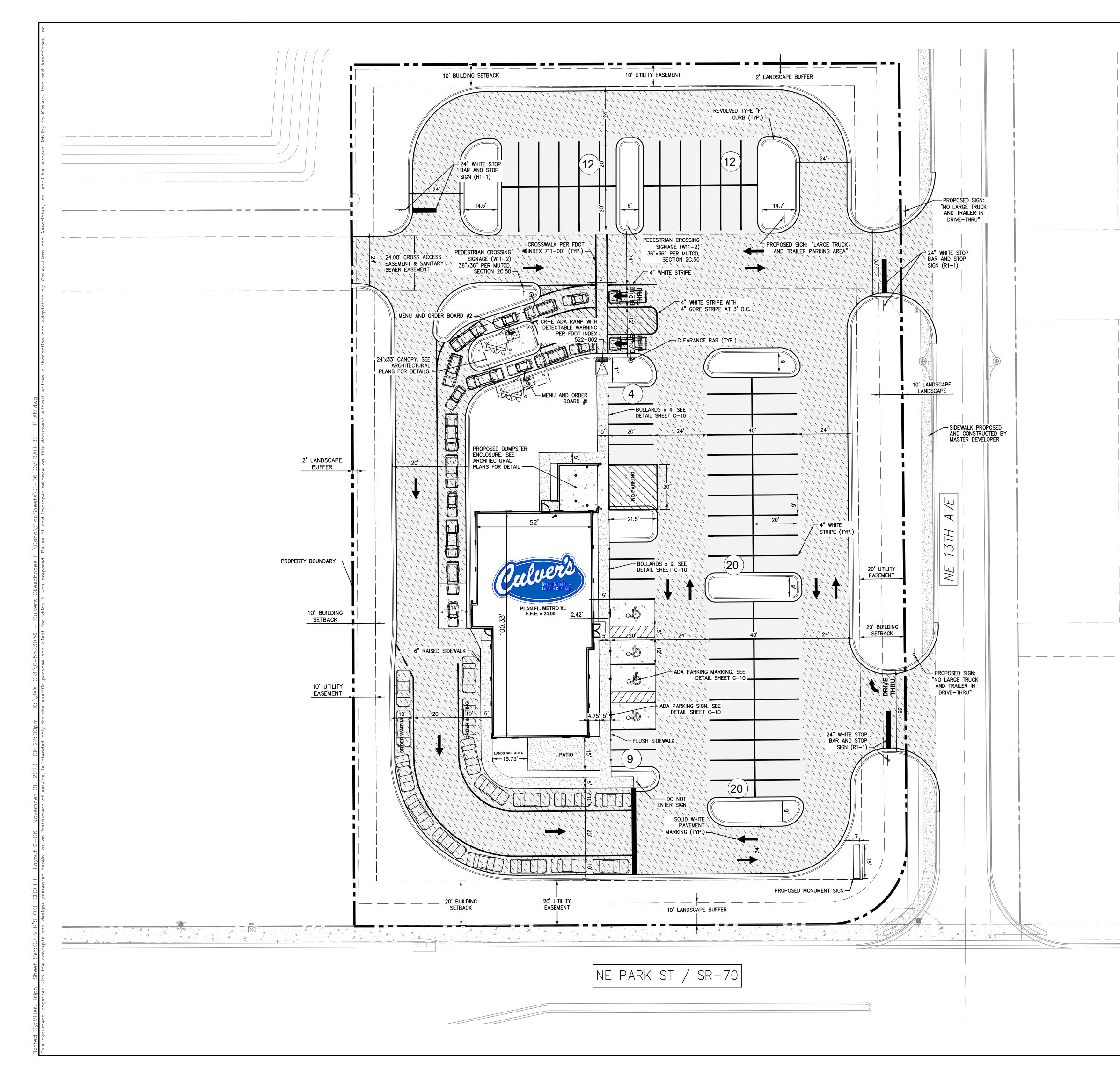
RIGHT PERSPECTIVE (EAST VIEW)

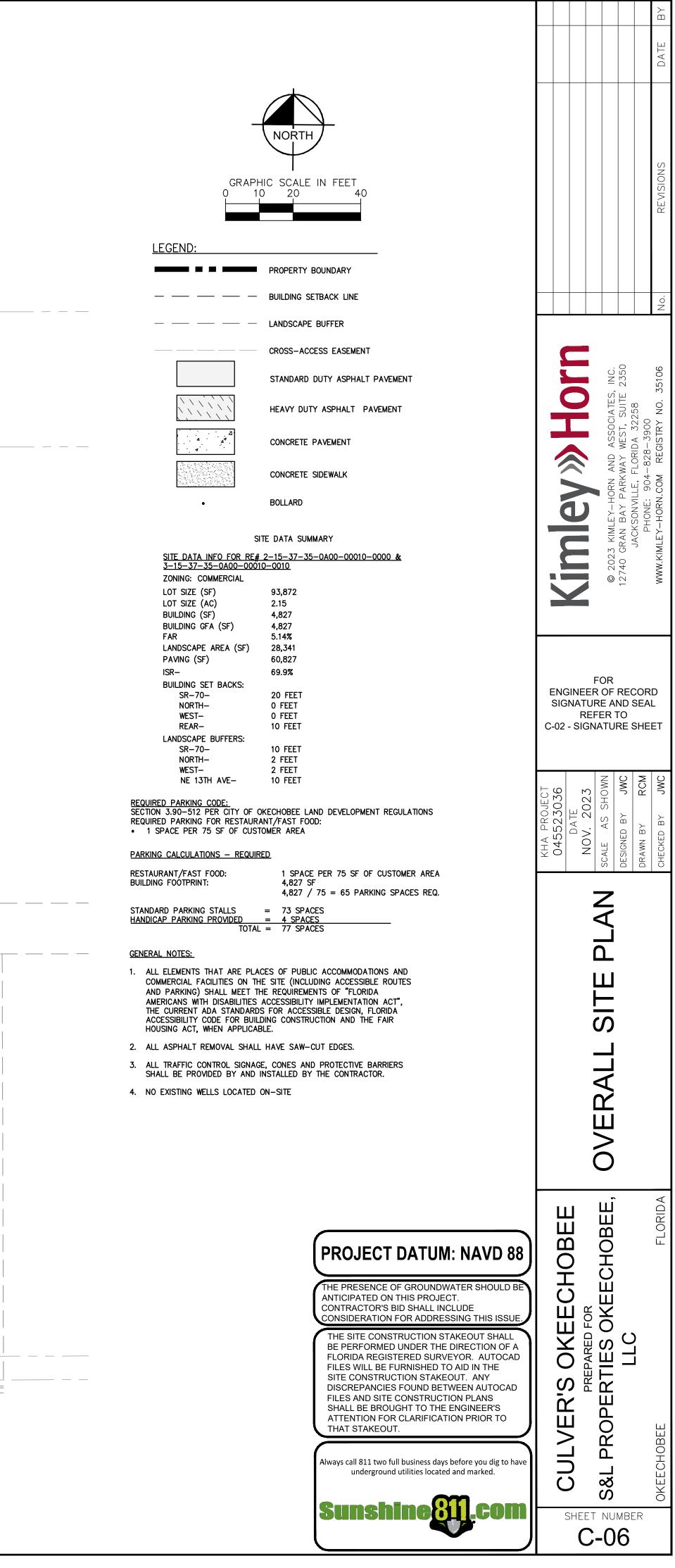


OW	NER FURNISHED, FOOD SERVI	CE E	QUIPMENT CONTRACTOR (FSE	C) IN	STALLED	OW	NER FU
ITEM	EQUIPMENT SCHEDULE	ITEM	EQUIPMENT SCHEDULE	ITEM	EQUIPMENT SCHEDULE	ITEM	EQU
1	WALK IN COOLER (BOX, LINE SETS, & COIL DRAIN PIPES)	44	GRILL SIDE WARMER CART	80	REFRIGERATED S/S BACKCOUNTER	9	COOKER /
2	WALK IN FREEZER (BOX, LINE SETS, & COIL DRAIN PIPES)	45	STAINLESS STEEL WALLCAP / ELEC CHASE COVER	81	HEATED PASS-THRU UNIT	12	WASTE OII
3	WALK-IN COOLER RACKS	47	FRONT PASS-THRU STAINLESS STEEL CHANNEL TRIM	82	TOOL SHELF	15	BAG-IN-BO
4	WALK-IN COOLER DUNNAGE RACKS	48	REFRIGERATED MEAT CART	83	REACH IN REFRIGERATOR - NOT INCLUDED	17	TRASH CA
4A	MOBILE CUSTARD MIX RACKS	50	MOBILE WORKTABLE	90	FRONT SERVICE COUNTER	18	BUN RACK
5	WALK-IN FREEZER SHELVING	51	GRILL AND STAND	92	CUP DISPENSERS	33	CHEMICAL
6	WALK-IN FREEZER DUNNAGE RACKS	52	OPEN NUMBER	93	STAINLESS STEEL DRIVE-THRU COUNTER	36	MICROWA
7	WORKTABLE / SINK	53	SANDWICH WRAP STATION	94	STAINLESS STEEL DRIVE-THRU COUNTER	43	BUN TOAS
8	WALL SHELF	54	ORDER BARS	95	CUP DISPENSERS	46	OPEN NUM
11	LOCKERS	57	STAINLESS STEEL REFRIGERATION CHASE COVER	100	DISPLAY CABINET	55	WRAP PAP
14	STAINLESS STEEL WALL CORNER GUARDS, SHOWN: (14)	58	FRY STATION HEAT LAMP	101	TRAY / TRASH / RECYCLE CABINETS	56	HEATED BI
16	OPEN NUMBER	60	FRYERS	102	TRAY / TRASH CABINET - NOT INCLUDED	63	TRIPLE CU
20	DISHTABLES AND UTENSIL SINKS	61	CUSTOM FISH REFRIGERATOR	106	RETAIL FREEZER	69	ASTRO BLI
21	WALL SHELF	62	OPEN NUMBER	107	CONDIMENT CUP DISPENSERS	70	MILKSHAKI
24	OPEN NUMBER	65	MOBILE STAINLESS STEEL WORKTABLE	108	TABLES / CHAIRS / BOOTHS	72	HEATED S
27	SLANTING WALL SHELF	66	FUTURE ICE MACHINE - ROUGH-IN ONLY	109	CONDIMENT DISPENSERS	73	LIQUID TO
29	CLEAN PAN SHELVING	66A	REMOTE ICE MAKER	110	NAPKIN DISPENSERS	84	COFFEE M
31	JANITOR'S SHELVING	67	OPEN NUMBER	113	BEVERAGE COUNTER	85	POWER W
34	REACH-IN FREEZER	68	WALL SHELF	114	CONDIMENT COUNTER	86	MENU BOA
35	NOT USED	71	WORK TABLE	115	OPEN NUMBER	87	WAFFLE C
37	REFRIGERATED DRAWER BASE	74	REFRIGERATED TOPPING TABLE	119	WASTE RECEPTACLES	88	
38	OPEN NUMBER	75	CUSTARD DIPPING CABINET	120	HIGH TOP TABLES AND CHAIRS	89	FOOD LAB
39	OPEN NUMBER	76	CAKE CONE DISPENSER	121	DROP IN CUSTARD COLD PAN	91	CASH REG
40	BREAD SHELF - NOT INCLUDED	77	SUNDAE TOPPING SYSTEM	123	WORKTOP REFRIGERATOR - NOT INCLUDED	96 07	ICE AND S
41	REFRIGERATED WORKTABLE	78	DISH / CUP DISPENSERS	505	REFRIGERATION LINE SETS (SEE H6 / A105)	97	CARBONA
42	MOBILE WORKTABLE / OVERSHELF	79	WALL SHELVES				







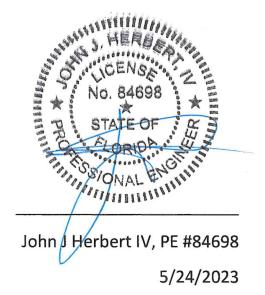


Park Street Commerce Center

Master Storm System

Storm Report by







Stormwater Summary

Park Street Commerce Center

5/24/2023

SJRWMD #:

Basin Size: 16.65 AC (Total Site)

Special Basin Criteria: Lake Okeechobee (+50% treatment over standard SFWMD criteria)

Wetlands: #56-00002-M Bluefield Ranch Mitigation Bank

Hydraulic Soils: Manatee #6 Type B/D (77%), Immokalee #11 Type B/D (23%)

Impervious Area: 0.05 AC existing, 6.80 AC Total Impervious Area

Treatment Volume Required: 1.5 " over basin or 2.5" x % impervious plus 50%

Peak Design Storm: 100 Yr - 72 hr

Pre vs Post Discharge: S-133 Basin 15.6 cfs/SM for 25YR-72HR storm or 0.36 cfs, proposed 0 cfs

Pond Volume: Dry Pond 4.938 AF (22 Top, 17.60 Bottom), Wet Pond 4.07 AF (20 Top, 15 NWL)

Pond Recovery: 72 hours

Seasonal High Groundwater Elevation: 14.50 elv. (See Geotech Report)

FEMA: Zone X (12093C0480C - 7/16/2016)

Slopes: 4:1 Dry pond

Maintenance Berm: 10 ft

Fencing: No fence proposed







Site Location / Existing Conditions:

The project is 16.65 acres in size and located on 1000 East S.R.70 in the City of Okeechobee, Florida. The site is currently undeveloped apart from an unoccupied house in the back northeast corner. A man-made drainage swale has been cut through the center of the site emptying into Taylor Creek in the north. Several isolated wetlands are present on the property. Topography of the site flows from higher elevations in the east to lower elevations in the west, ultimately discharging north into the creek. During geotechnical investigations organic muck layers were discovered in portions of the site (approx. 27%) that correlated with higher water tables relative to the borings without muck.

Proposed Conditions:

Project proposes four individual commercial lots and city-controlled roadway created with master stormwater. Lot 1 (1 AC impervious) is proposed carwash, Lot 2 (1.5 AC impervious) is Culver's restaurant, Lot 3 & 4 have no current planned development but are still allotted 1.75 AC of impervious area each in the master system. The proposed roadway is allotted 0.80 AC of impervious in the same shared system.

Water Quality:

The site requires nutrient loading be met for Lake Okeechobee basin and includes 50% additional treatment volume over the standard requirements. SFWMD criteria of 1" over basin or 2.5" over percent impervious area (less roofs & wet ponds). The system is designed to exceed the required 2.13 AC-FT of dry retention over the basin providing 3.58 AC-Ft before discharging into the wet pond. All required treatment is met within the dry pond, additional treatment is provided in the wet pond further surpassing the requirements.

Water Quantity:

Rainfall tables used from SFWMD regarding the 10 year – 1 day (5"), 25 year – 3 day (9") and 100 year – 3 day (10") storm events were used to set the final elevations of roads and buildings via ICPR routing simulation. The proposed storm system maintains the 100 Year storm event with additional freeboard. No outfall is proposed.

	Dry Pond (elv)	Wet Pond (elv)
No Outfall – 100 YR – 72 Hour storm :	21.57	18.95
No Outfall – 25 YR – 72 Hour storm :	21.30	18.09
No Outfall – 10 YR – 24 Hour storm :	19.76	15.53

provided min. road elv = 20.56



Water Discharge:

No discharge is proposed, all storm events held within proposed storm system.

System Recovery:

Drawdown of the storm pond was performed using a 3-day recovery via groundwater with percolation rates at half rate given in geotech report. The dry pond holds 3.58 AC-FT at elevation 20.95 before discharging into the wet pond at the rear of the site for attenuation. The required treatment volume of 2.13 AC-FT recovers within the required 72 hour simulation with total recovery in 72 hour window equaling recovers 2.32 AC-FT

Operation & Maintenance:

Lots 1 - 4 will be owned and operated by separate entities. The proposed roadway, storm easement and rear pond will be owned and operated by City of Okeechobee.

Dry Pond Stage / Storage					
Stage (ft)	Area (sf)	Area (ac)	Volume (ac-ft)		Notes
17.6	37,464	0.860	0.000		
18.0	41,429	0.951	0.362		
19.0	45,499	1.045	1.360		
20.0	49,670	1.140	2.452		
21.0	54,134	1.243	3.644		
22.0	58,598	1.345	4.938		
Rear Dry Pond					TOP
Wier Elevation					
Design Wier Elevation =			20.95	ft	
Treatment Volume Provided =			3.584	ac-ft	

Stage (ft)	Area (sf)	Area (ac)	Volume (ac-ft)	Notes
6.0	7,866	0.181	0.000	
7.0	8,677	0.199	0.190	
8.0	9,534	0.219	0.399	
9.0	10,440	0.240	0.628	
10.0	11,401	0.262	0.879	
11.0	12,423	0.285	1.152	
12.0	13,519	0.310	1.450	
13.0	28,332	0.650	1.931	
14.0	33,541	0.770	2.641	
15.0	38,878	0.893	0.000	Waterline (2.641 AF vol
16.0	44,343	1.018	0.955	
17.0	49,929	1.146	0.000	
18.0	55,619	1.277	1.212	
19.0	61,409	1.410	2.555	
20.0	70,882	1.627	4.073	ТОР
r Elevation				

<u>Development Drainage Basin A</u>						
Basin	Area (AC)	% impervious	Cover	CN	Imperv (AC)	
Commerical	13.940	49%	Impervious	98	6.800	
			Drained Pervious (Type B Soil)	40		
			Roof			
			Sub-Total CN	68		
Totals	13.940	49%		68	6.800	

<u>Development Drainage Basin B</u>						
Basin	Area (AC)	%Water	Cover	CN	Imperv (AC)	
Wet Pond / FPL	2.71	33%	Water	100	0.000	
			Pervious (Type B Soil)	60		
Totals	2.71	33%		73	0.000	
Total Site	16.65	41%			6.800	

Time of Concentration

Assume Minimum 15 minimum for all basins

Treatment Volumes

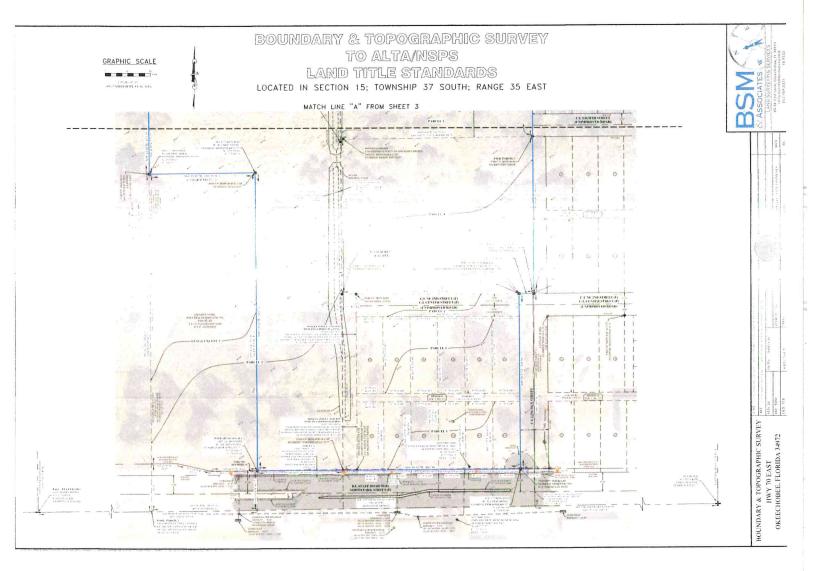
Basin Are Imperviou	 16.65 AC 41%	6.80	AC
1	velopment (16.65 AC 1.420 AC-FT 1 50% 2.09 AC-FT		
2	Dervious area " x 41% x 16.65 1.42" AC-FT 1 50% 2.130 AC-FT	5 AC	6.8 AC total Impervious

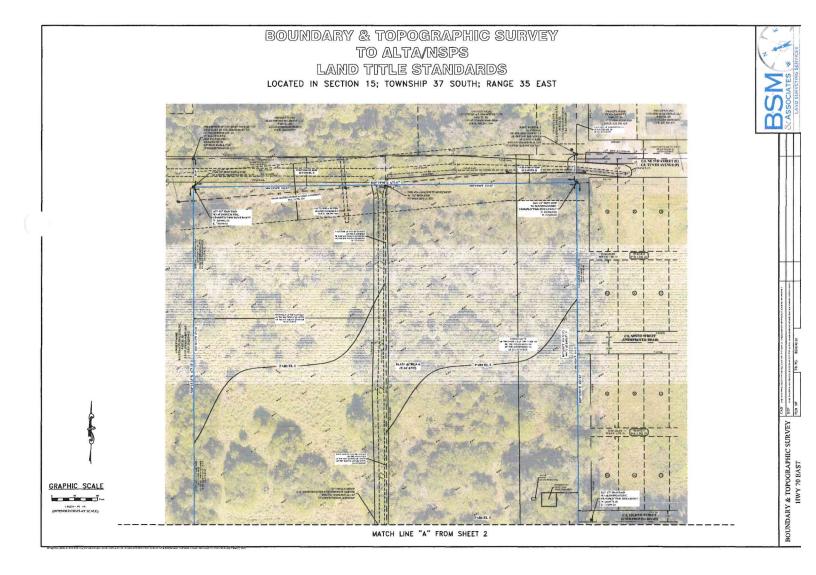
Impervious Area Accounted

Lot 1	1.00 AC
Lot 2	1.50 AC
Lot 3	1.75 AC
Lot 4	1.75 AC
Roadway	0.80 AC
Total	6.80 AC



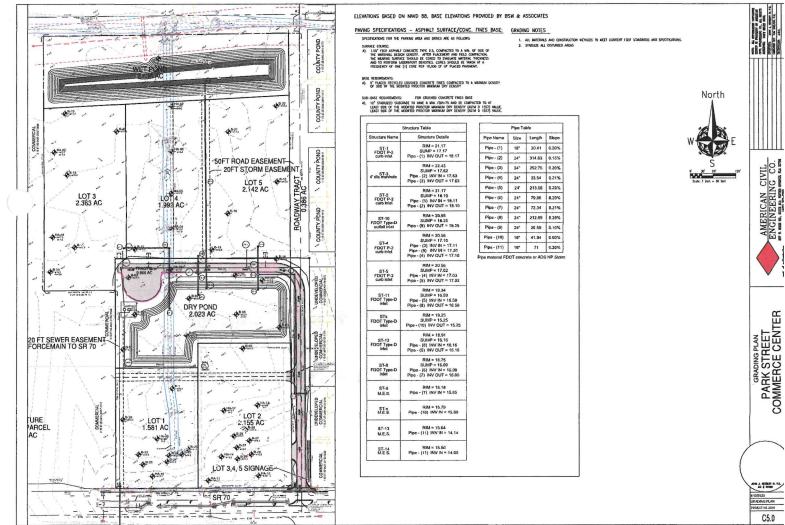
PRE-BASIN MAP







POST-BASIN MAP

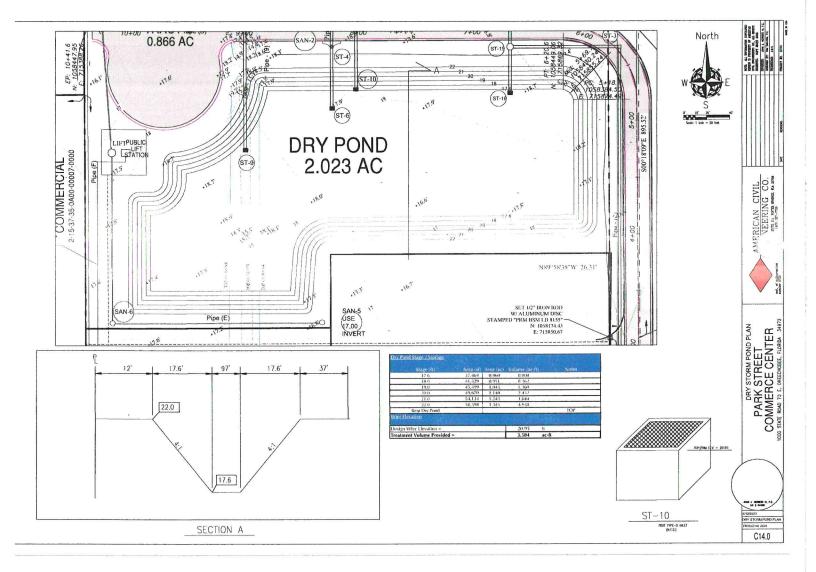


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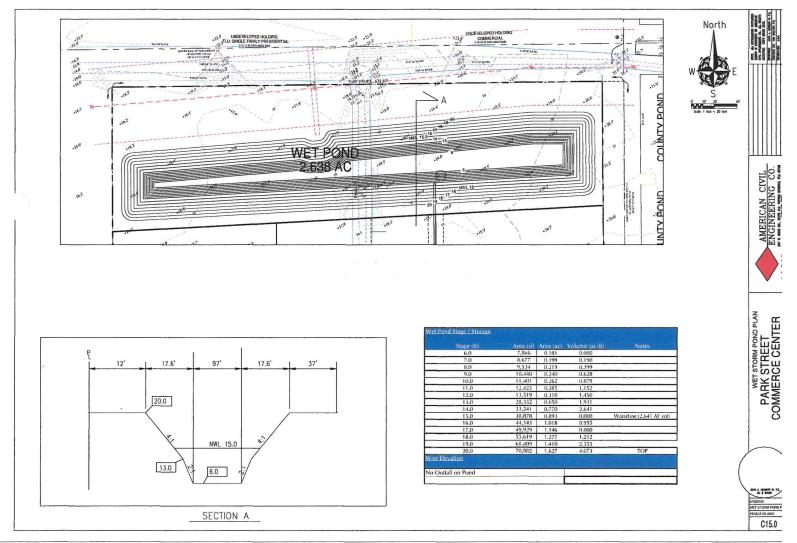
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ELEVATIONS BASED ON NAVD 88, BASE ELEVATIONS PROVIDED BY 854 & ASSOCIATES

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FEMA MAP

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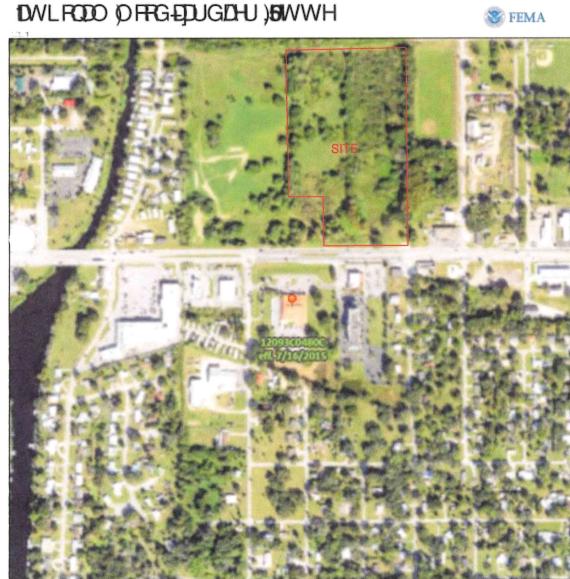
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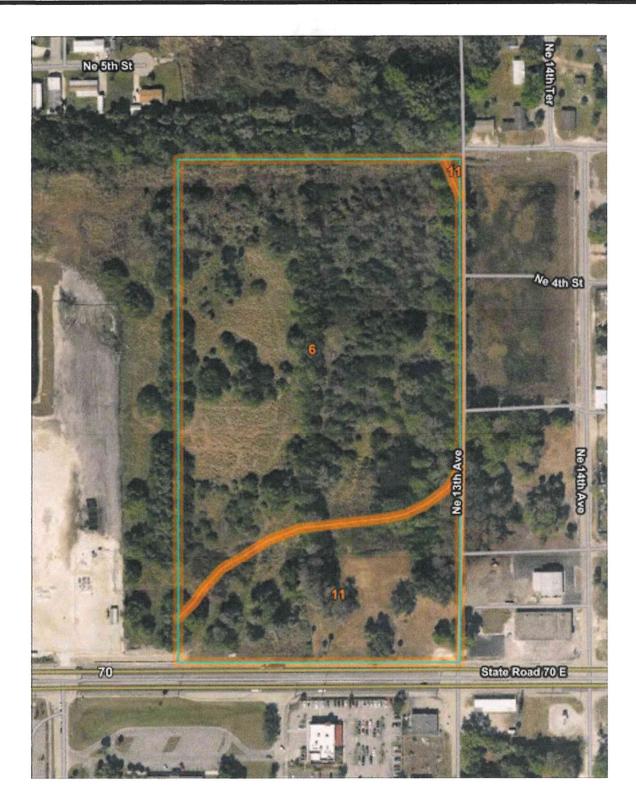
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SOIL MAP



USDA SOILS SURVEY

6—Manatee loamy fine sand, frequently ponded, 0 to 1 percent slopes 11—Immokalee fine sand, 0 to 2 percent slopes



ICPR

Node Max Conditions [Recover]

Node Name	Sim Name	Warning Stage [ft]	Max Stage [ft]	Min/Max Delta Stage [ft]	and the second	Max Total Outflow [cfs]	Max Surface Area [ft2]
Groundwater	Recover	14.50	14.50	0.0000	4.06	0.00	0
Recover	Recover	23.00	20.95	-0.0010	0.00	4.06	53921

Node Max Conditions [Scenario1]

Node Name	Sim Name	Warning Stage [ft]	[ft]	Min/Max Delta Stage [ft]	Max Total Inflow [cfs]	Max Total Outflow [cfs]	Max Surface Area [ft2]
POST WET POND	100YR-72HR	20.00	18.95	0.0010	15.34	0.00	64129
Post Dry Pond	100YR-72HR	22.00	21.57	0.0010	47.00	10.97	56689
Pre- Node	100YR-72HR	20.00	14.00	0.0000	27.00	0.00	0
POST WET POND	10YR-24HR	20.00	15.53	0.0003	5.46	0.00	42298
Post Dry Pond	10YR-24HR	22.00	19.76	0.0010	23.63	0.00	48646
Pre- Node	10YR-24HR	20.00	14.00	0.0000	7.23	0.00	0
POST WET POND	25YR-72HR	20.00	18.09	0.0010	8.46	0.00	58643
Post Dry Pond	25YR-72HR	22.00	21.30	0.0010	40.69	6.65	55482
Pre- Node	25YR-72HR	20.00	14.00	0.0000	22.73	0.00	0

Simple Basin: POST-BASIN DRY

The second se	
Scenario:	Scenario1
Node:	Post Dry Pond
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	15.0000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH323
Peaking Factor:	323.0
Area:	13.9400 ac
Curve Number:	68.0
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Construction sectors and action a president metalogic

Scenario:	Scenario1
Node:	POST WET POND
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	15.0000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH323
Peaking Factor:	323.0
Area:	2.7120 ac
Curve Number:	72.0
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: PRE-BASIN

Scenario: Scenario1 Node: Pre- Node Hydrograph Method: NRCS Unit Hydrograph Infiltration Method: Curve Number Time of Concentration: 45.0000 min Max Allowable Q: 0.00 cfs 1

Comment:

Node: Groundwater				
Scenario:	Recover			
Type:	Time/Stage			
Base Flow:	0.00 cfs			
Initial Stage:	14.50 ft			
Warning Stage:	14.50 ft			
Boundary Stage:				
Year Month	Day	Hour	Stage [ft]	
		0.0000		14 50

0	0	0	0.0000	14.50
0	0	0	72.0000	14.50

Comment:

Node: Recover Scenario: Recover Tyre: Stare/A

Type: Stage/Area Base Flow: 0.00 cfs Initial Stage: 20.95 ft Warning Stage: 23.00 ft

Stage [ft]	Area [ac]	Area [ft2]
17.60	0.8600	37462
18.00	0.9510	41426
19.00	1.0450	45520
20.00	1.1400	49658
21.00	1.2430	54145
22.00	1.3450	58588

Comment:

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Node:	Same and	States and the states of			A 16.00	and a lot

Scenario:	Scenario1
Type:	Stage/Area
Base Flow:	0.00 cfs
Initial Stage:	15.00 ft
Warning Stage:	20.00 ft

Stage [ft]	Area [ac]	Area [ft2]
15.00	0.8930	38899
20.00	1.6270	70872

Comment:

Node: Post Dry Pond

Scenario:	Scenario1
Type:	Stage/Area
Base Flow:	0.00 cfs
Initial Stage:	17.60 ft
Warning Stage:	22.00 ft

dage (fil	Anex (Hz)	A STATE OF A
17.60	0.8600	37462
18.00	0.9510	41426
19.00	1.0450	45520
20.00	1.1400	49658
21.00	1.2430	54145
22.00	1.3450	58588

Comment:

Node: Pre- Node Scenario: Scenario1 Type: Time/Stage Base Flow: 0.00 cfs Initial Stage: 14.00 ft Warning Stage: 20.00 ft Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	14.00
0	0	0	72.0000	14.00

Comment:

THE REPORT

Percolation Link: L-0110PERC			
Scenario:	Recover	Surface Area Option:	Vary Based on Stage/Area
From Node:	Recover		Table
To Node:	Groundwater	Vertical Flow Termination:	Horizontal Flow Algorithm
Link Count:	1	Perimeter 1:	1152.00 ft
Flow Direction:	Both	Perimeter 2:	1602.00 ft
Aquifer Base Elevation:	5.00 ft	Perimeter 3:	2216.00 ft
Water Table Elevation:	14.50 ft	Distance P1 to P2:	50.00 ft
Annual Recharge Rate:	0 іру	Distance P2 to P3:	100.00 ft
Horizontal Conductivity:	7.500 fpd	# of Cells P1 to P2:	50
Vertical Conductivity:	6.500 fpd	# of Cells P2 to P3:	50
Fillable Porosity:	0.250		
Layer Thickness:	3.10 ft		
Comment: 1/2 the perc rate for	FS		

Drop Structure Link:	Dry to Wet	Upstrea	am Pipe	Downstr	eam Pipe
Scenario:	Scenario1	Invert:	16.50 ft	Invert:	13.00 ft
From Node:	Post Dry Pond	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	POST WET POND	Geometry	y: Circular	Geometry	/: Circular
Link Count:	1	Max Depth:	1.50 ft	Max Depth:	1.50 ft
Flow Direction:	Both			Bottom Clip	
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	538.00 ft			Top Clip	
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.00	Op Table:		Op Table:	
Exit Loss Coef:	0.00	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				
Pipe Comment:					

Weir Co	mponent		
Weir:	1	Botto	m Clip
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Horizontal	Тор	Clip
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	20.95 ft	Op Table:	
Control Elevation:	20.95 ft	Ref Node:	
Max Depth:	2.00 ft	Discharge (Coefficients
Max Width:	3.00 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600

Orifice Table:

5

Weir Comment:

Drop Structure Comment:

Scenario: Recover Run Date/Time: 5/24/2023 11:28:42 AM Program Version: ICPR4 4.07.08 Run Mode: Normal Month Day Hour [hr] Year 0.0000 Start Time: 0 0 0 0 72.0000 0 End Time: 0 Surface Hydraulics Hydrology [sec] [sec] Min Calculation Time: 60.0000 0.1000 30.0000 Max Calculation Time: Da 0 0.0000 15.0000 0 0 Time Increment [min] 15.0000 0.0000 0 0 0 Save Restart: False Lookup Tables Boundary Stage Set: Rainfall Folder: Extern Hydrograph Set: Curve Number Set: Unit Hydrograph Folder: Green-Ampt Set: Vertical Layers Set:

Impervious Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight	0.5 dec		
Fact:			
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain	Global
		Opt:	
Max dZ:	1.0000 ft		
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~SFWMD-72
		Rainfall Amount:	0.00 in
Edge Length Option:	Automatic	Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area	
		(1D):	
		Energy Switch (1D):	Fnerov
		Energy Stricer (19).	2.10.97

Comment:

Simulation: 100YR-72HR				
	Scenario1			
	5/24/2023 11:28:49 AM			
Program Version:				
Frogram version.	ICFRT 4.07.00			
		General		
Run Mode:	Normal			
	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000
	Hydrology [sec]	Surface Hydraulics		
		[sec]		
Min Calculation Time:	60.0000	0.1000		
Max Calculation Time:		30.0000		
		Output Time Increments		

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000
Resta	rt File			
Save Restart:	a fight have been an a start of the second start of			
		Resources & Lookup Table	2S	
Reso	urces		Lookup	Tables
Rainfall Folder:		909 9	Boundary Stage Set:	
			Extern Hydrograph Set:	
Unit Hydrograph Folder:			Curve Number Set:	
Folder:			Green-Ampt Set:	
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			Impervious Set:	
		Tolerances & Options		
Time NA 11	CAOD			24 0000 br
Time Marching:	SAOR		IA Recovery Time:	24.0000 hr
Max Iterations: Over-Relax Weight	6 0.5 dec			
Fact:	0.5 dec			
dZ Tolerance:	0.0010 ft		Smp/Man Basin Rain	Global
			Opt:	
Max dZ:	1.0000 ft			
Link Optimizer Tol:	0.0001 ft		Rainfall Name:	~SFWMD-72
			Rainfall Amount:	10.00 in
Edge Length Option:	Automatic		Storm Duration:	72.0000 hr
			Dflt Damping (1D):	0.0050 ft
			Min Node Srf Area	100 ft2
			(1D):	
			Energy Switch (1D):	Energy
Comment:				
				- 6 - 6 - 6 - 6 - 6 - 6 - 6 - 6 - 6 - 6
Simulation: 10YR-24HR				
Scenario:	Scenario1	n name di sensa fina pina di se dan dan manan mada di sensa da dan pilakan di sensa di sebagai di sensa di seb		e mananan et anni et an anna et an
Run Date/Time:	5/24/2023 11:28:59 AM			
Program Version:	ICPR4 4.07.08			
		General		
Run Mode:	Normal		and take a second and a second se	

Month

Year

Day

Hour [hr]

7

				8
Start Time: End Time:	0 0	0 0	0 0	0.0000 24.0000
	Hydrology [sec]	Surface Hydraulics [sec]		
Min Calculation Time: Max Calculation Time:	60.0000	0.1000 30.0000		
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Hydr	ology			
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Unit Hydrograph Folder:			Curve Number Set:	
			Green-Ampt Set:	
			Vertical Layers Set: Impervious Set:	
		Tolerances & Options		
Time Marching: Max Iterations:	SAOR 6		IA Recovery Time:	24.0000 hr
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dZ Tolerance:	0.0010 ft		Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft			
Link Optimizer Tol:	0.0001 ft		Rainfall Name: Rainfall Amount:	~SCSII-24 5.00 in
Edge Length Option:	Automatic		Storm Duration:	24.0000 hr
			Dflt Damping (1D): Min Node Srf Area	0.0050 ft 100 ft2
	Automatic		Dflt Damping (1D):	0.0050 ft

Energy Switch (1D): Energy

Comment: Scenario: Scenario1 Run Date/Time: 5/24/2023 11:29:01 AM Program Version: ICPR4 4.07.08 Run Mode: Normal Hour [hr] Year Month Day 0.0000 Start Time: 0 0 0 72.0000 0 0 End Time: 0 Hydrology [sec] Surface Hydraulics [sec] 60.0000 0.1000 Min Calculation Time: Max Calculation Time: 30.0000 Time Increment [min] 0 0.0000 15.0000 0 0 15.0000 0.0000 0 0 0 **Restart File** Save Restart: False Lookup Tables Resources Boundary Stage Set: Rainfall Folder: Extern Hydrograph Set: Curve Number Set: Unit Hydrograph Folder: Green-Ampt Set: Vertical Layers Set: Impervious Set:

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Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6		
Over-Relax Weight	0.5 dec		
Fact:			
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain	Global
		Opt:	
Max dZ:	1.0000 ft		
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~SFWMD-72
•		Rainfall Amount:	9.00 in
Edge Length Option:	Automatic	Storm Duration:	72.0000 hr
		Dflt Damping (1D):	0.0050 ft
		Min Node Srf Area	100 ft2
		(1D):	
		Energy Switch (1D):	Energy
Comment:			



RECOVERY





GEOTECH REPORT

Headquarters 11345 U.S. Highway 1 Sebastian, FL. 32958 Orlando 723 Progress Way Sanford, FL. 32771



Mailing P.O. Box 78-1377 Sebastian, FL. 32978 Phone: 772-589-0712 C.A. # 5693 KSMengineering.net

December 30, 2022

Workspace Collective Adam Ramsay 603 E. Fort King Street Ocala, FL 34471

Re: 1000 State Road 70 Okeechobee, Florida KSM Project #: 2210339-b&p

Dear Mr. Ramsay:

As requested, KSM Engineering & Testing has performed a preliminary subsurface investigation at the referenced site. The purpose of this investigation was to determine the general nature of the subsurface conditions at the subject property and to offer preliminary guidance on the development of the property for its intended use. Presentation of the data gathered during the investigation, together with our geotechnical related opinions, are included in this report.

Scope of Work and Professional Service Agreement:

The scope of work and the agreement to perform a geotechnical exploration was provided by KSM's October 27, 2022, proposal to Workspace Collective, in care of Mr. Adam Ramsay. The agreement was signed by Mr. Ramsay on November 4, 2022 and was returned to KSM thereafter.

Summary of Findings and Conclusions:

The following is a summary of the principal findings and conclusions that are contained in this report, based on the results of KSM's subsurface exploration and analytical laboratory testing:

- Within the depth of exploration, the property was underlain by generally weak (very loose to loose) near surface layer of granular material with an organic content varying from less than 5%, which is the typically accepted limit before a soil is considered problematic for construction, to 10.9%. When inherently problematic soils were encountered they were typically at the surface with an estimated thickness of 1 foot however several borings encountered problematic soil deposits to depths ranging from 1 to 6 feet below grade. Below the organic layer deposits of loose to medium dense cohesionless fine sand underlain by loose to medium dense clayey/silty fine sand were discovered.
- The recorded depth to the surface of the groundwater body was very shallow and KSM



- The discovered subsurface conditions are expected to negatively impact the development of this property in that creation of a stable subgrade for support of the proposed dwellings and roadways will be difficult due to the combined effect of the expected shallow position of the groundwater surface, the excessive organic content and weak nature of the upper zones of soil. Specifically, we anticipate that the excavation and backfill of near surface organic deposits and the compaction of very loose subgrade soils may be difficult due to the shallow water table position. Consideration should also be given to scheduling the earthwork operations to be performed during the seasonally dry winter and spring months to decrease the amount of anticipated dewatering that will be required.
- The installation of buried utility lines (i.e., stormwater piping, water supply lines, power lines, and telecommunication lines) are likely to involve excavation of trenches below the groundwater surface. Accordingly, dewatering of the trench excavations is likely to be necessary to enable the installation to be performed in dry conditions.
- Given the existing weak nature of the subgrade soils, the foundations that support the new structures on this property should bear at elevations that are as shallow as practical, in order to contain the stress transmitted by the foundations within the compacted engineered fill soils that will be placed during the mass grading operations. Monolithic slab foundation systems are generally better suited to this situation than are conventional shallow spread footings and a separate slab-on-grade systems.

The nature and extent of earthwork methods that would best suit this property will depend to a great extent on the details of the proposed grading and drainage plan. The development of the land into a retail development would benefit from the raising the land surface above the existing landform, by installation of structural fill, in order to enable the streets and structures to be supported above the seasonal groundwater levels, upon engineered fill materials. This report offers preliminary recommendations that assume that the landform will be raised a height of not less than 5 feet above the existing land surface.

Site Description:

<u>Location & Physiography</u> – The project site was located in Okeechobee, Florida, on State Route (SR) 70. At the time of the investigation, the site was found to have a generally flat topography. Vegetation on the site consisted mostly of light ground surface cover vegetation and many trees.

Project Description:

The following information is based, in part, on our review of the Conceptual Sizing Plans for "Park Prime Retail (22.10.10)" and "Plan Park Street Commerce Center (12.14.22)" by American Civil Engineering Co. Due to the preliminary nature of this project, it is our understanding that the development site plan has not been finalized. Please contact KSM to provide the most recent plans, so we can make any adjustments and review this report accordingly.

Overall Development – It is our understanding that the proposed site may be developed with



for the purpose of stormwater management on the site. Typical pavement areas will consist of driveways and parking areas.

The Scope of KSM's Study Included:

- 1. Performed soil borings within the approximate limits of the proposed structures and pavement areas, as well as in the proposed pond locations.
- 2. Measured the encountered groundwater level at each boring.
- 3. Reviewed the soil samples and field soil boring logs (by a geotechnical engineer) in our laboratory and assigned analytical laboratory testing to selected samples.
- 4. Performed the assigned analytical laboratory tests on the selected soil samples.
- 5. Evaluated the discovered subsurface conditions with respect to the construction of the proposed structures and roadways.
- 6. Prepared this preliminary report to document the data that was gathered, to present our findings and to present our preliminary recommendations.

Site Investigation:

<u>Limitations</u> – The preliminary opinions and recommendations are based on the discovered subsurface conditions in the locations of the performed tests.

<u>Subsurface Testing</u> – KSM's site investigation program consisted of performing the following tests:

- Twenty-nine (29) Standard Penetration Test (SPT) Borings, terminated at approximate depths ranging from 10 to 15 feet below the existing ground surface, were performed within the limits of the site.
- Fifteen (15) Hand Auger (HA) Borings with corresponding Static Cone Penetrometer (SCP) Soundings, terminated at an approximate depth of 6 feet below the existing ground surface, were performed within the limits of the proposed pavement areas.
- Two (2) SPT borings, denoted as PB, terminated at an approximate depth of 20 feet below the existing ground surface, were performed within the limits of the proposed stormwater management areas.

<u>SPT Borings</u> – The SPT borings were performed in general accordance with procedures described in ASTM D1586.

<u>HA Borings</u> – The HA borings were performed using a bucket auger tool to advance the borehole and to return disturbed samples of the soils. The drilling was performed in general accordance with the procedures delineated in ASTM D1452.

SCP Soundings – Execution of a SCP sounding consists of pushing a thin steel shaft, with an



attached proving ring with a calibrated gauge. The value of the bearing pressure exerted by the cone point has been correlated with the relative soil density. The relationship of the SCP reading to the relative density is listed in the table below:

Static Cor	Static Cone Penetrometer				
Relative Density	Static Penetrometer Reading				
Very Loose or Soft	<15				
Loose	15-40				
Medium Dense	40-70				
Dense	>70				

<u>Soil Classification</u> – The field soil boring logs and recovered soil samples were transported to KSM's office from the project site. Following the completion of the field exploration activities, visual and tactile examination of the soil samples was performed by a geotechnical engineer to identify the engineering classification of the soil samples that were obtained in the field exploration. The visual classification of the samples was performed in general accordance with the current United Soil Classification System (ASTM D2487).

<u>General Subsurface Soil Classification Summary</u> – The following table outlines the general subsurface conditions encountered during our investigation. Refer to the boring logs and location map for specific information regarding our interpretation of the field boring logs.

Generalized Soil Profile				
Approximate Depth Below Grade (Feet)	Discovered Subsurface Conditions			
	Variable near surface soil conditions include:			
0 to 5	 Very loose sand with organic material (problematic soil deposits) generally within 1 foot of the surface however some borings revealed problematic soil deposits to depths of 5 feet below grade; 			
	Very loose to medium dense fine sand			
6 to 15	Loose to medium-dense fine sand, slightly clayey/slightly silty sand, and clayey sand			

<u>Observed Groundwater Table</u> – Following the completion of each soil boring, the groundwater was allowed to attain an equilibrium level and the approximate depth to the surface of the groundwater was measured from existing ground elevation and recorded in the field log. The typical observed water table was encountered at approximate depths ranging from at or above the existing grade to depths of 5.3 feet below existing grade. These values were recorded at the time of investigation, which took place between the dates of November 21, 2022, and December 15, 2022.



Analytical Laboratory Testing:

<u>Natural Moisture Content</u> – Testing was performed in general accordance with procedures described in ASTM D2216-19.

<u>Fines Content</u> – Testing was performed in general accordance with procedures described in ASTM D1140-17.

<u>Organic Content Tests</u> – Testing was performed in general accordance with procedures described in ASTM D2974-20e1.

	Sample	Analytical Laboratory Tes			Organia
Boring	Sample Depth (ft)	Soil Description	Moisture %	Fines %	Organic Content %
B-1	2	Gray Clayey Sand	19%	24%	
B-3	13	Brown Clayey Sand	22%	29%	
B-9	6	Dark Gray Slightly Clayey Sand	26%	5.2%	
B-10	2	Dark Brown Sand with Organics	34%		
B-11	6	Light Gray Sand	24%	1.2%	
B-19	6	Gray Slightly Silty Sand	35%	5.5%	
B-19	13	Gray Clayey Sand	25%	23%	
B-22	2	Gray Sand	17%	3.1%	
B-24	2	Dark Gray Sand	34%	3.7%	
B-24	13	Brown Clayey Sand	2%	33%	
B-25	4	Brown Sand	31%	4.9%	1
B-26	1	Dark Gray Sand with Traces of Roots	27%		
B-29	2	Brown Sand	22%	2.5%	
B-29	13	Brown Slightly Clayey Sand	29%	11%	
HA-2	1	Dark Gray Silty Sand with Organics	47%	6	7.9%
HA-9	1	Dark Gray Silty Sand with Organics	45%	12%	8.0%
HA-12	1	Dark Gray Sand, Slightly Silty with Organics	49%		10.9%



and the second	Analytical Laboratory Testing Results (Continued)					
Boring Sample (ft)		th Soil Description	Moisture %	Fines %	Organic Content %	
PB-1	3	Light Gray Slightly Silty Sand	27%	5.8%		
PB-1	5	Grayish Brown Sand	24%	3.7%		
PB-1	10	Grayish Brown Sand	25%	2.2%		
PB-1	12	Gray Slightly Clayey Sand	23%	9.8%		
PB-2	3	Light Gray Sand	25%	2.0%		
PB-2	5	Dark Gray Sand, Slightly Silty	24%	11%		
PB-2	7	Light Gray Sand	25%	0.6%		
PB-2	12	Gray Clayey Sand	22%	33%		
PB-2	16	Light Brown Clayey Sand	27%	19%		
PB-2	18	Light Gray Clayey Sand with Shell	13%	12%		

Engineering Evaluation:

Based on the information obtained from this site investigation, we are pleased to offer the following evaluation:

<u>Limitations</u> – Due to the preliminary nature of this project, KSM recommends that additional testing is performed within the development features once the final locations are known. The following information is preliminary and based on the initial conceptual site layout and may not correspond to the final design site layout.

<u>Seasonal Groundwater Fluctuation</u> – The following table indicates the recorded measurement taken from the existing grade to the encountered groundwater table for each test location along with our estimated depth normal wet season water table and normal dry season water table depths (below existing grade) for the test locations. The measurements were taken after the borings were performed and the groundwater table was allowed to stabilize.

Estir	nated Normal Season	Groundwater Table Flu	ctuation
Test Location (See Location Plan)	Depth (feet,') Below Existing Grade		
	Measured Encountered Groundwater Table	Estimated Normal Wet Season Water Table	Estimated Normal Dry Season Water Table
PB-1	1.0'	0.3'	3.3'
PB-2	2.4'	1.0'	4.0'



featured deeper groundwater surface depths were likely to have been drilled at locations where the land surface altitude was above boring locations that featured shallower groundwater surface depth measurements, or that the range and depth of the water table elevation may be affected by the proximity of man-made or natural drainage features. Accordingly, as part of the design phase geotechnical studies, KSM recommends that the land surface elevation of the borings be surveyed to determine the approximate altitude of the groundwater surface, at the time that the measurements were made. Using that data, KSM can provide a more precise estimate of seasonal groundwater levels.

<u>Dewatering</u> – Given the normal wet season groundwater surface level estimates, and assuming that KSM's estimates of the height of the fill stated in the "Project Description" section is accurate, it is KSM's preliminary opinion that the position of the groundwater table is unlikely to affect either the design or the installation of the shallow foundations that will support the dwellings constructed on this property. Conversely, KSM anticipates that the earthwork stage of site development will require the compaction and/or excavation of soils located below the groundwater surface. Additionally, excavations that are made to install buried utility lines could also require excavations below the groundwater surface. Accordingly, the earthwork contractor should recognize that temporary dewatering of excavations that penetrate below the prevailing groundwater surface will be necessary to allow the earthwork operations to be performed in dry conditions and plan his operations accordingly.

<u>Analysis and Opinions: Fill Suitability</u> – Based on the discovered soils in the locations of PB-1 and PB-2, KSM offers the following recommendations on the suitability of fill deposits that will be used to rough grade the property in preparation for the installation of the roadway and utility networks and for the construction of the individual dwellings.

- Fine sands deposits, which feature less than 5 percent "fines", are considered to be best suited as a structural fill source because they drain freely when excavated below the water table and are not as moisture sensitive as material that contains higher fines.
- The slightly clayey/silty fine sands containing between 5 and 12 percent fines, are suitable as structural fill, but may require extra effort to be properly moisture conditioned and compacted. These soil deposits drain fairly well but will require more effort than the fine sand deposits above to create optimum moisture conditions in order to avoid compaction issues. Thinner lifts not exceeding 6 inches in loose thickness may be required for placement and compaction of these soils.
- Clayey and/or silty fine sand deposits, (i.e., sand deposits that contain more than 12 percent fines, by weight) are typically not considered desirable for structural fill, due to their poor workability characteristics in comparison with sand deposits that feature fewer fines. Due to the moisture sensitive nature of these materials a substantial amount of time and effort would be required in order to improve their workability. The discovered clayey sand deposits may be more suitable for use as fill material in non-structural areas outside the building pad and within the pavement area footprint, and potentially as a stabilized subgrade component in the roadway pavement cross



necessary to moisture condition and densify such soil deposits. Such efforts could include draining/drying of saturated soils before attempting compaction, reduction in the thickness of lifts that are compacted, and the use of non-vibratory compaction machinery.

• Soil deposits that featured organic contents greater than 5 percent should not be considered suitable soils for structural fill.

<u>Borrow Source Suitability Opinions</u> – KSM assumes that the soil that is excavated to create the stormwater management basin will be used as a source of fill in creating the landform of the developed property. Based on the results of the field investigation, together with the analytical laboratory testing of the selected soil samples, KSM offers the following opinions:

- From the surface to an approximate depth of 10 feet below existing grade Deposits of fine sand and slightly clayey/silty sand were discovered. These granular, low fines content deposits can be considered suitable for structural fill.
- From a depth of approximately 10 to 15 feet below existing grade Deposits of clayey/silty sand were discovered. We anticipate that most of the excavated soils within this depth interval will feature fines contents that exceed 12 percent. These soil deposits are expected to be moisture sensitive soils, given their elevated fines content and the estimated shallow position of the surface of the wet season groundwater table. To avoid compaction-related issues during construction, it is KSM's opinion that excavated soils with elevated fines content should not be considered suitable as structural fill for building pads or in pavement areas. In no case should these materials be used in areas that are expected to contain septic drain fields, due to their expected low internal permeability.
- From a depth of approximately 15 to 20 feet below existing grade Deposits of fine sand with shell and slightly silty/clayey sand were encountered. These granular, low fines content deposits can be considered suitable for structural fill.

The contractor and civil engineer should coordinate to determine the appropriate methods for borrow source excavation. It is important to segregate the low-fines soil deposits from the near-surface clayey and silty sand deposits.

<u>Preliminary Utility and Storm Sewer Opinions</u> – Due to the expected very shallow seasonal depth to the surface of the groundwater, we believe that difficulties may arise when installing any utility that will require trenching and/or that will rely on gravity flow. Trench excavations that encounter very loose subsurface materials may require over-excavation, typically to a depth of 1 foot below the utility subgrade elevation, backfilled with ³/₄-inch stone compacted to produce a firm, unyielding surface. Any excavated materials with elevated fines content will likely prove to be problematic if intended to be used as backfill. Blending of the excavated material with dry, clean fine sand may be necessary, but due to the time and effort required to properly blend these materials, for ease of construction and scheduling considerations, it may



<u>Analysis and Opinions : Preliminary Subgrade Opinions</u> – Based on our experience in the area, the results of the borings, and KSM's understanding of the project, we believe that the current conditions of the near surface soils are problematic for development due to the high degree of variation between borings, elevated organic content, to the elevated fines content, to the very loose to loose condition of the near-surface soil deposits, and to the anticipated shallow depth to the groundwater table. Excavation of problematic soils and backfill of the exposed areas to create a stable platform for the expected fill deposits will require that temporary dewatering systems be installed to depress the groundwater level. Please note that estimating the vertical and horizontal limits of any problematic material was as part of our scope for this investigation. KSM recommends that a design level geotechnical investigation is performed on this site to aid in the development of design plans.

<u>Preliminary Minimum Roadway Opinions</u> – It is our preliminary opinion that the discovered subsurface conditions are generally problematic to support a roadway subgrade. It appears that prior to the installation of the proposed roadway section, improvement of the existing subgrade can be achieved using a proper excavation and backfilling techniques. Additionally, the cost of dewatering should be considered. Provided that the subgrade is properly prepared and that the building pads are properly installed, it is KSM's opinion that the improved subgrade can support the proposed roadway section.

The pavement should be designed for the anticipated axle weights, vehicle velocities, traffic mix and frequencies. Please refer to the following table for the minimum recommended pavement section.

Minimum Pavement Section				
Pavement Type	Ant war	Layer Thickness (in)		
	Material	Standard Duty	Heavy Duty	
Flexible	Florida DOT Asphalt Type 3	1.5	2.5	
	Base Course* (Min. LBR of 100) Cemented Coquina Rock	6	10	
	Stabilized Subgrade* (Min. LBR of 40)	12	12	

A minimum of 16 inches of separation should be maintained between the bottom of the base and the high-season water table.

* Compacted to minimum 98 percent of its modified dry Proctor value (AASHTO T180)

<u>Preliminary Foundation Opinions</u> – In order for a shallow foundation to perform satisfactorily, it must be able to support the structural loads while limiting both total and differential settlement to tolerable values. It is our preliminary opinion that the discovered subsurface conditions are generally problematic to support a building pad subgrade. It appears that prior to the installation of the proposed buildings pads, improvement of the existing subgrade can be achieved using a proper excavation and backfilling techniques. Additionally, the cost of dewatering should be considered. Provided that the subgrade is properly prepared and that the building pade are prepared winstalled it is KSM's existent that appears that prior



For more precise building area site and roadway preparation recommendations, as well as recommendations pertaining to foundation design and settlement calculations, we recommend performing a design level investigation. KSM should be provided with the civil construction drawing set as well as the structural plans for review. Please see the section titled "Future Studies" for additional information.

Estimated Aquifer Parameters:

<u>Limitations</u> – Due to the preliminary nature of this project, KSM recommends that additional testing is performed within the proposed stormwater management features once the locations and elevations are better defined. The following information is preliminary and based on the initial stormwater management layout and may not correspond to the proposed stormwater management layout.

<u>Factor of Safety</u> – KSM has not applied a factor of safety to the estimated aquifer parameters within this report. The Engineer of Record is responsible for applying the appropriate factor(s) of safety to the estimated aquifer parameters contained within this report for use in their design. For any stratum where the estimated flow rate exceeds 10 inches per hour (20 feet per day), we recommend that a design flow rate equal to 10 in/hr (20 ft/day) is used.

<u>In-Field Testing</u> – At the test location, Usual Condition Test was performed in general conformance with the South Florida Water Management District described procedures for the 'Usual Open-Hole Test' method.

Estimated Aquifer Parameters – In-Field Testing		
Test Location	Estimated Hydraulic Conductivity (CFS/SF- Ft Head)	
P-1	2.5 x 10 ⁻⁵	
P-2	7.6 x 10 ⁻⁵	

<u>Laboratory Testing and Professional Judgement</u> – Selected samples obtained from our site investigation were tested in our laboratory in general accordance with ASTM D2434, ASTM D1140-17 and ASTM C136.

Estimated Aquifer Parameters – Laboratory Testing				
Test Location	Stratum Depth Range (ft)	Horizontal Saturated Flow Rate (in/hr)	Vertical Saturated Flow Rate (in/hr)	
P-1	0.8 – 1.8	1.9	0.9	
	1.8 – 4.2	3.2	2.2 †	
P-2	0.0 - 4.5	7.0	5.8	

[†]Estimation: reduction of estimated horizontal saturated flow rate applied.



<u>Restrictive Stratum</u> – Based on the results of our soil borings and the laboratory testing, in boring PB-1 we encountered a stratum which we estimate exhibit restrictive flow rates relative to the overlying stratum, and are described below:

• Deposits of Dark Gray Silty Sand with Organics encountered at an approximate depth range from the surface to 0.8 feet below grade.

Future Studies:

<u>Design Phase Geotechnical Explorations</u> – KSM recommends that a design-phase geotechnical exploration be performed to determine whether individual structures are underlain by any organic soil deposits or inherently problematic soils and to generate the subsurface data that is necessary to provide site specific foundation design and earthwork recommendations. Upon request, KSM will provide a detailed scope of work and cost proposal to address these features, based on the preliminary plan documents.

Closure:

Based upon KSM's subsurface investigation at the above-mentioned project location, the reliance of the preliminary opinions and recommendations presented within this signed and sealed report is predicated on KSM being engaged to perform design-basis geotechnical explorations and testing. The opinions and recommendations given in this report are preliminary and should not be used to create final plan documents and specifications.

This report has been prepared in accordance with generally accepted soil and foundation engineering practices based on the results of the borings and the assumed loading conditions. No warranties, either expressed or implied, are intended or made. This report does not reflect any variations which may occur between the borings. If variations appear evident during construction, it will be necessary for you or your representative to engage KSM to perform any supplementary studies and to re-evaluate the recommendations made in this report.

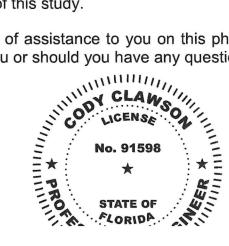
Environmental conditions, wetland delineation, karst activity, water quality, and municipal requirements were not a part of this study.

KSM is pleased to have been of assistance to you on this phase of your project. When we may be of further service to you or should you have any questions, please contact us.

Respectfully,

Christopher LeBrun

Christopher S. LeBrun, E.I. Geotechnical Engineer



Cody C. Clawson, P.E. Geotechnical Engineer



207 NORTH MOSS ROAD, SUITE 211 • WINTER SPRINGS, FLORIDA 32708 Telephone: (407) 327-7700 • www.americancivilengineering.com Advancing Civilization Since 1990

Stormwater Letter

To: City of Okeechobee General Services Department 55 S.E. 3rd Avenue, Room 101 Okeechobee, FL 34974

Regarding: Stormwater Carwash – Portion of Park Street Commerce Center

Storm water for the carwash is met as part of the co-submitted master storm report for the infrastructure of Park street Commerce Center. The master report allows for up to 1.00 AC, the site proposed at 0.94 AC meets this criteria and is fully accounted for in the master permit.

	Impervious Area Accounted
Lot 1	1.00 AC
Lot 2	1.50 AC
Lot 3	1.75 AC
Lot 4	1.75 AC
Roadway	0.80 AC
Total	6.80 AC

Please contact me directly on my cell phone 407-376-1777 regarding any questions.

Thank you,

Johnny Herbert IV PE Partner

PARK STREET COMMERCE CENTER TRAFFIC STUDY

OKEECHOBEE COUNTY, FLORIDA

July 2023



BUCKHOLZ TRAFFIC



BUCKHOLZ TRAFFIC 3585 KORI ROAD JACKSONVILLE, FLORIDA 32257 (904) 886-2171 jwbuckholz@aol.com

July 11, 2023

Mr. Johnny Herbert IV, P.E. American Civil Engineering Company 207 N. Moss Road / #211 Winter Springs, Florida 32708

Re: Park Street Commerce Center, Revised Traffic Study

Dear Mr. Herbert:

Attached is the revised traffic study. If there are any questions or comments regarding this study, please contact me.

Sincerely,

Digitally signed by Jeffrey W. Buckholz DN: cn=Jeffrey W. Buckholz, o=BUCKHOLZ TRAFFIC ENGINEERING, ou, email=jwbuckholz@aol.c om, c=US Date: 2023.07.11 12:45:31 -04'00'

Jeffrey W. Buckholz, P.E., PTOE Principal

This item has been digitally signed and sealed by Jeffrey W. Buckholz, P.E. on the date indicated on the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

PARK STREET COMMERCE CENTER TRAFFIC STUDY

INTRODUCTION

This proposed mixed-use development will include a 4600 sf automated car wash, a 5000 sf fast food restaurant with drive-thru window, a 100 room hotel, 52 apartment units, and 10,000 sf of retail space. The fast food restaurant will open at 10 AM and the car wash will open at 9:00 AM. The development will be located in the northwest quadrant of the NE Park Street/SE 13th Avenue intersection in Okeechobee, Florida. A cul-de-sac road that extends 13th Avenue to the north will be installed on site property and access will be provided to the various land uses from this road. NE Park Street (SR 70) is a four lane divided urban major arterial with an FDOT access management classification of 7 and a posted speed limit of 35 mph. SE 13th Avenue is a two lane undivided local road with a posted speed limit of 25 mph.

Figure 1 shows the site location and surrounding road network while Appendix A contains the proposed site plan. The development is expected to be complete and fully occupied by the end of 2024. Consequently, 2024 was chosen as the design year for this study.

EXISTING TRAFFIC VOLUMES

Weekday peak period manual turning movement counts were conducted by Buckholz Traffic personnel at the intersection of NE Park Street with SE 13th Avenue and with the Hampton Inn driveway located west of SE 13th Avenue. These counts, which are provided in Appendix B, were conducted during the weekday AM peak period (6:45-8:45 AM) and the weekday PM peak period (3:45 – 6:00 PM) with school in session. The data was recorded at 15-minute intervals and includes a separate tabulation for trucks and pedestrians. Figure 2 graphically summarizes the AM and PM peak hour traffic counts.

Appendix C provides daily traffic volumes from two nearby FDOT traffic count stations on SR 70. The existing average daily traffic on NE Park Street in the vicinity of the site is about 29,000 vehicles per day.

TRIP GENERATION

Trip generation calculations were carried out using the 11th edition of ITE's <u>Trip Generation Manual</u> by referencing land use codes 948 (Automated Car Wash), 934 (Fast Food Restaurant with Drive-Thru Window), 312 (Business Hotel), 220 (Low Rise Multifamily Housing) and 822 (Strip Retail Plaza). Tables 1 through 5 contain the daily, AM peak hour, and PM peak hour trip generation calculations. During an average weekday the development is expected to generate 4424 trips (2212 entering and 2212 exiting) with 87 trips (36 entering and 51 exiting) occurring during the AM peak hour and 369 trips (193 entering and 176 exiting) occurring during the PM peak hour.

SITE TRIP DISTRIBUTION AND TRAFFIC ASSIGNMENT

Weekday AM and PM peak hour site trips for this commercial development were directionally distributed based on engineering judgment after reviewing the trip distribution percentages used in the 2020 traffic study for the nearby RaceTrac commercial development. The results are provided in Figures 3 and 4.

BUCKHOLZ TRAFFIC

PARK STREET COMMERCE CENTER TRAFFIC STUDY

Page 2

FUTURE TRAFFIC VOLUMES

The expected weekday 2024 peak hour background (No Build) traffic volumes and total (Build) traffic volumes at intersections of interest are graphically depicted in Figures 5 through 8. The No Build traffic volumes were obtained by multiplying the existing traffic volumes by the appropriate FDOT seasonal adjustment factor (0.96) and then by an annual growth rate. A linear regression analysis of FDOT daily traffic counts in the area (see graphs C-1 and C-2 in Appendix C) indicates that daily traffic volumes have been increasing at an average annual rate of 1.5% over recent years. The 2024 Build traffic volumes were obtained by adding the traffic generated by the new development to the 2024 No Build traffic volumes.

TURN LANE EVALUATION

A formal analysis was made to determine if a right turn lane is warranted on westbound NE Park Street at the two new roadways: NE 13th Avenue and NE 12th Avenue. The methodology contained in NCHRP Report 279 was used to conduct this analysis. As is indicated in Figures 9 and 10, right turn volumes under expected 2024 Build conditions will be high enough to warrant an exclusive right turn lane at NE 13th Avenue but will not be high enough to warrant an exclusive right turn lane at NE 13th Avenue. However, NCHRP Report 420 - which requires 110 right turns per hour to warrant a right turn lane on a multi-lane roadway with a posted speed of 45 mph or less – does not support the installation of an exclusive right turn lane at either location.

UNSIGNALIZED INTERSECTION CAPACITY ANALYSIS

The NE Park Street/13th Avenue intersection and the NE Park Street/Hampton Inn Driveway/NE 12th Avenue intersection were analyzed using the two-way stop control methodology contained in the year 2023 version of the Highway Capacity Software. The supporting calculations are provided in Appendix D. Table 6 summarizes the capacity analysis results under existing conditions while Table 7 summarizes the capacity analysis results under 2024 Build conditions.

All minor movements currently operate at level of service C or better during both weekday peak hours at the NE Park Street/SE 13th Avenue intersection with minimal queuing and a volume-to-capacity ratio well below one. Under 2024 Build conditions at the new NE Park Street/13th Avenue intersection with dual directional median opening all minor movements are expected to continue to operate at level of service C or better during both peak hours with minimal queuing and a volume-to-capacity ratio still well below one.

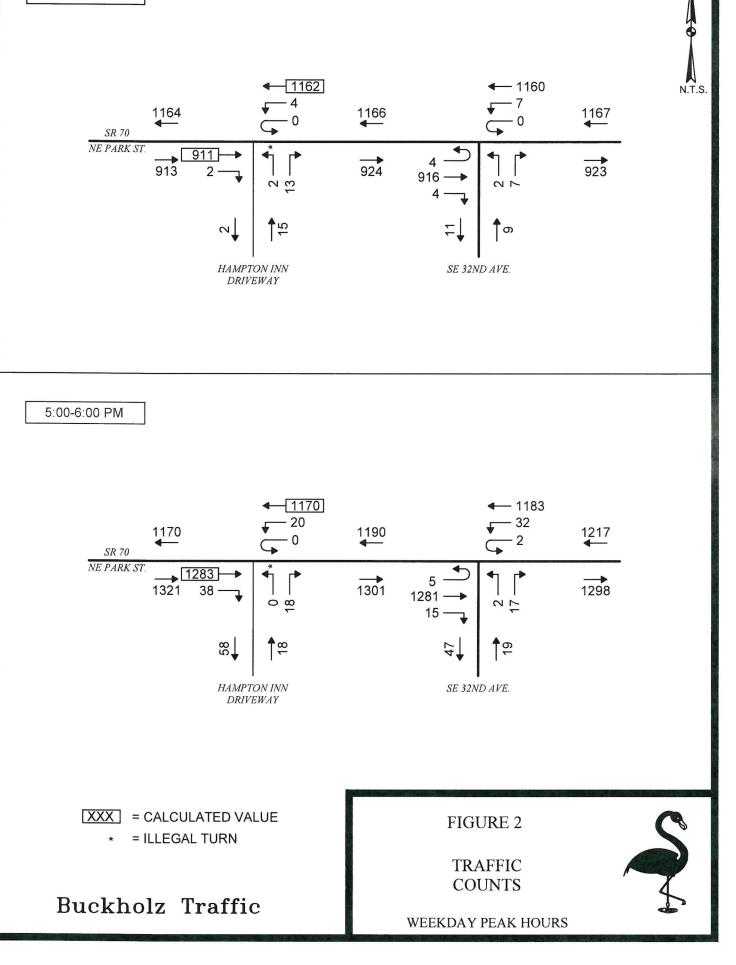
At the NE Park Street/Hampton Inn Driveway intersection all minor movements currently operate at level of service B or better during both weekday peak hours with minimal queuing and a volume-to-capacity ratio well below one.

Under 2024 Build conditions all minor movements at the NE Park Street/Hampton Inn Driveway/NE 12th Avenue intersection are expected to operate at level of service C or better during both weekday peak hours – with one exception. The westbound left turn is expected to operate at level of service F during the PM peak hour. However, only moderate queuing and a volume-to-capacity ratio below one are expected for this movement movements.

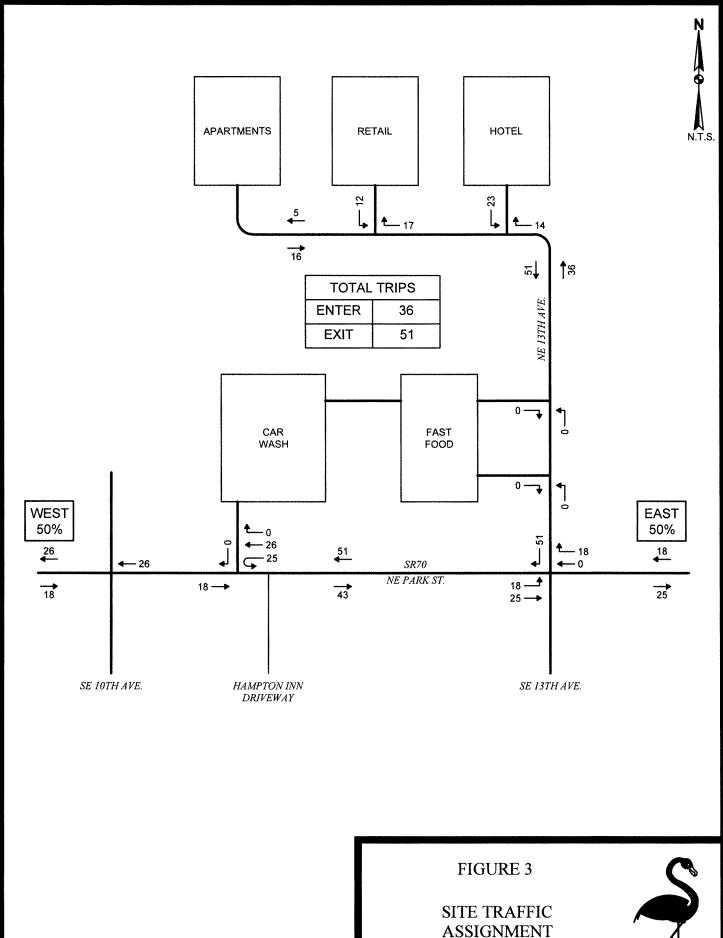
BUCKHOLZ TRAFFIC



7:15-8:15 AM



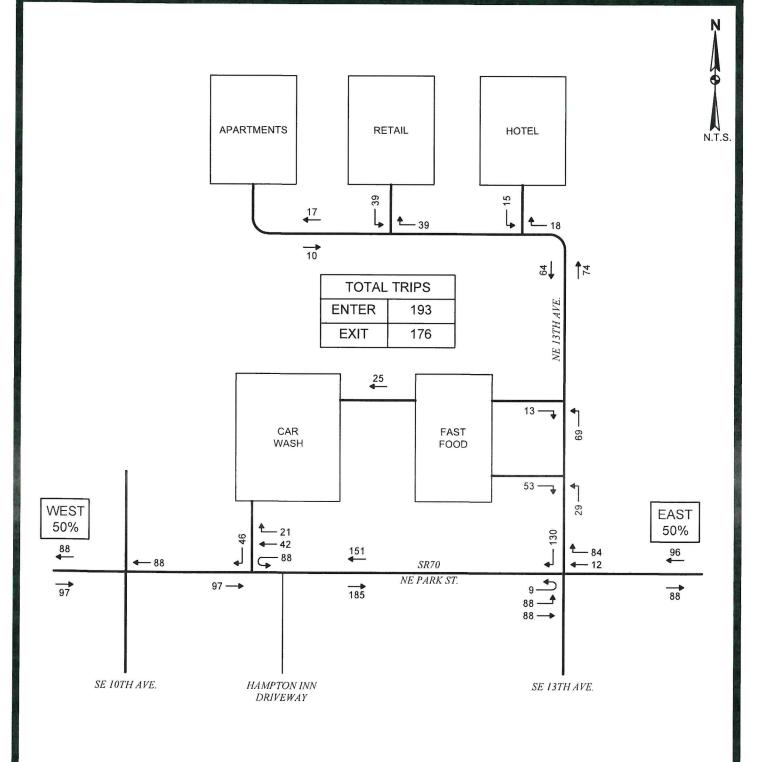
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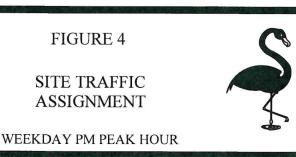


E: \23-1820\CAD2\FIG_03.DWG

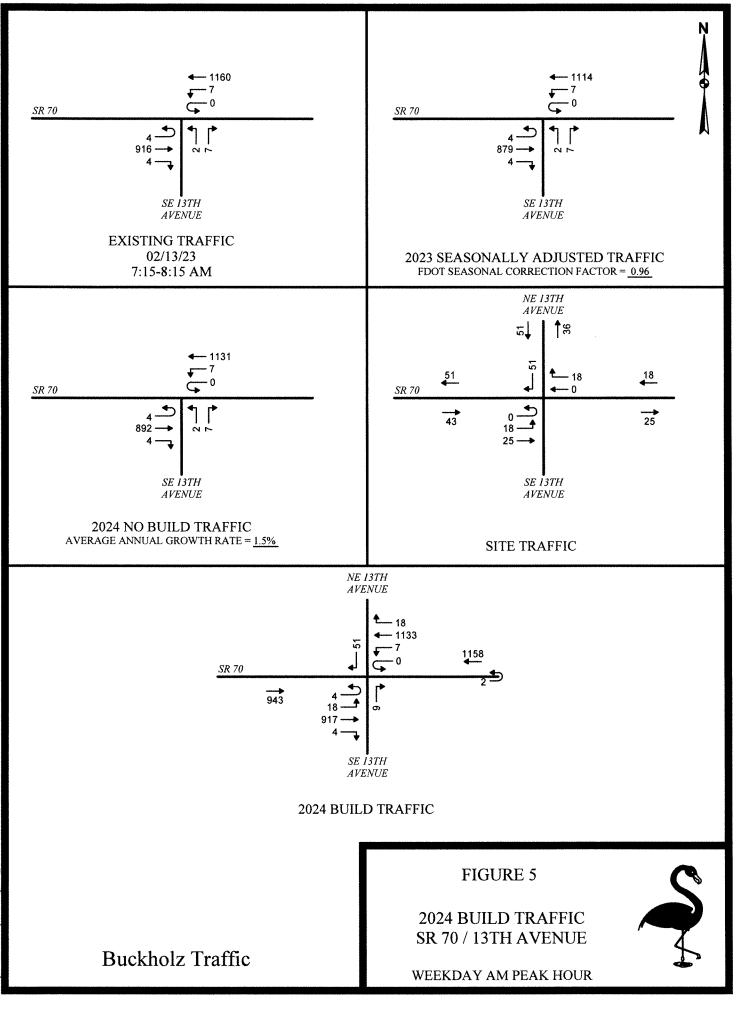
Buckholz Traffic

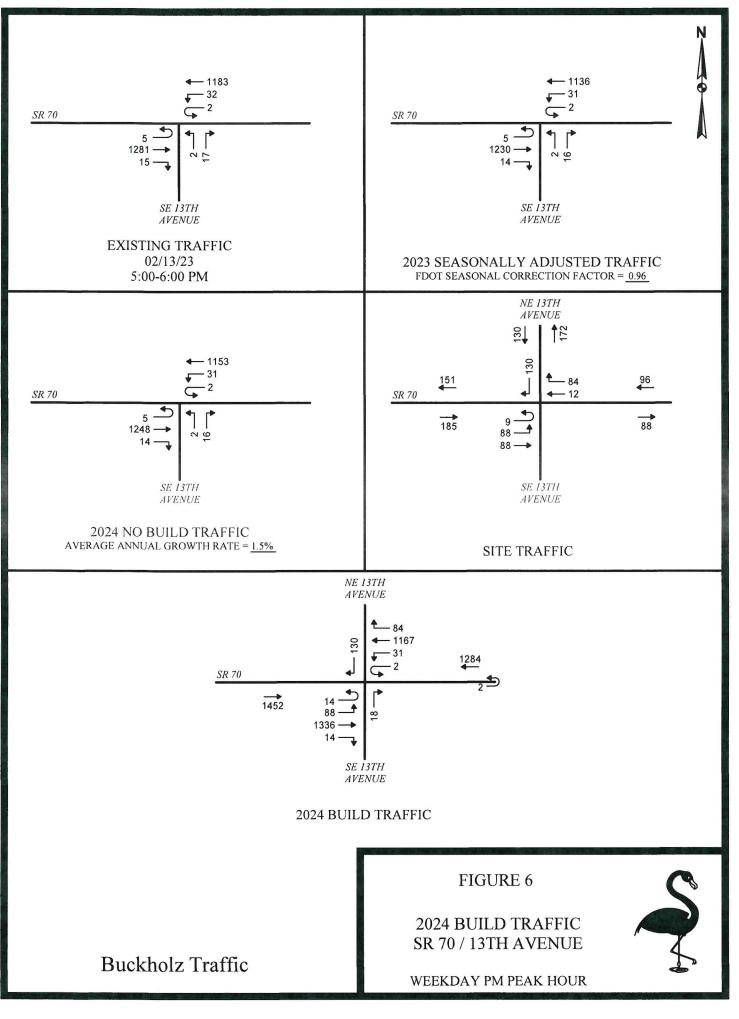
WEEKDAY AM PEAK HOUR



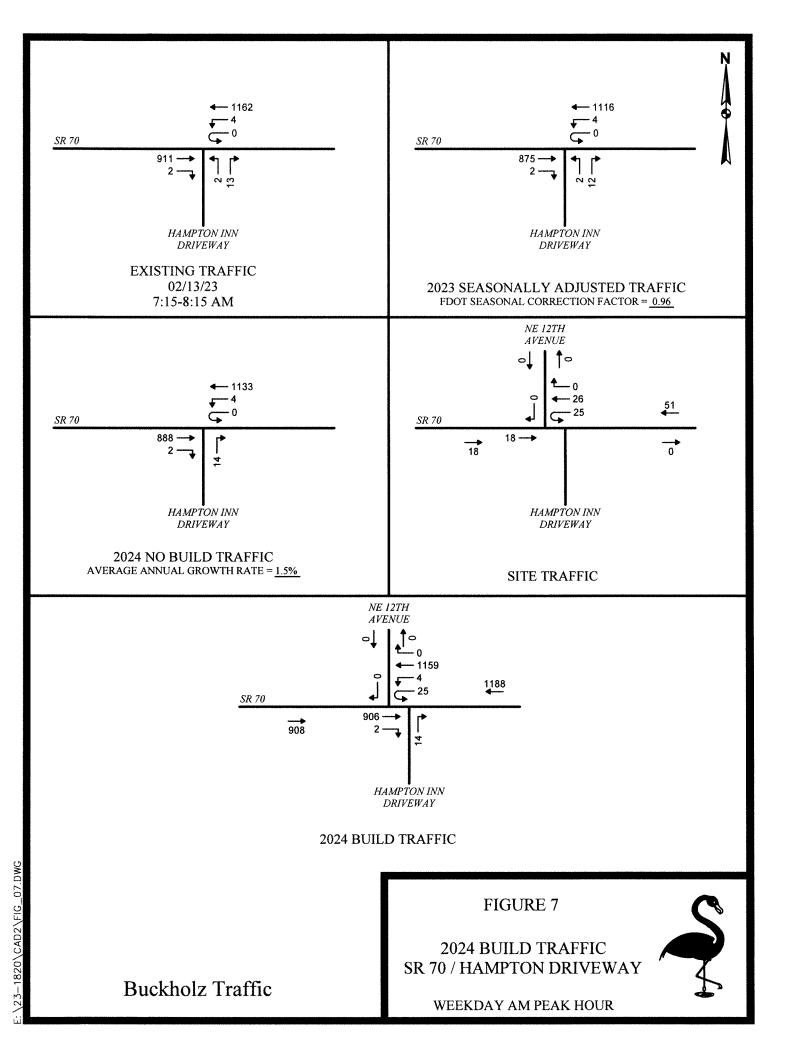


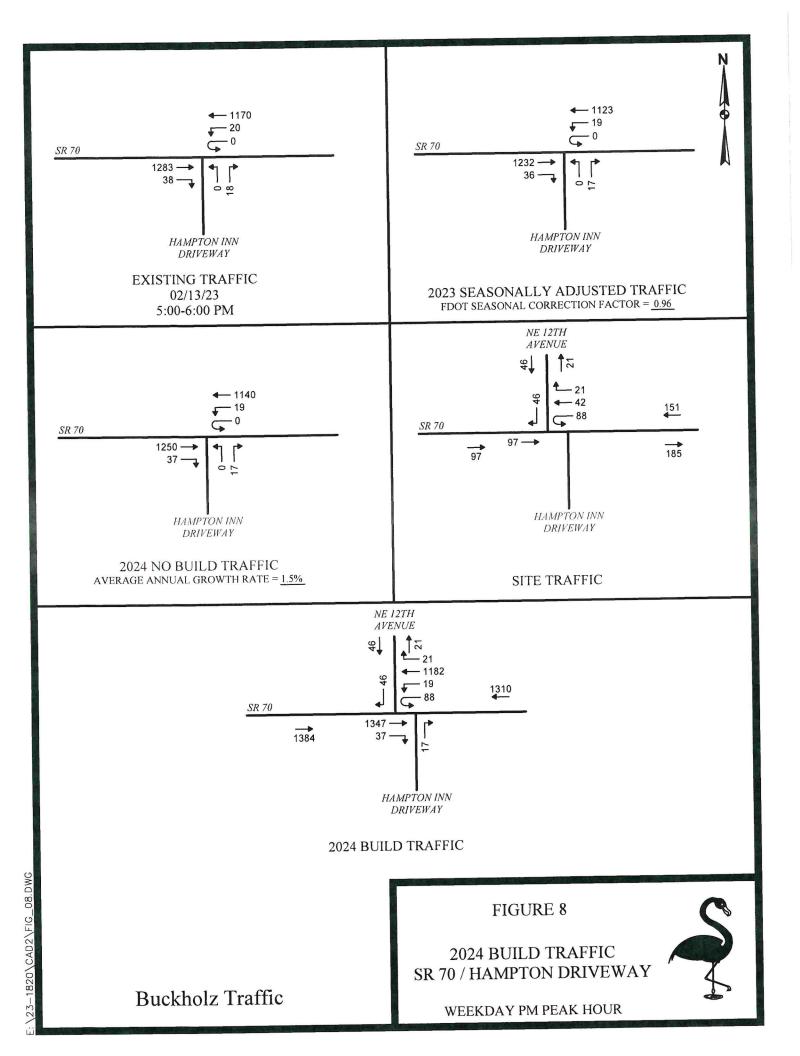
Buckholz Traffic



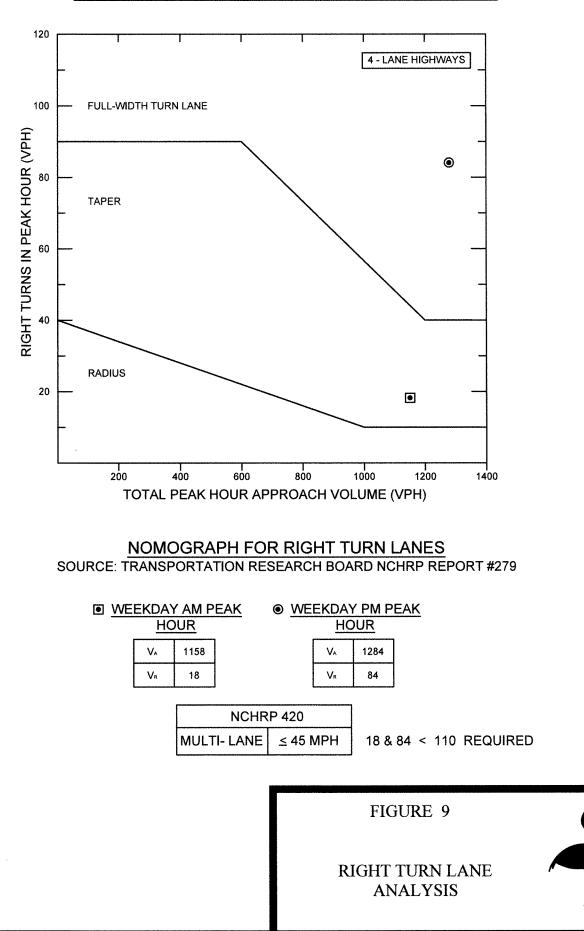


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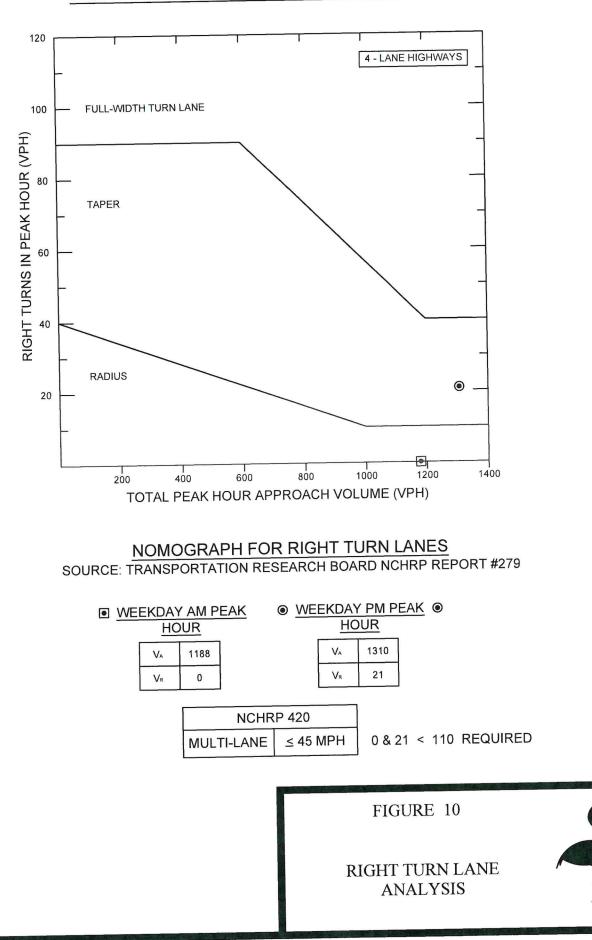




WESTBOUND NE PARK STREET @ NE 13TH AVENUE



WESTBOUND NE PARK STREET @ NE 12TH AVENUE



TRIP GENERATION CALCULATIONS

AUTOMATED CAR WASH

Land Use Code 948

T = Number of Vehicle Trip Ends

X = 4600 GSF = 4.6

TIME PERIOD	TOTAL TRIP GENERATION <u>EQUATION</u>	TOTAL TRIP <u>ENDS</u>	PERCENT <u>ENTERING</u>	PERCENT <u>EXITING</u>	TOTAL TRIP ENDS <u>ENTERING</u>	TOTAL TRIP ENDS <u>EXITING</u>
AVERAGE WEEKDAY						
Daily	T = 14.2/8.7% (X)	750	50%	50%	375	375
AM Peak Hour	NOT OPEN					
PM Peak Hour	T = 14.20 (X)	66	50%	50%	33	33

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

Estimated Using ITE Hourly Percentages from LUC 949

TRIP GENERATION CALCULATIONS

FAST-FOOD RESTAURANT WITH DRIVE-THRU WINDOW

Land Use Code 934

T = Number of Vehicle Trip Ends

Size of Building = 5000 (X = 5.0)

TIME PERIOD	TOTAL TRIP GENERATION <u>EQUATION</u>	TOTAL TRIP <u>ENDS</u>	PERCENT <u>ENTERING</u>	PERCENT <u>EXITING</u>	TOTAL TRIP ENDS <u>ENTERING</u>	TOTAL TRIP ENDS <u>EXITING</u>
AVERAGE WEEKDAY						
Daily	T = 467.48 (X)	2338	50%	50%	1169	1169
AM Peak Hour			NOT OPEN			
PM Peak Hour	T = 33.03 (X)	165	52%	48%	86	79

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

NEW TRIPS		NEW			NEW	NEW
TIME PERIOD	PERCENT NEW TRIPS	TRIP <u>ENDS</u>	PERCENT <u>ENTERING</u>	PERCENT EXITING	TRIP ENDS <u>ENTERING</u>	TRIP ENDS <u>EXITING</u>
AVERAGE WEEKDAY						
Daily	52%	1216	50%	50%	608	608
AM Peak Hour	50%		NOT OPEN			
PM Peak Hour	55%	91	52%	48%	47	44

SOURCE: ITE, "Trip Generation", 11th Edition (2021), Excel Pass-By Tables

Estimated Value

TRIP GENERATION CALCULATIONS

BUSINESS HOTEL

Land Use Code 312

T = Number of Vehicle Trip Ends

X = Rooms = 100

TIME PERIOD	TOTAL TRIP GENERATION <u>EQUATION</u>	TOTAL TRIP <u>ENDS</u>	PERCENT <u>ENTERING</u>	PERCENT EXITING	TOTAL TRIP ENDS <u>ENTERING</u>	TOTAL TRIP ENDS <u>EXITING</u>
AVERAGE WEEKDA	Y					
Daily	T = 2.90 (X) + 151.69	442	50%	50%	221	221
AM Peak Hour	T = 0.30 (X) + 6.94	37	39%	61%	14	23
PM Peak Hour	T = 0.21 (X) + 12.03	33	55%	45%	18	15

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

TRIP GENERATION CALCULATIONS

MULTIFAMILY HOUSING (LOW-RISE) Not Close to Rail Transit

Land Use Code 220

T = Number of Vehicle Trip Ends

X = Number of Dwelling Units = 52

TIME PERIOD	TRIP GENERATION EQUATION	TOTAL TRIP <u>ENDS</u>	PERCENT ENTERING	PERCENT <u>EXITING</u>	TOTAL TRIP ENDS <u>ENTERING</u>	TOTAL TRIP ENDS <u>EXITING</u>
WEEKDAY						
Daily	T = 6.74 (X)	350	50%	50%	175	175
AM Peak Hour	T = 0.40 (X)	21	24%	76%	5	16
PM Peak Hour	T = 0.51 (X)	27	63%	37%	17	10

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

TABLE 5 TRIP GENERATION CALCULATIONS

STRIP RETAIL PLAZA (Less Than 40,000 gsf)

Land Use Code 822 T = Number of Vehicle Trip Ends Size of Buildings = 10,000 gsf ----> % 10.0

TIME PERIOD AVERAGE WEEKDA	TOTAL TRIP GENERATION EQUATION AY	TOTAL TRIP <u>ENDS</u>	PERCENT <u>ENTERING</u>	PERCENT EXITING	TOTAL TRIP ENDS <u>ENTERING</u>	TOTAL TRIP ENDS <u>EXITING</u>
Daily	T = 54.45 X	544	50%	50%	272	272
AM Peak Hour	Ln(T) = 0.66Ln(X) + 1.84	29	60%	40%	17	12
PM Peak Hour	Ln(T) = 0.71Ln(X) + 2.72	78	50%	50%	39	39

SOURCE: Institute of Transportation Engineers, "Trip Generation", 11th Edition (2021)

NEW TRIPS

		NEW			NEW	NEW
		TRIP	PERCENT	PERCENT	TRIP ENDS	TRIP ENDS
TIME PERIOD	PERCENT NEW TRIPS	ENDS	ENTERING	<u>EXITING</u>	ENTERING	EXITING
AVERAGE WEEKDAY	ζ.					
Daily	64.0%	348	50%	50%	174	174
AM Peak Hour	64.0%	18	60%	40%	11	7
PM Peak Hour	64.0%	50	50%	50%	25	25
20112 02 TED 11						

SOURCE: ITE "Trip Generation Handbook", 3rd Edition, Table E.9

Estimated Value

TABLE 6 UNSIGNALIZED INTERSECTION CAPACITY RESULTS EXISTING CONDITIONS

NE PARK STREET / SE 13TH AVENUE

		WEEKDAY AM PEAK HOUR						
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)				
Eastbound U-Turn	C	21.9 sec/veh	0.02	1				
Westbound Left Turn	В	11.0 sec/veh	0.01	1				
Northbound Approach	C	15.6 sec/veh	0.03	1				

		WEEKDAY PM PEAK HOUR						
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)				
Eastbound U-Turn	C	20.8 sec/veh	0.02	1				
Westbound Left Turn	В	13.7 sec/veh	0.08	1				
Northbound Approach	C	16.6 sec/veh	0.06	1				

NE PARK STREET / HAMPTON INN DRIVEWAY

		WEEKDAY AM PEAK HOUR					
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)			
Westbound Left Turn	В	10.4 sec/veh	0.01	1			
Northbound Right Turn	В	12.3 sec/veh	0.03	1			

	WEEKDAY PM PEAK HOUR						
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)			
Westbound Left Turn	В	12.7 sec/veh	0.04	1			
Northbound Right Turn	В	14.9 sec/veh	0.05	1			

TABLE 7UNSIGNALIZED INTERSECTION CAPACITY RESULTS2024 BUILD CONDITIONS

NE PARK STREET / 13TH AVENUE

		WEEKDAY AM PEAK HOUR						
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)				
Eastbound Left Turn	C	15.1 sec/veh	0.07	1				
Westbound Left Turn	В	11.3 sec/veh	0.01	1				
Northbound Approach	В	13.7 sec/veh	0.02	1				
Southbound Approach	С	15.4 sec/veh	0.15	1				

		WEEKD	AY PM PEAK HO	UR
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Eastbound Left Turn	C	18.9 sec/veh	0.30	1.3
Westbound Left Turn	В	14.9 sec/veh	0.09	1
Northbound Approach	C	15.5 sec/veh	0.05	1
Southbound Approach	C	18.1 sec/veh	0.34	1.5

NE PARK STREET / NE 12TH AVENUE / HAMPTON INN DRIVEWAY

		WEEKD	AY AM PEAK HO	UR
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Westbound Left Turn	C	17.6 sec/veh	0.11	1
Northbound Right Turn	В	12.5 sec/veh	0.03	1
Southbound Right Turn	В	14.0 sec/veh	0.00	1

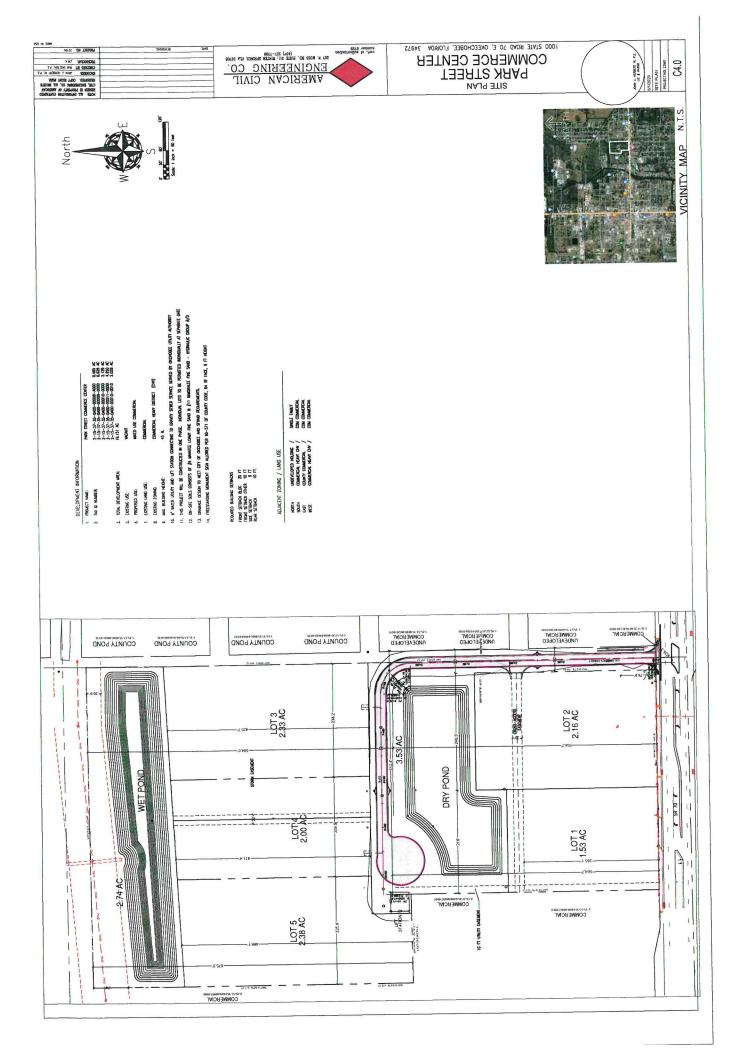
		WEEKD	AY PM PEAK HO	UR
Movement	LOS	Delay	V/C Ratio	95th % Queue (vehicles)
Westbound Left Turn	F	68.7 sec/veh	0.72	4.3
Northbound Right Turn	C	15.9 sec/veh	0.05	1
Southbound Right Turn	C	15.2 sec/veh	0.12	1

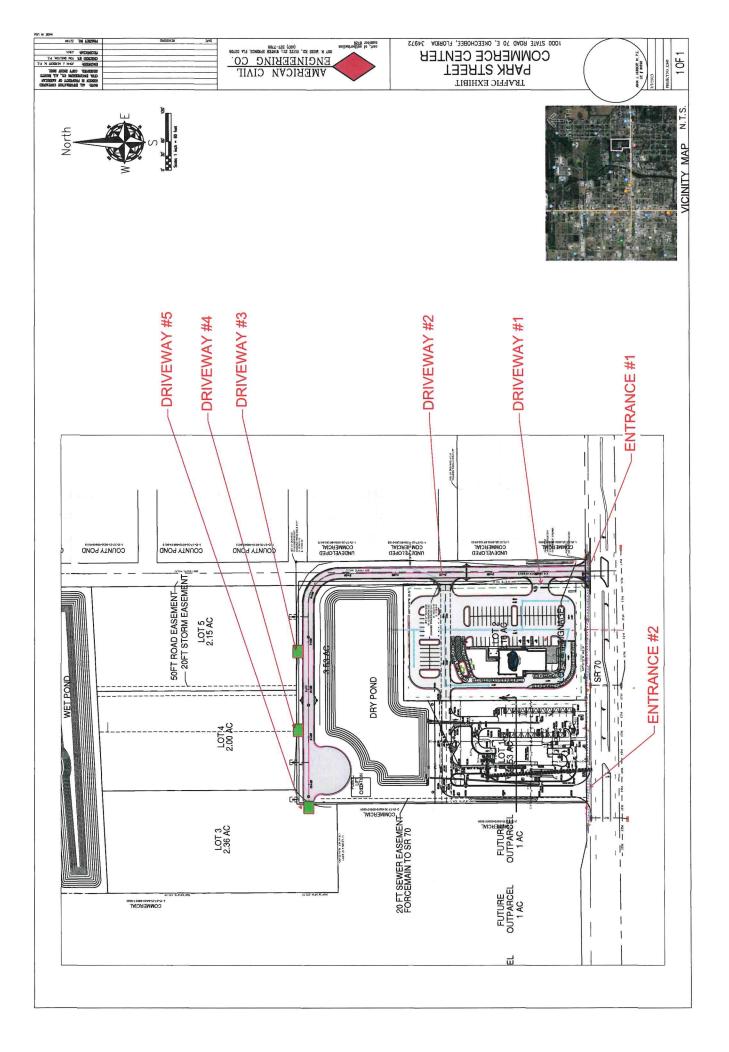
APPENDIX A

SITE PLAN



PROJECT DIRECTORY Mean and a feature and a state of the and a state o	PLANS ISSUED FOR: DATE OKECHOBEE. SITE PLAN REVEW 1/15/2023 OKECHOBEE. SITE PLAN REVEW 1/15/2023 INDEX OF SHEETS 1/15/2023 SHEET DESCRIPTION SHEET DESCRIPTION CI0 SHERT AND CI0 MSTER BANPFLAN CI0 MSTER AGNONG PLAN CI0 UTITYPELAN CI0
TE PLANS OF: OMMERCE CENTER SHP 3 SOIIT: AME 35 EAST SS-000-00009-0001 1: 2-15-37-33-0400-00010-0010 1: 2-15-37-33-0400-00010-0010	NORTH NORTH UTILITY COMPANIES SAURAY SEVER: 065075-96E UTILITY AUTHORITY WATER DISTRIBUTION: 0650753-9400 KATER OFFICIAL POWER: 72, 353-9400 ELECTRICAL POWER: 72, 353-9400 ELECTRICAL POWER: 72, 353-9400 FIR: 06507535-3576 FIR: 06507535-3576 MATER DISTRIBUTION: 0650755-3576 MATER DISTRIBUTION: 0650755-3576 MATER DISTRIBUTION: 06507555-3576 MATER DISTRIBUTION: 05507555-3576 MATER DISTRIBUTION: 05507555555 MATER DISTRIBUTION: 05507555555555 MATER DISTRIBUTION: 05507555555555555555555555555555555555
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APPENDIX B

TURNING MOVEMENT COUNTS



TABLE B-1 NE Park Street (SR 70) / Hampton inn Driveway TURNING MOVEMENT COUNTS - ALL VEHICLES

		wonday, rebi	uary 15, 2025			
	N	E PARK STREE	T	HAMPTON IN	N DRIVEWAY	Sec. 1
	EB RightTurn	WB U-Turn	WB Left Turn	NB Left Turn	NB Right Turn	All
6:45-7:00 AM	0	0	1	0	0	1
7:00-7:15 AM	0	0	0	0	0	0
7:15-7:30 AM	1	0	2	0	1	4
7:30-7:45 AM	1	0	0	0	4	5
7:45-8:00 AM	0	0	2	2	4	8
8:00-8:15 AM	0	0	0	0	4	4
8:15-8:30 AM	3	0	0	0	3	6
8:30-8:45 AM	2	0	0	0	2	4
AM PEAK PERIOD:	7	0	5	2	18	32
AM PEAK HOUR:	2	0	4	2	13	21

Monday, February 13, 2023

M PEAK HOUR: 7:15-8:15 AM

Monday, February 13, 2023

	N	E PARK STREE	ET .	HAMPTON IN	N DRIVEWAY	and a
	EB RightTurn	WB U-Turn	WB Left Turn	NB Left Turn	NB Right Turn	All
3:45-4:00 PM	4	0	3	0	5	12
4:00-4:15 PM	4	1	7	0	5	17
4:15-4:30 PM	1	0	3	0	4	8
4:30-4:45 PM	3	0	5	0	9	17
4:45-5:00 PM	13	0	6	0	12	31
5:00-5:15 PM	8	0	2	0	5	15
5:15-5:30 PM	7	0	5	0	4	16
5:30-5:45 PM	14	0	5	0	4	23
5:45-6:00 PM	9	0	8	0	5	22
PM PEAK PERIOD:	63	1	44	0	53	161
PM PEAK HOUR:	38	0	20	0	18	76
5:00-6:00 PM						

DATE: 02/13/23

WEATHER: CLEAR & DRY BEGIN TIME (MILITARY):06:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC MANUAL TURNING MOVEMENT COUNTS NE PARK STREET @ SE 13TH AVENUE OKEECHOBEE COUNTY, FLORIDA

Site Code : 4444444 Start Date: 02/13/23 File I.D. : 021323AM Page : 1

AUTOMOBILES, COMMERCIAL VEHICLES

					NE PARK	STREET		12	SE 13TH	AVENUE			NE F	PARK	STREET			
	From Nor	th		1	From Eas	st		11	from Sou	uth			From	wes	t		1	
				t				I					1				T	
	Left			Other			-	U-TURN			-	Other	Le	eft	Thru	Right	U-TURN	Tota
ate 02/	13/23																	
6:45	0	0	0	0	1	220	0	0	0	0	2	0	I	0	218	1	1	44
7:00	0	0	0	0	0	169	1	1	0	0	3	0	I	0	249	1	1	43
7:15	0	0	0	0	4	220	0	0	0	0	2	0	1	0	209	l	1	43
7:30	0	0	0	0	1	314	0	0	0	0	2	0	1	0	217	1	2	53
Ir Total	0	0	0	0	6	923	1	1	0	0	9	0	1	0	893	4	5	184
)7:45	0	0	0	0	2	332	0	0	1	0	ı	0	I.	0	244	1	1	58
8:00	0	0	0	0	0	294	0	0	1	0	2	0	Ĺ	0	246	1	0	54
8:15	0	0	0	0	2	211	0	0	1	0	0	0	l	0	184	0	1	3
8:30	0	0	0	0	0	237	0	0	1	0	0	0		0	239	0	0	4
ir Total	0	0	0	0	4	1074	0	0	4	0	3	0	I	0	913	2	2	200
TOTAL	0	0	0	0	10	1997	1	1	4	0	12	0	 l	0	1806	6	ן ר	384
	r Analys rt 07:15		Entire	Intersec			Period:	07:15 to	08:15 0	on 02/1								
olume	0	0	0	0	07:15 7	1160	0	0	07:19 2		7			07:15				
ercent	0%	0%	0%	0%	1%	99%	0%	0%	2 22%	0 68	78%	0 0%		0	916	4	4	
k total		0.	0.8	0.01	1167	55%	0.5	0.0	225	Ua	185	0.5		0% 924	99%	0%	0%	
ighest	06:45			1	07:4			1	08:0	'n)8:00			1	
olume	0	0	0	0 1	2	332	0	0	1	0	2	0		00:00	246	1	0	
i total			Ū		334		0	- 1	3	v	2	0	÷	247	240	1	v 	
	. 0			1	.87			1	. 75					/			1	

DATE: 02/13/23 WEATHER: CLEAR & DRY

BEGIN TIME (MILITARY):06:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC MANUAL TURNING MOVEMENT COUNTS NE PARK STREET @ SE 13TH AVENUE

Site Code : 4444444 Start Date: 02/13/23 File I.D. : 021323AM Page : 1

OKEECHOBEE COUNTY, FLORIDA

					NE PARK	STREET		1.	SE 13TH	AVENUE			NE PARE	SIREEI		l	
F	'rom Nor	th			From Ea	st		11	From Sou	ith			From We	est		1	
	Left	Thru	Right	Other	 Left	Thru	Right	U-TURN	Left	Thru	Right	Other	 Left	Thru	Right	U-TURN	Tota
Date 02/1	.3/23																
06:45	. 0	0	0	0	1	186	0	0	0	0	1	0	0	174	1	1	36
07:00	0	0	0	0	0	143	1	1	0	0	2	0	0	208	1	1	35
07:15	0	0	0	0	3	195	0	0	0	0	1	0	0	176	0	1	37
07:30	0	0	0	0	1 1	266	0	0 1	0	0	2	0	0	177	1	2	44
Hr Total	0	0	0	0	5	790	1	1	0	0	6	0	0	735	3	5	154
07:45	٥	0	0	0	2	288	0	0	1	0	0	0	0	201	1	1	49
08:00	0	0	0	0	0	237	0	0	1	0	2	0	0	197	1	0	43
08:15	0	0	0	0	1	177	0	0	l	0	0	0	0	143	0	1	32
08:30	0	0	0	0	0	179	0	0	<u> </u>	0	0	0	0	195	0	0	37
Hr Total	0	0	0	0	3	881	0	0	4	0	2	0	0	736	2	2	163
TOTAL	0	0	0	0	8	1671	1	1	4	0	8	0	0	1471	5	7	317
Peak Hour				Interse				07:15 to									
Peak star	rt 07:15				07:1	.5		1	07:1	5			07:	15		ł	
Volume	0	0	0	0	6	986	0	0	2	0	5	0	0	751	3	4	
Percent	0%	0%	0%	0%	1%	99%	0%	0%	29%	0*	71%	0%	0%	99%	08	1%	
Pk total	0				992			I	7				758			-	
Highest	06:45				07:4	5		1	08:0	0			07:-	45		I	
Volume	0	0	0	0	2	288	0	0	ı	0	2	0	0	201	1	1	
Hi total	0				290			1	3				203			1	
PHF	. 0				.86			1	. 58				. 93				

DATE: 02/13/23

WEATHER: CLEAR & DRY

BEGIN TIME (MILITARY):06:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC MANUAL TURNING MOVEMENT COUNTS NE PARK STREET @ SE 13TH AVENUE OKEECHOBEE COUNTY, FLORIDA

Site Code : 4444444 Start Date: 02/13/23 File I.D. : 021323AM Page : 1

COMMERCIAL VEHICLES

																	•••••	
				1	NE PARK	STREET			SE 13TH				·		STREET		1	
	From Nor	th		1	From Eas	st		1	From Sou	ith			Fro	om Wes	st		1	
											-		1					
	Left			Other	Left	Thru	Right	Other	Left			Other		Left	Thru	-	Other	Total
Date 02/	13/23																	
06:45	0	0	0	0	0	34	0	o	0	0	1	0	1	0	44	0	0	79
07:00	0	0	0	0	0	26	0	0	0	0	1	0	1	0	41	0	0	68
07:15	0	0	0	0	1	25	0	0	0	0	1	0	1	1	33	1	0	62
07:30	0	0	0	0	0	48	0	0	0	0	0	0	Í	0	40	0	0 [88
Hr Total	0	0	0	0	1	133	0	0	0	0	3	0	1	1	158	1	0	297
07:45	0	0	0	0	0	44	0	0	0	0	1	0	Ĩ.	0	43	0	0	88
08:00	0	0	0	0	0	57	0	0	0	0	0	0	1	0	49	0	0	106
08:15	0	0	0	0	1	34	0	0	0	0	0	0	1	0	41	0	0	76
08:30	0	0	0	0	0	58	0	0	0	0	0	0	1	0	44	0	0	102
Hr Total	0	0	0	0	ı	193	0	0	0	0	1	0	I	0	177	0	0	372
TOTAL	0	0	0	0	2	326	0	0	0	0	4	0		1	335	1	0	669
	ir Analys			Intersec	tion fo	r the H	Period:	07:15 tc	08:15									
Peak sta	rt 07:15	5		1	07:1	5		1	07:1	5				07:19	5		1	
Volume	0	0	0	0	1	174	0	0	0	0	2	0	1	1	165	1	0	
Percent	0%	0%	0%	0%	1%	99%	0%	0%	0%	0%	100%	0%	1	1%	99%	1%	0%	
Pk total	0			1	175			i	2				1	167			1	
Highest	06:45	5			08:0	0		I	07:1	5			1	08:00	D		1	
Volume	0	0	0	0	0	57	0	0	0	0	1	0	1	0	49	0	0	
Hi total	0				57			1	l				1	49			1	
PHF	. 0			1	. 77				.50				1	.85			1	

DAY: MONDAY DATE: 02/13/23 WEATHER: CLEAR & DRY BEGIN TIME (MILITARY):06:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC MANUAL TURNING MOVEMENT COUNTS NE PARK STREET @ SE 13TH AVENUE OKEECHOBEE COUNTY, FLORIDA

Site Code : 4444444 Start Date: 02/13/23 File I.D. : 021323AM Page : 1

PEDESTRIAN & BICYCLE

					NE PARK				SE 13TH				NE PARE	STREET	:	1	
	From Nor	th			From Eas	st		1	From Sou	ith			From We	est		1	
	Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	 Left	Thru	Right	PEDS	Tota
ate 02/	13/23						*******										
6:45	0	0	0	0	0	0	0	0	o	0	0	0	0	0	0	0	
7:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
)7:30	0	0	0	1	0	0	0	0	00	0	0	0	1 0	0	0	0	
ir Total	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0 [
7:45	0	0	0	2	0	0	0	0	0	0	0	1	0	0	0	1	
8:00	0	0	0	2	0	0	0	0	0	0	0	1	0	0	0	0	
8:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:30	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	
ir Total	0	0	0	5	0	0	0	0	0	0	0	2	0	0	0	1	
TOTAL*	0	0	0	6	0	0	0	0	0	0	0	2	0	0	0	1	9
	r Analys			Interse	ction for	r the F	eriod:	07:15 to	08:15	on 02/1		-					
Peak sta	rt 07:15	i			07:1	5			07:1	5			07:	15		1	
/olume	0	0	0	5	0	0	0	0	0	0	0	2	0	0	0	1	
ercent	0%	0%	0%	100%	0%	0%	0%	0%	0%	0%	0%	100%	0%	0%	0%	100%	
k total	5				0			-	2				1			1	
ighest	07:45	i			06:4	5		1	07:4	5			07:	45		1	
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i total	2				1 0			1	1				1			1	
HF	. 62				.0				. 50				. 25				

DATE: 02/13/23

WEATHER: CLEAR & DRY

BEGIN TIME (MILITARY):15:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC MANUAL TURNING MOVEMENT COUNTS NE PARK STREET @ SE 13TH AVENUE OKEECHOBEE COUNTY, FLORIDA

Site Code : 02132023 Start Date: 02/13/23 File I.D. : 021323PM Page : 1

					NE PARK	STREET		15	SE 13TH 2	AVENUE			NE F	PARK S	STREET		1	
Fro	om Nor	th			From Ea	st		1	From Sou	zh			From	n West			1	
					1			1					1					Maha
1	Left	Thru	Right	Other	Left	Thru	Right	U-TURN				Other			Thru		U-TURN (Tota
ate 02/13,	/23																	
5:45	0	0	0	0	1	269	0	1	1	0	3	0	I	0	291	3	1	57
6:00	0	0	0	0	4	257	0	0	2	0	5	0	1	0	324	9	1	60
6:15	0	0	0	0	2	295	0	2	1	0	l	0	l	0	313	4	0	61
.6:30	0	0	0	0	6	300	2	1	4	0	4	0	1	0	255	4	3	57
Ir Total	0	0	0	0	13	1121	2	4	8	0	13	0	1	0	1183	20	5	236
6:45	0	0	0	0	6	289	0	1	2	0	2	0	١	0	265	0	2	56
7:00	0	0	0	0	12	306	0	0	0	0	2	0	1	0	375	1	2	69
7:15	0	0	0	0	10	289	0	0	0	0	4	0		0	332	2	1	6.
17:30	0	0	0	0	4	277	0	0	1	0	4	0	1	0	301	5	2	5
Hr Total	0	0	0	0	32	1161	0	1	3	0	12	0	I	0	1273	8	7	249
17:45	0	0	0	0	6	311	0	2	1	0	7	0	1	0	273	7		6
Hr Total	0	0	0	0	6	311	0	2	l	0	7	0	1	0	273	7	0	6
TOTAL	0	 0	0	0	51	2593	2	7	12	0	32	0	1	0	2729	35	12	54
Peak Hour			Entire	Inters		tor the	FGLT00:	17:00 C	1 17:0				1	17:0	D		1	
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DATE: 02/13/23 WEATHER: CLEAR & DRY

BEGIN TIME (MILITARY):15:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC MANUAL TURNING MOVEMENT COUNTS

NE PARK STREET @ SE 13TH AVENUE OKEECHOBEE COUNTY, FLORIDA Site Code : 02132023 Start Date: 02/13/23 File I.D. : 021323PM Page : 1

AUTOMOBILES

					NE PARK			•	SE 13TH				NE PARI			1	
FI	com Nor	th			From Ea	st		1	From Sou	ith			From We	est		1	
	Left	Thru	Right	Other	l Teft	Thru	Right	U-TURN	Left	Thru	Right	Other	 I.oft	Thru	Picht	U-TURN	Tota
ate 02/13	-		-				-				-						
.5 : 45	0	0	0	0	1	236	0	1	1	0	3	0	1 0	261	3	1	50
6:00	0	o	ů O	0		214	0	0	2	ů 0	5	0	•		9	1	51
6:15	0	o	0	0	•	252	0	2	- 1	0	1	0	1	267	4	0	52
6:30	0	0	0	0	•	258	2	1	4	0	4	0			4	3	50
ir Total	0	0	0	0		960	2	4	8	0	13	0			20	5	205
.6 : 45	0	0	0	0	6	257	0	1	2	0	2	0	0	221	0	2	49
.7:00	0	0	0	0	11	279	0	0	0	0	2	0	0	337	1	2	63
7:15	0	0	0	0	10	259	0	0	0	0	4	0	0	293	2	1	5
7:30	0	0	0	0	4	254	0	0	1	0	4	0	1 0	276	5	2	54
ir Total	0	0	0	0	31	1049	0	1	3	0	12	0	0	1127	8	7	223
.7:45	0	0	0	0	•	282	0	2	1	0	7	0	1 0	247	7	0	55
ir Total	0	0	0	0	6	282	0	2	1	0	7	0	0	247	7	0	55
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k total	0				1107			1	19				1173			1	
ighest	15:45				17:0	0			17:4	5			17:				
olume	0	0	0	0	11	279	0	0	1	0	7	0	0	337	1	2	
i total	0				290				8				340			, I	
					•				. 59							'	

DATE: 02/13/23

WEATHER: CLEAR & DRY

BEGIN TIME (MILITARY):15:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC MANUAL TURNING MOVEMENT COUNTS NE PARK STREET @ SE 13TH AVENUE OKEECHOBEE COUNTY, FLORIDA

Site Code : 02132023 Start Date: 02/13/23 File I.D. : 021323PM Page : 1

COMMERCIAL VEHICLES

-					NE PARK				SE 13TH						STREET		1	
Fi	com Nor	th			From Ea	st			From So	uth			Fre	om Wes	st			
	Left	Thru	Dight	Other	Toft	Thru	Dight	Other	 Left	Thru	Dicht	Other	1	OFF	Thru	Dicht	Other	Tota
ate 02/11							-						1 1	De L C	iniu	Right	other	100
ace 02/1.	5/25																	
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6:00	0	0	0	0	1 0	43	0	0	0	0	0	0	1	0	40	0	0	
6:15	0	0	0	0	0	43	0	0	1 0	0	0	0	Î.	0	46	0	0	
6:30	0	0	0	0	0	42	0	0	0	0	0	0	1	0	33	0	0	1
r Total	0	0	0	0	0	161	0	0	0	0	0	0	I	0	149	0	0	3
6:45	0	0	0	0	0	32	0	0	0	0	0	0	I	0	44	0	0	
7:00	0	0	0	0	1	27	0	0	0	0	0	0	I	0	38	0	0	
7:15	0	0	0	0	0	30	0	0	0	0	0	0	1	0	39	0	0	
7:30	0	0	0	0	0	23	0	0	0	0	0	0		0	25	0	0	
r Total	0	0	0	0	1	112	0	0	0	0	0	0	I	0	146	0	0	2
7:45	0	0	0	0	0	29	0	0	0	0	0	0	1	0	26	0	0	
r Total	0	0	0	0	0	29	0	0	0	0	0	0	I	0	26	0	0	
TOTAL*	0	0	0	0	1	302	0	0	0	0	0	0		0	321	0	0	62
eak Hour										on 02/1								
eak star		-			1 17:0				17:0				1	17:00)		1	
olume	0	0	0	0	1 1	109	0	0	4. 5.37	0	0	0	i	0	128	0	0	
ercent	0%	0%	0%		,	99%			• °	0%	0%	08		0%	100%	0%	0%	
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ghest	15:45				17:	15			15:4	5			i	17:1	5		1	
olume	o	0	0	0		30	0	0		0	0	0	i	0	39	0	0	
i total	0		-	•	1 30	5.5			0	-	U U		1	39		· ·		
					1 50													

DATE: 02/13/23 WEATHER: CLEAR & DRY

BEGIN TIME (MILITARY):15:45 Hrs

JW BUCKHOLZ TRAFFIC ENGINEERING INC MANUAL TURNING MOVEMENT COUNTS NE PARK STREET @ SE 13TH AVENUE OKEECHOBEE COUNTY, FLORIDA

Site Code : 02132023 Start Date: 02/13/23 File I.D. : 021323PM Page : 1

PEDESTRIAN & BICYCLE

Fi	com Nor	th			NE PARK				SE 13TH . From Sou				NE PARK			1	
													1				
	Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	Left	Thru	Right	PEDS	Tota
Date 02/13	3/23																
15:45	0	0	0	0	0	0	0	0	0	o	0	0	0	0	0	0	
16:00	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	
16:15	0	0	0	0	O	0	0	0	0	0	0	0	0	0	0	0	
16:30	0	0	0	0	<u> </u>	0	0	0	1	0	0	0	0	0	0	0 [
Hr Total	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	
16:45	0	0	0	0	0	0	0	0	0	0	0	o	0	0	0	0	
17:00	0	0	0	0	0	0	0	o	0	0	0	0	0	0	0	0	
17:15	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	
17:30	0	0	0	1	<u> 0</u>	0	0	0	0	0	0	0	1 0	0	0	0	
Hr Total	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	
17:45	0	0	0	3	0	0	0	0	10	0	0	0	<u> </u>	0	0	0	
Hr Total	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	
TOTAL	0	0	0	6	0	0	0	1	0	0	0	1	0	0	0	0	8
Peak Hour							Deriod.										
Peak star			GHERIC	meerse	17:0			17.00 0	17:00		15/15		17:0	0		1	
Volume	0	0	0	6	•	0	0	0		, 0	0	0	•	0	0	0	
Percent	0%	0%	0%	100%		0*	0%	0%		0*	0%	0%	,	0%		0%	
Pk total	6				1 0	- •			1 0	- •			0		- •		
Highest	17:45	5			15:4	5			15:45	5			15:4	15		1	
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	-				1 V				, •				, •			1	

APPENDIX C

FDOT TRAFFIC DATA



TABLE C-1 LINEAR REGRESSION ANALYSIS

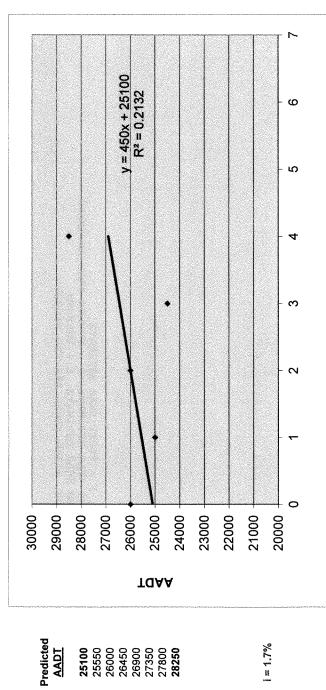


Actual AADT (Y)

<u>Year</u> <u>X</u>

26000 25000 26000 24500 28500

2017 0 2018 1 2019 2 2020 3 2022 5 2022 5 2023 6 2023 6



BUCKHOLZ TRAFFIC

FLORIDA DEPARTMENT OF TRANSPORTATION TRANSPORTATION STATISTICS OFFICE 2021 HISTORICAL AADT REPORT

COUNTY: 91 - OKEECHOBEE

SITE: 0007 - SR 70, WEST OF SR 710/EAST OF OKEECHOBEE

T FACTOR	18.20	16 50		14.8U	17.90	15.30	13 00		13.60	14.40	UV VL	0	14.40	11.60	11 60		L3.6U	16.50		0/ · CT	15.50	
D FACTOR			07.10	57.30	57.90	58.80		07.70	56.60	58.10		01.00	57.50	56.90	22 27	10. VI	57.93	57,88		60.38	58 36	
*K FACTOR	00 0		9.00	9.00	00.6	00 6		9.00	9.00	00 6		9.00	9.00	00 6		TU.33	10.97	30 11	00·11	10.65	10 EA	۴0.0H
DIRECTION 2			W 12500	W 13000	M 12500			W 13000	W 10000		M FOOO	W 10500	W 10500			W TISUO	W 11000		DOCTT M	W 12000		M TZOOO
DIRECTION 1		E 14000	E 12000	F 12000			E TSUUU	E 12500	F: 10000		DOCOT H	E 10500	F: 10500			E 11000	F 11000		DOGTT R	F: 11500		E 11500
AADT		28500 C	24500 C				26000 C	25500 C			21000 S	21000 F			225000 F	22500 C		2 2000 2	23000 C	JAEOO C	00001	23500 C
YEAR	1 1 1	2021	0000		STO7	2018	2017	2016		CT N7	2014	5100		7TO7	2011	2.010		2002	2008	5000	1007	2006

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN *K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE X30 VALUES *K FACTOR:

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UNTY: ATION SCRIP ART DI ART T		12	WEST 0 021	SR	710/EAST	OF OKEECHOBEE	HOBEE							
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A. W. A. W. A. W. A. W. A. W. A. M. M. A. M.	111 111 111 111 111 111 1115 1115 1115		VOLUME 1137 144 146 1137 1137 1137 1137 1137 1137 1137 882 1282 1125 1125 1125 1282 2282 1282 1	Д Д Д Д Д Д Д Д Д Д Д Д Д Д	73 73 73 74 74 74 77 77 74 77 77 74 74 77 74 712 712 712 712 712 712 712 712 712 712		30 23 19 117 19 117 156 123 1556 799 123 123 167 24 179 117 123 130 123 239 221 239 221 239 221 239 227 239 227 239 213 239 214 177 174 174 174 174 172 179 217 239 218 177 172 179 217 239 218 177 172 179 217 239 218 177 172 179 217 239 218 177 239 239 239 239 107 171 107 171	СССССССО ССССССССССССССССССССССССССССС	14 14 23 23 23 23 23 23 23 23 23 23 23 23 23	79 71 71 71 125 318 689 988 830 704 749 819 749 819 762 752 819 819 762 752 835 835 835 9267 173 173 173 173 173 173 173 173 173 17	152 152 101 105 101 1775 1531 1531 1533 1556 1556 1556 1556 155			
DIR 1 E 174 W 148	2 6138 6049	3 4243 4048	5 6 4 5 6	CLAS 5 1045 897	CLASSIFICATION 5 6 7 45 259 41 97 383 24	SUMM 8 25 27		DATABASE 9 10 673 21 730 23	11 20	12 1 1	13 14 100 100	15 TOT 27 22 508 23	OTTRK TOTVOL 2362 12944 2395 13148	/OL 944 148
GENERATED	Ъ Т	SPS 5.0.	55P	1 4 1 1 1	8 9 9 8 4 8	5 5 1 1 2 2 4		1 1 1 1 1	1 1 1 1 1 1	1 1 1 1 1 1 1	E E E E E E E E E E E E E E E E E E E			1

TABLE C-2 LINEAR REGRESSION ANALYSIS

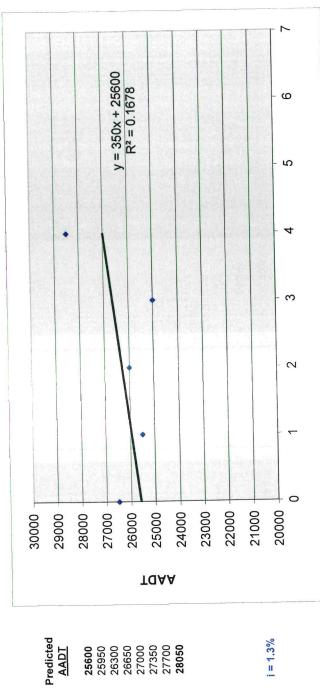


Actual <u>AADT (Y)</u>

<u>Year</u> <u>X</u>

26500 25500 26000 25000 28500

2017 0 2018 1 2019 2 2020 3 2021 4 2022 5 2023 6 2023 6 2023 6



FLORIDA DEPARTMENT OF TRANSPORTATION TRANSPORTATION STATISTICS OFFICE 2021 HISTORICAL AADT REPORT

COUNTY: 91 - OKEECHOBEE

SITE: 5012 - SR 70, EAST OF SR 15/700/US 98/441

YEAR	AADT	DI	DIRECTION 1	IQ	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	28500 C	i Ei	14000	N	14500	00.6	58.00	15.60
2020	25000 C	E4	12500	Μ	12500	9.00	57.20	18.00
2019	26000 C	ы	13000	М	13000	9.00	57.30	16.40
2018	25500 C	ы	13000	3	12500	9.00	57.90	17.60
2017	26500 C	ы	13500	М	13000	9.00	58.80	14.00
2016	25500 C	ជ	13000	М	12500	9.00	57.40	13.50
2015	27000 C	ы	13500	M	13500	9.00	56.60	13.00
2014	28000 C	ы	14000	Μ	14000	9.00	58.10	13.20
2013	26000 F	ы	13000	M	13000	9.00	58.10	11.30
2012	26000 C	ы	13000	М	13000	9.00	57.50	11.30
2011	29500 F	ы	14500	М	15000	9.00	56.90	8.20
2010	29500 C	ы	14500	Μ	15000	10.99	56.24	8.20
2009	31500 C	ជ	16000	М	15500	10.97	57.93	8.70
2008	29500 C	ы	15000	Μ	14500	11.05	57.88	8.00
2007	31500 C	ы	16000	Μ	15500	10.65	60.38	8.40
2006	35000 C	ជា	18000	М	17000	10.64	58.36	7.50

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN *K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES *K FACTOR:

					TOTTRK TOTVOL 1906 13029 2177 13147
			1		15 74 56
		本 本 本 本 本 本 4 4 4 4 4 4 4 4 4 4 4 4 4			14
		00000000000000000000000000000000000000	.60		113
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	DIRE 2ND	112 112 112 112 112 112 112 112 112 1		/ DATABASE	9 723 743
441	1ST	21 21 21 21 21 21 21 21 23 23 23 23 23 23 23 23 23 23	16.56	SUMMARY	8 200 220
5 98/441	1 1 1				7 40 20
15/700/US		75 75 75 75 75 75 75 75 75 75 75 75 75 7		CLASSIFICATION	6 198 247
F SR 15	Е 4 ТН			CLAS	5 665 874
AST 0 21		VOLUQA N.: VOLUQA VOLUA	63		4 5 0 4 5 0
91 5012 SR 70, 1 08/11/2 1645	DIRE(2ND	C	14.		3 3611 4132
	ST		PERCENTAGE		2 7300 6641
COUNTY: STATION: DESCRIPTION START DATE: START TIME:			1		1 138 141
COUNTY: STATION DESCRIP START DI START TI	· F	A M M M M M M M M M M M M M M M M M M M	TRUCK		DIR W

GENERATED BY SPS 5.0.55P

APPENDIX D

CAPACITY CALCULATIONS UNSIGNALIZED INTERSECTIONS



AM PEAK HOUR

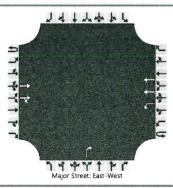
		and the second second	HCS 1		2.2.97	- Sugar				4						
General Information							Site I	Inforn	natior	1						
Analyst	J. Buc	kholz	in Sector of the Sector Sec	*******	9,538,669,669,669,669,669,669,669,669,669,66		Inters	ection		i in an	NE Pa	irk Street	:/SE 13	th Aven	ue	
Agency/Co.	BUCK	HOLZ T	RAFFIC	NER ANN ANNA ANNA			Jurisd	iction	000000000000000000000000000000000000000	******	Okeed	chobee C	County		antal de resen a despersión	*****
Date Performed	2/14/2	2023	*****			ana mangang kana kana kana kana kana kana kan	East/\	Nest Stre	et	*****	NE Pa	rk Street		ingóil stálaí)ú hindinna	n (na stand an	inghigt guniya gudiga
Analysis Year	2023	******					North	/South S	itreet		SE 13	th Avenu	e			1940-1929/2049900
Time Analyzed	Week	day AM	Peak Ho	our	ning (ning an	skolota kanstriates lett	Peak	Hour Fac	tor	<u>Distructured Longen</u>	0.86	68000/07401080-1740074081			and the second second second se	
Intersection Orientation	East-V	Vest	within a shake the				Analy	sis Time	Period (hrs)	0.25	atinisiendessusets	27724001942001246090		Salah Disebut dan dari kang barang	nay 2014 ng ang ang ang ang ang ang ang ang ang
Project Description	#23-1	820	i cimun inning agus	*****	ana ang ang ang ang ang ang ang ang ang	and an a faith of the second secon	2	la interpretation de la constantion aplema		ationis per provident mar	8		20.461200 0 000200000	*******	ektrimistrikisterastan samt und	
Lanes		Internet Manufacut		*****				998.899.999.999.999					******		*****	12/04/04/25/95/25/25/25/25/25/25/25/25/25/25/25/25/25
						or Street: Ea						apanae franska kara opera		MCR20-SIG/V0-9U-04	escrito;conton;n culu	
Vehicle Volumes and Ad	ljustme	nts											-			
Approach	-		bound	generation and the second			bound				bound			y	bound	
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	L T	R
Priority	10	1	2	3	4U	4	5	6	-	7	8	9	-	10	11	12
Number of Lanes	1	0	2	0	0	1	2	0		0	1	0		0	0	0
Configuration	U	NV77 2010 NVAN4423	T	TR		L	T		****		LR		1970-1997-1997-1997-1997-1997-1997-1997-		ļ	-
Volume (veh/h)	4		879	4	0	7	1114			2		7				ļ
Percent Heavy Vehicles (%)	0			ļ	0	14			-	0		29	*******		ļ	Ļ
Proportion Time Blocked			L	L	<u> </u>											L
Percent Grade (%)			NA CANADARA SI DA SI		L	***	ninezitar eta de la composition				0	aannaannaanna				maantaanaadaada
Right Turn Channelized		ussileiniksissinni misini) Institutionalisti			an a					Otovianominumindolasinomian					
Median Type Storage			adarya wite Manada kacameta a	Left -	+ Thru				auliakananananana	teansis automatication	a george and a subscription of the	1		an a		and the second
Critical and Follow-up H	leadway	ys														
Base Critical Headway (sec)	6.4			[4.1				7.5		6.9		I		Ī
Critical Headway (sec)	6.40	9780689997890190699		Î		4.38				6.80		7.48		Γ	Í	
Base Follow-Up Headway (sec)	2.5	an ann an Anna an Anna Anna Anna Anna A				2.2			haidi de pierre salarciria	3.5		3.3				
Follow-Up Headway (sec)	2.50	andred na designed and an an	1		l	2.34	Í			3.50	Ì	3.59		I		
	nd Leve	l of S	ervice	1										1268 H 123 H 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		******
a an			T	r	Γ	8	1	[]		<u></u>	10			Г	T	T
Delay, Queue Length, ar	5		สู้จะการแกรงสา	1	ŀ	605					350			1		\mathbf{h}
Delay, Queue Length, ar Flow Rate, v (veh/h)	5 218				8	3						lannana		fammenzoa	<u> </u>	1
Delay, Queue Length, ar Flow Rate, v (veh/h) Capacity, c (veh/h)	218	*****				0.01	outeros a			1	0.03			1	1	1
Delay, Queue Length, ar Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio	218 0.02	*****				0.01					0.03 0.1					-
Delay, Queue Length, ar Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio 95% Queue Length, Q95 (veh)	218 0.02 0.1					0.0					0.1					
Delay, Queue Length, ar Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio 95% Queue Length, Q ₉₅ (veh) Control Delay (s/veh)	218 0.02 0.1 21.9					0.0 11.0					0.1 15.6					
Delay, Queue Length, ar Flow Rate, v (veh/h) Capacity, c (veh/h) v/c Ratio 95% Queue Length, Q95 (veh)	218 0.02 0.1).1			0.0 11.0 B				1	0.1					

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General Information		Site Information	
Analyst	J. Buckholz	Intersection	NE Park Street / Hampton Inn Driveway
Agency/Co.	BUCKHOLZ TRAFFIC	Jurisdiction	Okeechobee County
Date Performed	2/14/2023	East/West Street	NE Park Street
Analysis Year	2023	North/South Street	Hampton Inn Driveway
Time Analyzed	Weekday AM Peak Hour	Peak Hour Factor	0.86
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	#23-1820		n Gezzana n (nemperaturan nemperaturan karan nemperaturan karan menteran nemperaturan karan karan karan karan ka

Lanes



Vehicle Volumes and Adjustments

Approach		Eastb	bound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6	1	7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	1	2	0		0	0	1	allowed and a second	0	0	0
Configuration		(*************************************	Т	TR	Construction of the second	L	Т					R				
Volume (veh/h)			875	2	0	4	1116			an and an		14	oda no kaconitra sono omno			
Percent Heavy Vehicles (%)					0	2		anna an an Anna Anna Anna A	1			2	anatoria - en dentar	and the second second		n petalanakan satilar
Proportion Time Blocked																1
Percent Grade (%)			Accession and the second		1		Arren and a second	A			0	Arrangements		Louisenan	Levennessee	la anticipa de la constante de
Right Turn Channelized										N	10					
Median Type Storage				Left	Only				1]				izmani Sizmandu d
Critical and Follow-up H	leadway	ys						1000						4.9		1.8
Base Critical Headway (sec)						4.1		decumenta di Soni dati			[6.9			[
Critical Headway (sec)		(and				4.14		1.2.2	1			6.94				
Base Follow-Up Headway (sec)						2.2		ncer Manafarenzada				3.3	beganning-soloestane			and post one "Links
Follow-Up Headway (sec)	i secol			r.		2.22		5.5				3.32				
Delay, Queue Length, ar	nd Leve	l of S	ervice	begenere en son en en en en	Automotion and a second	Longeron and second	domanon cacavado		Annen an		leanne na ann an	. 35	been an	Leonore and a second second		berraue a series and
Flow Rate, v (veh/h)	1					5		al ann sa réanna ann an a	1		-	16			[
Capacity, c (veh/h)		251				676						509			- 2013	
v/c Ratio				alarto contra a conservare		0.01						0.03				
95% Queue Length, Q ₉₅ (veh)						0.0						0.1	PARAMAN INC 12 (1997) 45 (199			
Control Delay (s/veh)						10.4						12.3				
Level of Service (LOS)						В		Salah da Marris I waxa da ka	100			В				
Approach Delay (s/veh)	1					0	.0			12	2.3				3	
Approach LOS			non er solle Autorit di Lenand			Inter and descent states	Ą				В					

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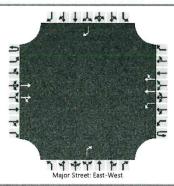
		ł	ICS 1	wo-	Way	Stop	o-Cor	ntrol	Repo	ort						
General Information							Site I	Inforr	natio	n						
Analyst	J. Buc	kholz	ini ni Sunna a contra				Inters				NE Pa	ark Street	t / SE 13	th Aveni	16	
Agency/Co.		HOLZ T	RAFFIC		1939-1936-1928-1938-1949-1949-1949-1949-1949-1949-1949-194		Jurisd				ผู้สองครองสองสองสองสองสองสองสองสองสองสองสองสองสอ	chobee (
Date Performed	7/7/2		3016-00030000-00000000			7181141.01994.519244	-	Vest Str	eet	bawww.manada.com	hannann	ark Street	en en san se	ariaander Factoriaan an	a ta a calado ta masa kan	
Analysis Year	2024	nin taala kaana kaana kaana ka		******		en te ponte analysis de la designate		/South !			สู้สมเหตุการและ	th Avenu	SALAR CONTRACTOR		****	200304/2010/00/4666
Time Analyzed	AM P	eak Hr. I	BUILD Tr	affic		1914-0-14-14-0-14-04-04		Hour Fa			0.86	anniqua an tao amin'		an a	1947 <u>9-19456-1947</u> 9	en ander som
Intersection Orientation	East-	West	n).Commissional commission				Analy	sis Time	Period	(hrs)	0.25			an (Constitution of Co		
Project Description	#23-1	1820	******	(electricales) and the second	arkina nasofi i kalili ana		1	n ar an		nant-Konserveriakisuksia				n fan se fan skiel de	alimeteri desirati anti anti	
Lanes	1	001161.27594.6.4943.		an a		1994 - 140 (1999), 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1										kana asaké da kana kang s
				JALAALU		or Street: Ea	ast -West									
Vehicle Volumes and Adju	ustme	ents														
Approach	-	Eastk	oound		Ī	West	bound		Ī	North	bound		I	South	bound	*****
Movement	U	L	Т	R	U	L	т	R	U	L	T	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6	4	7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	1	1	0	0	1		0	0	1
Configuration		L	T	TR	1	L	Т	R				R				R
Volume (veh/h)	4	18	917	4	0	7	1133	18	1	İ	1	9			-	51
Percent Heavy Vehicles (%)	0	2			0	14						29				2
Proportion Time Blocked		1	1	1		Í i			Ì	ĺ	ĺ	1				Ì
Percent Grade (%)											0				0	
Right Turn Channelized						1	No				No			Ν	lo	
Median Type Storage				Left ·	+ Thru								1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)	6.4	4.1				4.1	T		ľ			6.9				6.9
Critical Headway (sec)	6.40	4.14	1	Í	l	4.38				İ	İ	7.48	1			6.94
Base Follow-Up Headway (sec)	2.5	2.2	l .	l		2.2				Ī		3.3				3.3
Follow-Up Headway (sec)	2.50	2.22	I			2.34			I		Γ	3.59				3.32
Delay, Queue Length, and	I Leve	l of S	ervice)												
Flow Rate, v (veh/h)	[26	T	T	T	8	Γ	[[T	Τ	ľ	10	l i	T		59
Capacity, c (veh/h)	l	383		t		580	t					425	t	t		406
v/c Ratio	 	0.07	T	 		0.01	1			1	1	0.02		1	Ì	0.15
95% Queue Length, Q ₉₅ (veh)		0.2	t -	İ	İ	0.0				1	1	0.1	İ	İ	[0.5
Control Delay (s/veh)		15.1	t	İ.	T	11.3	1		1	1	Î	13.7	1	1		15.4
Level of Service (LOS)		с	t	1	İ	В	İ			1	1	В	l	 		с
	fennesen	สีเลาสองออกเสียงเล	eliceronanonanon	สมัครงารสองสรรมการจะสุด	tan and the second second second second second second second second second second second second second second s	aline and a state of the state).1	housensisten	1	ະອັກການເຄັນແລະນະ 1			former service and the service ser	สังหมวงสมมา	daaxaannaan r A	สมัญสารเหตุลางเล
Approach Delay (s/veh)		C).4			L L	1.1		ii a	T	3.7		202	1	5.4	

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General Information		Site Information	
Analyst	J. Buckholz	Intersection	NE Park St. / Hampton Inn / NE 12th Ave
Agency/Co.	BUCKHOLZ TRAFFIC	Jurisdiction	Okeechobee County
Date Performed	7/7/2023	East/West Street	NE Park Street
Analysis Year	2024	North/South Street	Hampton Inn Drive / NE 12th Avenue
Time Analyzed	AM Peak Hr. BUILD Traffic	Peak Hour Factor	0.86
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	#23-1820	an an an an an an an an an an an an an a	

Lanes



Vehicle Volumes and Adjustments

Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9	de de la companya de la companya de la	10	11	12
Number of Lanes	0	0	2	0	0	1	2	0		0	0	1		0	0	1
Configuration	1		Т	TR		L	Т	TR			Contraction of the second	R	andraating - margania			R
Volume (veh/h)			906	2	25	4	1159	0				14				1
Percent Heavy Vehicles (%)				anen aut tanan mana maanan	2	2		(all and a second second second second second second second second second second second second second second s				2				0
Proportion Time Blocked				and all and the summary process												
Percent Grade (%)		Geographic Annalysis (Construction	dageniouninggenerad		1	Basaran san san san san	allowers and an and an and an and an an an an an an an an an an an an an			(0	-		(0	deservation and the second second second second second second second second second second second second second
Right Turn Channelized										N	10	en constante a constante a constante a constante a constante a constante a constante a constante a constante a		N	lo	anter standon mendelande
Median Type Storage				Left	Only	Alabar (1997) (1997) (1997) (1997)	Strein und sin ereinen antorise			tano		1	L		********	
Critical and Follow-up H	leadway	ys					00001008008000800000000000									
Base Critical Headway (sec)					6.4	4.1		Kalina (Annan I) Angelan				6.9		annai geolanteida		6.9
Critical Headway (sec)		in.k			6.44	4.14		2				6.94		1		6.90
Base Follow-Up Headway (sec)					2.5	2.2						3.3		MALE ROOM CONTRACTOR	Participant Practice	3.3
Follow-Up Headway (sec)	6 22.202				2.52	2.22						3.32			27 10.	3.30
Delay, Queue Length, ar	nd Leve	l of S	ervice		Accession	fearen artaran en e	damanaan	NCRUNCALCHUR CO.			Longer Arte Martinese	Section 1			Assantanta	
Flow Rate, v (veh/h)					1	34					1	16				1
Capacity, c (veh/h)	120 -					320						495				402
v/c Ratio		and the Castal State State State State			1	0.11						0.03				0.00
95% Queue Length, Q ₉₅ (veh)		1		en felen i del groot have of laar		0.4						0.1				0.0
Control Delay (s/veh)					1	17.6						12.5				14.0
Level of Service (LOS)		Name of Contract of Contract				С						В	-			В
Approach Delay (s/veh)		an bottombartid on das Aldonian	Lonnanangerard	an a maarine of course arrested in	1	0	.4	an an an an an an an an an an an an an a		12	2.5	Anno ann an an Anna an Anna an Anna an Anna an Anna an Anna an Anna an Anna an Anna an Anna an Anna an Anna an		14	1.0	
Approach LOS				alan Alan - Alan Andrea	-		A				В			E	3	

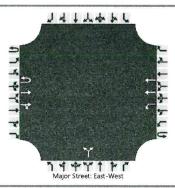
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PM PEAK HOUR

General Information		Site Information								
Analyst	J. Buckholz	Intersection	NE Park Street / SE 13th Avenue							
Agency/Co.	BUCKHOLZ TRAFFIC	Jurisdiction	Okeechobee County							
Date Performed	2/14/2023	East/West Street	NE Park Street							
Analysis Year	2023	North/South Street	SE 13th Avenue							
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.91							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	#23-1820	หมายของการ พระพบของการการเพื่องหมาย และสาวอย่างการการการที่สาวาทสามสะบบของสุดที่สามสะบบของสมบองสามสะบบของสาวาท	หม ื ดสมมณฑายางแขนของสมมณฑายางการการการการการการการการการการการการการก							

Lanes



Vehicle Volumes and Adjustments

Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	T	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	1	0	2	0	0	1	2	0		0	1	0		0	0	0
Configuration	U	2006 () 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 	Т	TR		L	Т	200 manufalan di munu di kang di kang di ka			LR				1000-000-000-000-000-000-000-000-000-00	
Volume (veh/h)	5		1230	14	2	31	1136	Norden and Angel Grand		2	1	16				Í
Percent Heavy Vehicles (%)	0				0	3			1	0		0				
Proportion Time Blocked									1							
Percent Grade (%)			lanan series sa muna series d			3					0	Concentration		Canada anno anno anno anno anno anno anno	den en en en en en en en en en en en en e	descenceuro
Right Turn Channelized							lan bere dia kenari say tar		Í		inousta faindiston allo and	ndan (Darke recommendada)			ALCO OCUMA ADOLUZIO	uti niataria
Median Type Storage				Left -	- Thru				1		anan manan manan manan manan ka]	1			o iveralo dur idan iya
Critical and Follow-up H	leadway	ys														
Base Critical Headway (sec)	6.4	and a minimum of a disal time operator	provinsion of a star build	oranica software o	6.4	4.1				7.5		6.9	adalah di kasalah palah yang di kasalah sebut kasalah palah sebut kasalah palah sebut kasalah palah sebut kasa			Γ
Critical Headway (sec)	6.40	all and a standard of the processo		2300-1100-010-010-010-010-010-010-010-010	6.40	4.16				6.80		6.90	naları inselistiştirderi oler	1		
Base Follow-Up Headway (sec)	2.5	nerostoren seuren sen			2.5	2.2		*******		3.5	Contraction of the second second second second second second second second second second second second second s	3.3	anadaalianii aasiinnaadoonsaa	an an too all the all the second demonstrations	Desclosivential segred contact	
Follow-Up Headway (sec)	2.50	sin California Solution for the second second		-1008,000,000,000,000,000,000	2.50	2.23		andan arabata pa		3.50		3.30				
Delay, Queue Length, ar	nd Leve	l of Se	ervice		Annonanan managanan ma		denne on manned	diculog "revisitive destage			ano and a second second second second second second second second second second second second second second se	NAME AND ADDRESS OF ADDRESS OF ADDRESS OF ADDRESS OF ADDRESS OF ADDRESS OF ADDRESS OF ADDRESS OF ADDRESS OF ADD		2 March Concernment		-
Flow Rate, v (veh/h)	5				[36			[20					
Capacity, c (veh/h)	234			eelihtiissa hiteeskä taihun		449					329					
v/c Ratio	0.02	and the contract of the second second second second second second second second second second second second se		eder Mourn Groetpur		0.08			Parton de l'Anna de la composition de	and the second se	0.06		almatic at a name of the land			
95% Queue Length, Q ₉₅ (veh)	0.1	anta di Manda Yanakanya aktifa		en Michael Mithe Mithemal		0.3			1		0.2					
Control Delay (s/veh)	20.8	****				13.7					16.6		100,010,000,000,000,000			1
Level of Service (LOS)	С					В		998 AD 2014 AV 000 (A 131), DB (94			С					
Approach Delay (s/veh)		0	.1	gomu Scholarum (order Landa		0	.4	ettelakon markaten kar		16	5.6				hereesseessaatassaatassa	Baccineration
Approach LOS	1	ł	4	anchi ine Amerikan (Kerke eta)			Ą			(C			enerren ganten (konstanten (non milliopethiop secontaria and	

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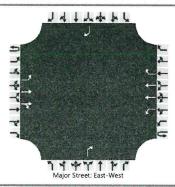
			HCS T	WO-	way	Stop	o-Cor	ntol	кер	ort						
General Information							Site I	nforr	natio	n						
Analyst	J. Buc	kholz					Inters	ection			NE P	ark Street	t / Hamp	oton Inn	Drivewa	ıy
Agency/Co.	BUCK	HOLZ 1	RAFFIC				Jurisd	iction		ža (žela	Okee	chobee (County	diandonialan diananan		
Date Performed	2/14/	2023	anational in state and a state in the	907/2499/Agendericangeouscow	NDSNINGSKI KONESKI K	******	East/V	Vest Str	eet	anan karala kulonkulo kulo kara	NE P	ark Stree		99997999999999999999999999999999999999	******	an an an an an an an an an an an an an a
Analysis Year	2023		20000000000000000000000000000000000000	angen biogramme try any			North	/South S	Street		Ham	pton Inn	Drivewa	У		*****
Time Analyzed	Week	day PN	l Peak Ho	ur	*****************		Peak I	Hour Fac	ctor	99-9-92 - 92 - 92 - 92 - 92 - 92 - 92 -	0.91	*********	999.4999.9999.9999.9999.9999.999	****	anas diana kanakana di bina ka	*****
Intersection Orientation	East-	West	an an an an an an an an an an an an an a				Analys	sis Time	Period ((hrs)	0.25		and the source of the source o	ndandin dan kurakan	ADDU-HUNTISTOCKASTICK	
Project Description	#23-1	1820	86÷894683829678686459488	14,6474-992 M(1992-902-909) M(19	107399099999999999999999999999999999999	ummi Androszkowanista		********	entra anti-	00000000000000000000000000000000000000	สร้องของสงคมสรร	********	understa käänen optie Optieran		nanoènakananananan	
Lanes																an Shide an Anna Anna Anna
						or Street: Ea										
Vehicle Volumes and Adj	ustme	ents														
Approach		East	bound			West	bound			North	nbound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	T	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	1	2	0		0	0	1		0	0	0
Configuration			Т	TR		L	Т					R				
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Proportion Time Blocked																
Percent Grade (%)											0					
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General Information		Site Information								
Analyst	J. Buckholz	Intersection	NE Park Street / SE 13th Avenue							
Agency/Co.	BUCKHOLZ TRAFFIC	Jurisdiction	Okeechobee County							
Date Performed	7/7/2023	East/West Street	NE Park Street							
Analysis Year	2024	North/South Street	SE 13th Avenue							
Time Analyzed	PM Peak Hr. BUILD Traffic	Peak Hour Factor	0.91							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	#23-1820		หม ่ง และสามารถสายสายสายสายสายสายสายสายสายสายสายสายสายส							

Lanes



Vehicle Volumes and Adjustments

Approach		East	bound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	T	R	U	L	T	R	U	L	Т	R
Priority	10	1	2	3	40	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	1	Chars Driverna ruoranoon	0	0	1		0	0	1
Configuration		L	Т	TR		L.	T	R		Periodi anti anti anti anti anti anti	all of the second second	R				R
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Critical and Follow-up H	leadwa	ys														
Base Critical Headway (sec)	6.4	4.1			6.4	4.1						6.9				6.9
Critical Headway (sec)	6.44	4.14			6.40	4.16						6.90				6.94
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Follow-Up Headway (sec)	2.52	2.22			2.50	2.23					1.1	3.30				3.32
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General Information							Site I	Inform	natio	n						
Analyst	J. Buc	kholz		anna fang offerie Allerte		adarda ayo na yang kana	Inters	ection			NE Pa	ark St. / H	lamptor	n Inn / N	E 12th A	ve.
Agency/Co.	BUCK	HOLZ T	RAFFIC	************	laatus ike usatasikaa		Jurisd	iction		10000000000000000000000000000000000000	Okee	chobee (County	Advante-aktions	*******	
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Analysis Year	2024			29794-0-50004250740			North	/South !	Street	*********	Ham	pton Inn	Drivewa	y / NE 1	2th Aver	nue
Time Analyzed	PM P	eak Hr.	BUILD Tra	affic			Peak I	Hour Fa	ctor		0.91					*****
Intersection Orientation	East-	West			******		Analy	sis Time	Period	(hrs)	0.25		arionofarational	*******		
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Approach		East	bound			West	bound			North	bound			South	bound	
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Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	1	2	0	1	0	0	1		0	0	1
Configuration			Т	TR		L	Т	TR				R				R
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OKEECHOBEE UTILITY AUTHORITY

100 SW 5th Avenue Okeechobee, Florida 34974-4221

> (863) 763-9460 FAX: (863) 467-4335

November 7, 2023

Mr. Jeff Liegel S & L Properties Okeechobee, LLC 2651 Kirking Court Portage, WI 53901

Ref: Water Capacity Request

Parcel ID: 2-15-37-35-0A00-00010-0000 3-15-37-35-0210-00010-0010 City of Okeechobee, Okeechobee County

Dear Mr. Liegel:

In reference to a request for the availability of water capacity to the subject property, I submit the following information for your use in meeting the potable water demand requirements for the project.

The Okeechobee Utility Authority owns and operates two water treatment plants with a combined treatment capacity of 6 MGD. During the twelve month period from July 2022 to June 2023, the maximum daily flow was 3.37 MGD, or about 65% of capacity. The OUA does have a potable water distribution main in State Road 70E right-of-way near the subject property. Any upgrade requirements to the water system infrastructure due to the demands of the proposed project will be at the developer's expense.

Should you have any questions, comments or concerns with regards to the water system capacity, please contact the OUA at 863-763-9460.

Sincerely,

John F. Hayford Executive Director Okeechobee Utility Authority



OKEECHOBEE UTILITY AUTHORITY

100 SW 5th Avenue Okeechobee, Florida 34974-4221

> (863) 763-9460 FAX: (863) 467-4335

November 7, 2023

Mr. Jeff Liegel S & L Properties Okeechobee, LLC 2651 Kirking Court Portage, WI 53901

Ref: Wastewater Capacity Request

Parcel ID: 2-15-37-35-0A00-00010-0000 3-15-37-35-0210-00010-0010 City of Okeechobee, Okeechobee County

Dear Mr. Liegel:

In reference to a request of the availability of wastewater capacity for the subject property, I submit the following information for your use in the permitting for the above referenced project.

The Okeechobee Utility Authority owns and operates one regional wastewater treatment plant with a FDEP permitted capacity of 3.9 MGD. During the twelve month period from July 2022 to June 2023, the annual average daily demand was 0.967 MGD, or about 25% of the current 3.9 MGD treatment capacity. The OUA has a wastewater force main near the subject property. Any infrastructure improvements required by this project will be at the developer's expense.

Should you have any other questions, comments or concerns with regards to the wastewater system capacity, please contact the OUA at 863.763.9460.

Sincerely,

John R_Hayford Executive Director Okeechobee Utility Authority

September 20, 2023

RE: Culver's Restaurants Drive-Through Queueing Observations

To Whom it may Concern:

Kimley-Horn has conducted drive-through queueing observations at two existing Culver's locations:

- 7818 Gate Parkway in Jacksonville, Florida (approx. 5,000 gross square feet)
- 1767 Blanding Boulevard in Middleburg, Florida (approx. 5,100 gross square feet)

Based on the hourly distribution of vehicle trips by land use, published by the Institute of Transportation Engineers in *Trip Generation*, 11th Edition, the highest weekday three-hour period of trip generation for a fast-food restaurant with drive-through is 11:00 AM to 2:00 PM. Accordingly, data collection at both existing Culver's locations occurred from 11:00 AM to 2:00 PM. This letter summarizes the queueing observations and recommendations for future Culver's restaurant designs.

Gate Parkway Culver's Queueing Observations

Drive-through queueing data was collected at the existing Gate Parkway Culver's on Thursday, November 17, 2022. At this location, one Kimley-Horn staff member documented the queue behind the order boards (including vehicles at the order boards), and one Kimley-Horn staff member documented the queues ahead of the order boards (including queues between the order boards and the pay window and queues ahead of the pay window in the order waiting spaces). Kimley-Horn staff also noted whether the space between the order boards and the pay window filled up (whether the queue in front of the order boards backed up to the order boards). This allowed determination of whether the order board or the area ahead of the order board was the controlling factor for the maximum queue behind the order board.

Queues were noted in two-minute intervals. The queues noted for each interval represent the maximum queue that occurred over that two-minute interval. The following observations were noted at the Gate Parkway Culver's:

- This is a double drive-through facility and both drive-through lanes were fully operational.
- From approximately 11:30 AM to 2:00 PM, two employees stood at the order boards (one at each order board) and took orders from the customers.

- It appeared that customers paying with credit card paid the employees that were standing at the order boards, skipped the pay window, and proceeded to the order waiting area.
- It appeared that customers paying with cash paid at the pay window.
- The maximum queue noted behind the order boards (including vehicles at the order boards) was 10 vehicles.
- During the entire observation period, the queue behind the pay window only backed up to the order boards one time, and this backup lasted approximately two minutes (from 12:59 PM to 1:01 PM). During this time, there were several empty spaces ahead of the pay window in the order waiting spaces. The queue behind the order boards from 12:59 PM to 1:01 PM was a maximum of 6 vehicles.

Based on these observations, the queue storage for future sites in design should be based on the maximum queue observed behind the order boards rather than measuring the queue from the pay window.

Blanding Boulevard Culver's Queueing Observations

Drive-through queueing data was collected at the existing Blanding Boulevard Culver's on Tuesday, November 29, 2022. At this location, one Kimley-Horn staff member documented the queue behind the order board (including the vehicle at the order board). Based on the previous observations at the Gate Parkway location, queues ahead of the order board were not documented, as these queues were not observed to affect the queues behind the order boards.

The following observations were noted at the Blanding Boulevard Culver's:

- This is a single drive-through facility.
- Unlike the Gate Parkway location, customers at the Blanding Boulevard location ordered directly from the order board (not from an employee standing near the order board) and paid at the pay window.
- The maximum queue noted behind the order board (including the vehicle at the order board) was 6 vehicles.
- During the entire observation period, the queue behind the pay window backed up to the order board three times (at 12:07 PM, at 1:10 PM, and at 1:17 PM), each time lasting less than 2 minutes.

The data collection sheets for each location are attached.

Conclusion

On-site queueing observations were conducted at two existing Culver's restaurants located on Gate Parkway in Jacksonville, Florida and on Blanding Boulevard in Middleburg, Florida. At the Gate Parkway location (double drive-through), the maximum queue observed behind the order boards was 10 vehicles, including the vehicles at the order boards. At the Blanding Boulevard location (single drive-through), the maximum queue observed behind the order board was 6 vehicles, including the vehicle at the order board. At both sites, the space between the pay window and the order boards was rarely ever filled by the queue, and therefore the controlling factor for the maximum queue was the order boards.

Based on the observations, future Culver's restaurants should be designed to accommodate at least 10 vehicles behind the order boards on site, including vehicles parked at the order boards.

Please do not hesitate to contact me at (904) 828-3900 or <u>jack.hulsberg@kimley-horn.com</u> should you have any questions.

Sincerely, KIMLEY-HORN AND ASSOCIATES, INC.

Jack Hulsberg, P.E. Project Manager

This item has been digitally signed and sealed by:

Jack Hulsberg Date: 2023.09.20 11:15:46 -04'00'



on the date adjacent to the signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

Date: Thursday, November 17, 2022

	Time	Queue Behind Order Boards (Including	Did Space Between Window and	Queue Between Window	Vehicles in Order
	To	vehicles at the order boards)	Order Board Fill Up?	and Order Board	Waiting Spaces
~	1:02 PM	4	Yes	6	2
-	1:04 PM	4	No	2	9
~	1:06 PM	1	No	0	7
	1:08 PM	2	No	1	4
•••	1:10 PM	2	No	0	w
	1:12 PM	£	No	1	9
	1:14 PM	m	No	0	5
	1:16 PM	4	No	0	7
	1:18 PM	8	No	0	2
	1:20 PM	10	No	0	Q
E	1:22 PM	σι	No	щ	7
	1:24 PM	6	No	r.	9
	1:26 PM	10	No	a	٤
	1:28 PM	10	No	0	80
	1:30 PM	7	No	2	8
	1:32 PM	'n	No	4	9
	1:34 PM	4	No	3	7
	1:36 PM	ε	No	2	89
	1:38 PM	2	No	з	5
	1:40 PM	5	No	0	2
	1:42 PM	5	No	0	£
	1:44 PM	2	No	1	4
	1:46 PM	1	No	2	4
	1:48 PM	æ	No	o	4
	1:50 PM	5	No	0	9
	1:52 PM	4	No	1	9
	1:54 PM	m	No	£	5
	1:56 PM	m	No	0	8
	1:58 PM	2	No	1	7
	2:00 PM	2	No	1	9

Ë	Time	Queue Behind Order Boards	Did Space Between	Queue Between	Vehicles In Order
From	To	(including vehicles at the order boards)	window and Order Board Fill Up?	window and Order Board	Waiting Spaces
12:00 PM	12:02 PM	2	No	1	9
12:02 PM	12:04 PM	æ	No	0	٢
12:04 PM	12:06 PM	3	No	0	9
12:06 PM	12:08 PM	m	No	1	7
12:08 PM	12:10 PM	4	No	3	'n
12:10 PM	12:12 PM	9	No	m	ы
12:12 PM	12:14 PM	7	No	0	9
12:14 PM	12:16 PM	6	No	σ	ц,
12:16 PM	12:18 PM	Q	No	0	1
12:18 PM	12:20 PM	ŝ	No	0	6
12:20 PM	12:22 PM	1	No	0	7
12:22 PM	12:24 PM	0	No	o	S
2:24 PM	12:26 PM	з	No	0	2
12:26 PM	12:28 PM	2	No	0	2
12:28 PM	12:30 PM	2	No	o	2
12:30 PM	12:32 PM	2	No	0	4
12:32 PM	12:34 PM	1	No	0	4
12:34 PM	12:36 PM	4	No	1	5
12:36 PM	12:38 PM	4	No	2	5
12:38 PM	12:40 PM	£	No	0	6
12:40 PM	12:42 PM	ĸ	No	0	7
12:42 PM	12:44 PM	m	No	1	9
2:44 PM	12:46 PM	1	No	1	7
12:46 PM	12:48 PM	1	No	0	£
12:48 PM	12:50 PM	2	Na	0	4
12:50 PM	12:52 PM	2	No	o	4
12:52 PM	12:54 PM	Ħ	No	٥	4
2:54 PM	12:56 PM	4	No	0	4
12:56 PM	12:58 PM	5	No	ŝ	4
12:58 PM	1:00 PM	Q	Yes	2	m

	Location: Analysts:	Culver's 7818 Gate Parkway Jacksonville, FL 32256 Ryen Clark and Jack Huisberg	Gate Parkway 1: 32256 d Jack Hulsberg		
1	Time	Queue Behind Order Boards	Did Space Between	Queue Between	Vehicles in Order
From	To	(Including vehicles at the order boards)	Window and Order Board Fill Up?	Window and Order Board	Waiting Spaces
11:00 AM	11:02 AM	2	No	2	1
11:02 AM	11:04 AM	1	No	2	1
11:04 AM	11:06 AM	0	No	2	1
11:D6 AM	11:08 AM	1	No	0	3
11:08 AM	11:10 AM	S	No	0	3
11:10 AM	11:12 AM	5	Na	0	2
11:12 AM	11:14 AM	5	No	0	2
11:14 AM	11:16 AM	5	No	0	1
11:16 AM	11:18 AM	3	No	2	2
11:18 AM	11:20 AM	3	No	4	2
11:20 AM	11:22 AM	3	No	3	m
11:22 AM	11:24 AM	6	No	1	5
11:24 AM	11:26 AM	ი	No	0	5
11:26 AM	11:28 AM	ę	No	0	S
11:28 AM	11:30 AM	£	No	0	7
11:30 AM	11:32 AM	3	No	0	80
11:32 AM	11:34 AM	4	No	0	8
11:34 AM	11:36 AM	2	No	2	9
11:36 AM	11:38 AM	1	No	0	7
11:38 AM	11:40 AM	1	No	0	9
11:40 AM	11:42 AM	2	No	0	3
11:42 AM	11:44 AM	r 4	No	o	Ļ
11:44 AM	11:46 AM	2	No	0	2
11:46 AM	11:48 AM	0	No	0	m
11:48 AM	11:50 AM	Ħ	No	0	en
11:50 AM	11:52 AM	г,	No	0	2
11:52 AM	11:54 AM	4	No	0	£
11:54 AM	11:56 AM	2	No	. FT	4
11:56 AM	11:58 AM	7	No	0	9
11:58 AM	12:00 PM	3	No	0	ę

Location: Culver's 1767 Blanding Blvd Middleburg, FL 32068 Analyst: Ryen Clark

Ti	me	Queue Behind Order
From	То	Board (Including vehicle at the order board)
11:00 AM	11:02 AM	1
11:02 AM	11:04 AM	1
11:04 AM	11:06 AM	0
11:06 AM	11:08 AM	2
11:08 AM	11:10 AM	2
11:10 AM	11:12 AM	1
11:12 AM	11:14 AM	0
11:14 AM	11:16 AM	0
11:16 AM	11:18 AM	1
11:18 AM	11:20 AM	1
11:20 AM	11:22 AM	3
11:22 AM	11:24 AM	3
11:24 AM	11:26 AM	2
11:26 AM	11:28 AM	1
11:28 AM	11:30 AM	1
11:30 AM	11:32 AM	1
11:32 AM	11:34 AM	1
11:34 AM	11:36 AM	2
11:36 AM	11:38 AM	2
11:38 AM	11:40 AM	1
11:40 AM	11:42 AM	1
11:42 AM	11:44 AM	1
11:44 AM	11:46 AM	2
11:46 AM	11:48 AM	2
11:48 AM	11:50 AM	1
11:50 AM	11:52 AM	3
11:52 AM	11:54 AM	3
11:54 AM	11:56 AM	2
11:56 AM	11:58 AM	1
11:58 AM	12:00 PM	1

[l
Ti	me	Queue Behind Order Board (Including vehicle
From	То	at the order board)
12:00 PM	12:02 PM	2
12:02 PM	12:04 PM	2
12:04 PM	12:06 PM	4
12:06 PM	12:08 PM	2
12:08 PM	12:10 PM	1
12:10 PM	12:12 PM	2
12:12 PM	12:14 PM	1
12:14 PM	12:16 PM	0
12:16 PM	12:18 PM	1
12:18 PM	12:20 PM	2
12:20 PM	12:22 PM	1
12:22 PM	12:24 PM	1
12:24 PM	12:26 PM	1
12:26 PM	12:28 PM	1
12:28 PM	12:30 PM	1
12:30 PM	12:32 PM	0
12:32 PM	12:34 PM	5
12:34 PM	12:36 PM	6
12:36 PM	12:38 PM	5
12:38 PM	12:40 PM	3
12:40 PM	12:42 PM	4
12:42 PM	12:44 PM	1
12:44 PM	12:46 PM	1
12:46 PM	12:48 PM	1
12:48 PM	12:50 PM	0
12:50 PM	12:52 PM	2
12:52 PM	12:54 PM	2
12:54 PM	12:56 PM	1
12:56 PM	12:58 PM	1
12:58 PM	1:00 PM	1

Date: Tuesday, November 29, 2022

1

Ti	me	Queue Behind Order Board (Including vehicle
From	То	at the order board)
1:00 PM	1:02 PM	1
1:02 PM	1:04 PM	4
1:04 PM	1:06 PM	4
1:06 PM	1:08 PM	4
1:08 PM	1:10 PM	5
1:10 PM	1:12 PM	б
1:12 PM	1:14 PM	6
1:14 PM	1:16 PM	4
1:16 PM	1:18 PM	5
1:18 PM	1:20 PM	4
1:20 PM	1:22 PM	3
1:22 PM	1:24 PM	4
1:24 PM	1:26 PM	2
1:26 PM	1:28 PM	2
1:28 PM	1:30 PM	1
1:30 PM	1:32 PM	2
1:32 PM	1:34 PM	1
1:34 PM	1:36 PM	1
1:36 PM	1:38 PM	2
1:38 PM	1:40 PM	1
1:40 PM	1:42 PM	2
1:42 PM	1:44 PM	0
1:44 PM	1:46 PM	1
1:46 PM	1:48 PM	2
1:48 PM	1:50 PM	5
1:50 PM	1:52 PM	4
1:52 PM	1:54 PM	2
1:54 PM	1:56 PM	4
1:56 PM	1:58 PM	3
1:58 PM	2:00 PM	1

September 20, 2023

RE: Culver's Restaurants Drive-Through Queueing Observations

To Whom it may Concern:

Kimley-Horn has conducted drive-through queueing observations at two existing Culver's locations:

- 7818 Gate Parkway in Jacksonville, Florida (approx. 5,000 gross square feet)
- 1767 Blanding Boulevard in Middleburg, Florida (approx. 5,100 gross square feet)

Based on the hourly distribution of vehicle trips by land use, published by the Institute of Transportation Engineers in *Trip Generation*, 11th Edition, the highest weekday three-hour period of trip generation for a fast-food restaurant with drive-through is 11:00 AM to 2:00 PM. Accordingly, data collection at both existing Culver's locations occurred from 11:00 AM to 2:00 PM. This letter summarizes the queueing observations and recommendations for future Culver's restaurant designs.

Gate Parkway Culver's Queueing Observations

Drive-through queueing data was collected at the existing Gate Parkway Culver's on Thursday, November 17, 2022. At this location, one Kimley-Horn staff member documented the queue behind the order boards (including vehicles at the order boards), and one Kimley-Horn staff member documented the queues ahead of the order boards (including queues between the order boards and the pay window and queues ahead of the pay window in the order waiting spaces). Kimley-Horn staff also noted whether the space between the order boards and the pay window filled up (whether the queue in front of the order boards backed up to the order boards). This allowed determination of whether the order board or the area ahead of the order board was the controlling factor for the maximum queue behind the order board.

Queues were noted in two-minute intervals. The queues noted for each interval represent the maximum queue that occurred over that two-minute interval. The following observations were noted at the Gate Parkway Culver's:

- This is a double drive-through facility and both drive-through lanes were fully operational.
- From approximately 11:30 AM to 2:00 PM, two employees stood at the order boards (one at each order board) and took orders from the customers.

- It appeared that customers paying with credit card paid the employees that were standing at the order boards, skipped the pay window, and proceeded to the order waiting area.
- It appeared that customers paying with cash paid at the pay window.
- The maximum queue noted behind the order boards (including vehicles at the order boards) was 10 vehicles.
- During the entire observation period, the queue behind the pay window only backed up to the order boards one time, and this backup lasted approximately two minutes (from 12:59 PM to 1:01 PM). During this time, there were several empty spaces ahead of the pay window in the order waiting spaces. The queue behind the order boards from 12:59 PM to 1:01 PM was a maximum of 6 vehicles.

Based on these observations, the queue storage for future sites in design should be based on the maximum queue observed behind the order boards rather than measuring the queue from the pay window.

Blanding Boulevard Culver's Queueing Observations

Drive-through queueing data was collected at the existing Blanding Boulevard Culver's on Tuesday, November 29, 2022. At this location, one Kimley-Horn staff member documented the queue behind the order board (including the vehicle at the order board). Based on the previous observations at the Gate Parkway location, queues ahead of the order board were not documented, as these queues were not observed to affect the queues behind the order boards.

The following observations were noted at the Blanding Boulevard Culver's:

- This is a single drive-through facility.
- Unlike the Gate Parkway location, customers at the Blanding Boulevard location ordered directly from the order board (not from an employee standing near the order board) and paid at the pay window.
- The maximum queue noted behind the order board (including the vehicle at the order board) was 6 vehicles.
- During the entire observation period, the queue behind the pay window backed up to the order board three times (at 12:07 PM, at 1:10 PM, and at 1:17 PM), each time lasting less than 2 minutes.

The data collection sheets for each location are attached.

Conclusion

On-site queueing observations were conducted at two existing Culver's restaurants located on Gate Parkway in Jacksonville, Florida and on Blanding Boulevard in Middleburg, Florida. At the Gate Parkway location (double drive-through), the maximum queue observed behind the order boards was 10 vehicles, including the vehicles at the order boards. At the Blanding Boulevard location (single drive-through), the maximum queue observed behind the order board was 6 vehicles, including the vehicle at the order board. At both sites, the space between the pay window and the order boards was rarely ever filled by the queue, and therefore the controlling factor for the maximum queue was the order boards.

Based on the observations, future Culver's restaurants should be designed to accommodate at least 10 vehicles behind the order boards on site, including vehicles parked at the order boards.

Please do not hesitate to contact me at (904) 828-3900 or <u>jack.hulsberg@kimley-horn.com</u> should you have any questions.

Sincerely, KIMLEY-HORN AND ASSOCIATES, INC.

Jack Hulsberg, P.E. Project Manager

This item has been digitally signed and sealed by:

Jack Hulsberg Date: 2023.09.20 11:15:46 -04'00'



on the date adjacent to the signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

Date: Thursday, November 17, 2022

	Time	Queue Behind Order Boards (Includine	Did Space Between Window and	Queue Between Window	Vehicles in Order
	To	vehicles at the order boards)	Order Board Fill Up?	and Order Board	Waiting Spaces
~	1:02 PM	4	Yes	6	2
-	1:04 PM	4	No	2	9
~	1:06 PM	1	No	0	7
	1:08 PM	2	No	1	4
•••	1:10 PM	2	No	0	w
	1:12 PM	£	No	1	9
	1:14 PM	m	No	0	5
	1:16 PM	4	No	0	7
	1:18 PM	8	No	0	2
	1:20 PM	10	No	0	Q
E	1:22 PM	σι	No	T	7
	1:24 PM	6	No	Ŧ	9
	1:26 PM	10	No	Q	٤
	1:28 PM	10	No	0	80
	1:30 PM	7	No	2	8
	1:32 PM	'n	No	4	9
	1:34 PM	4	No	3	7
	1:36 PM	8	No	2	89
	1:38 PM	2	No	3	5
	1:40 PM	5	No	0	2
	1:42 PM	5	No	0	£
	1:44 PM	2	No	Ļ	4
	1:46 PM	1	No	2	4
	1:48 PM	æ	No	o	4
	1:50 PM	5	No	0	9
	1:52 PM	4	No	1	9
	1:54 PM	m	No	£	5
	1:56 PM	m	No	0	8
	1:58 PM	2	No	1	7
	2:00 PM	2	No	1	9

L L	Time	Queue Behind Order Boards	Did Space Between	Queue Between	Vehicles In Order
From	To	vehicles at the order boards)	window and Order Board Fill Up?	window and Order Board	Waiting Spaces
L2:00 PM	12:02 PM	2	No	1	9
12:02 PM	12:04 PM	e	No	0	7
12:04 PM	12:06 PM	£	No	0	9
12:06 PM	12:08 PM	e	No	1	7
12:08 PM	12:10 PM	4	oN	8	'n
12:10 PM	12:12 PM	9	No	m	5
12:12 PM	12:14 PM	7	No	0	9
12:14 PM	12:16 PM	6	No	σ	e
12:16 PM	12:18 PM	Q	No	0	4
12:18 PM	12:20 PM	m	No	0	ი
12:20 PM	12:22 PM	1	No	0	7
12:22 PM	12:24 PM	o	No	٥	S
2:24 PM	12:26 PM	з	No	0	2
12:26 PM	12:28 PM	2	No	0	2
12:28 PM	12:30 PM	2	No	ο	2
12:30 PM	12:32 PM	2	No	0	4
12:32 PM	12:34 PM	7	No	0	4
12:34 PM	12:36 PM	4	°N No	Ţ	S
12:36 PM	12:38 PM	4	No	2	5
12:38 PM	12:40 PM	3	No	a	9
12:40 PM	12:42 PM	Э	No	0	2
12:42 PM	12:44 PM	3	No	1	9
2:44 PM	12:46 PM	1	No	1	7
12:46 PM	12:48 PM	1	No	0	S
12:48 PM	12:50 PM	2	Na	0	4
12:50 PM	12:52 PM	2	No	0	4
12:52 PM	12:54 PM	Ħ	No	٥	4
2:54 PM	12:56 PM	4	No	0	4
12:56 PM	12:58 PM	5	No	3	4
12:58 PM	1:00 PM	9	Yes	5	æ

	Location: Analysts:	Culver's 7818 Gate Parkway Jacksonville, FL 32256 Ryen Clark and Jack Huisberg	Gate Parkway 1: 32256 d Jack Hulsberg		
1	Time	Queue Behind Order Boards	Did Space Between	Queue Between	Vehicles in Order
From	To	(Including vehicles at the order boards)	Window and Order Board Fill Up?	Window and Order Board	Waiting Spaces
11:00 AM	11:02 AM	2	No	2	1
11:02 AM	11:04 AM	1	No	2	1
11:04 AM	11:06 AM	0	No	2	1
11:D6 AM	11:08 AM	1	No	0	3
11:08 AM	11:10 AM	S	No	0	3
11:10 AM	11:12 AM	5	Na	0	2
11:12 AM	11:14 AM	5	No	0	2
11:14 AM	11:16 AM	5	No	0	1
11:16 AM	11:18 AM	3	No	2	2
11:18 AM	11:20 AM	3	No	4	2
11:20 AM	11:22 AM	3	No	3	m
11:22 AM	11:24 AM	6	No	1	5
11:24 AM	11:26 AM	6	No	0	5
11:26 AM	11:28 AM	ę	No	0	S
11:28 AM	11:30 AM	£	No	0	7
11:30 AM	11:32 AM	3	No	0	80
11:32 AM	11:34 AM	4	No	0	89
11:34 AM	11:36 AM	2	No	2	9
11:36 AM	11:38 AM	1	No	0	7
11:38 AM	11:40 AM	1	No	0	9
11:40 AM	11:42 AM	2	No	0	3
11:42 AM	11:44 AM	r 4	No	o	Ļ
11:44 AM	11:46 AM	2	No	0	2
11:46 AM	11:48 AM	o	No	0	m
11:48 AM	11:50 AM	Ħ	No	0	en
11:50 AM	11:52 AM	г,	No	0	2
11:52 AM	11:54 AM	4	No	0	£
11:54 AM	11:56 AM	2	No	, FI	4
11:56 AM	11:58 AM	7	No	0	9
11:58 AM	12:00 PM	æ	No	0	9

Location: Culver's 1767 Blanding Blvd Middleburg, FL 32068 Analyst: Ryen Clark

Ti	me	Queue Behind Order
From	То	Board (Including vehicle at the order board)
11:00 AM	11:02 AM	1
11:02 AM	11:04 AM	1
11:04 AM	11:06 AM	0
11:06 AM	11:08 AM	2
11:08 AM	11:10 AM	2
11:10 AM	11:12 AM	1
11:12 AM	11:14 AM	0
11:14 AM	11:16 AM	0
11:16 AM	11:18 AM	1
11:18 AM	11:20 AM	1
11:20 AM	11:22 AM	3
11:22 AM	11:24 AM	3
11:24 AM	11:26 AM	2
11:26 AM	11:28 AM	1
11:28 AM	11:30 AM	1
11:30 AM	11:32 AM	1
11:32 AM	11:34 AM	1
11:34 AM	11:36 AM	2
11:36 AM	11:38 AM	2
11:38 AM	11:40 AM	1
11:40 AM	11:42 AM	1
11:42 AM	11:44 AM	1
11:44 AM	11:46 AM	2
11:46 AM	11:48 AM	2
11:48 AM	11:50 AM	1
11:50 AM	11:52 AM	3
11:52 AM	11:54 AM	3
11:54 AM	11:56 AM	2
11:56 AM	11:58 AM	1
11:58 AM	12:00 PM	1

[l
Ti	me	Queue Behind Order Board (Including vehicle
From	То	at the order board)
12:00 PM	12:02 PM	2
12:02 PM	12:04 PM	2
12:04 PM	12:06 PM	4
12:06 PM	12:08 PM	2
12:08 PM	12:10 PM	1
12:10 PM	12:12 PM	2
12:12 PM	12:14 PM	1
12:14 PM	12:16 PM	0
12:16 PM	12:18 PM	1
12:18 PM	12:20 PM	2
12:20 PM	12:22 PM	1
12:22 PM	12:24 PM	1
12:24 PM	12:26 PM	1
12:26 PM	12:28 PM	1
12:28 PM	12:30 PM	1
12:30 PM	12:32 PM	0
12:32 PM	12:34 PM	5
12:34 PM	12:36 PM	6
12:36 PM	12:38 PM	5
12:38 PM	12:40 PM	3
12:40 PM	12:42 PM	4
12:42 PM	12:44 PM	1
12:44 PM	12:46 PM	1
12:46 PM	12:48 PM	1
12:48 PM	12:50 PM	0
12:50 PM	12:52 PM	2
12:52 PM	12:54 PM	2
12:54 PM	12:56 PM	1
12:56 PM	12:58 PM	1
12:58 PM	1:00 PM	1

Date: Tuesday, November 29, 2022

1

Ti	me	Queue Behind Order Board (Including vehicle
From	То	at the order board)
1:00 PM	1:02 PM	1
1:02 PM	1:04 PM	4
1:04 PM	1:06 PM	4
1:06 PM	1:08 PM	4
1:08 PM	1:10 PM	5
1:10 PM	1:12 PM	б
1:12 PM	1:14 PM	6
1:14 PM	1:16 PM	4
1:16 PM	1:18 PM	5
1:18 PM	1:20 PM	4
1:20 PM	1:22 PM	3
1:22 PM	1:24 PM	4
1:24 PM	1:26 PM	2
1:26 PM	1:28 PM	2
1:28 PM	1:30 PM	1
1:30 PM	1:32 PM	2
1:32 PM	1:34 PM	1
1:34 PM	1:36 PM	1
1:36 PM	1:38 PM	2
1:38 PM	1:40 PM	1
1:40 PM	1:42 PM	2
1:42 PM	1:44 PM	0
1:44 PM	1:46 PM	1
1:46 PM	1:48 PM	2
1:48 PM	1:50 PM	5
1:50 PM	1:52 PM	4
1:52 PM	1:54 PM	2
1:54 PM	1:56 PM	4
1:56 PM	1:58 PM	3
1:58 PM	2:00 PM	1

October 23, 2023

Mr. Ben Smith, AICP Morris-Depew Associates, Inc. 2914 Cleveland Avenue Fort Myers, FL 33901

Re: Culver's Okeechobee Application Number: 23-004-TRC Submittal #1 Comments & Responses KHA #045523036

Mr. Smith:

Kimley-Horn and Associates, Inc. (Kimley-Horn) is in receipt of the City of Okeechobee TRC Staff Report email dated September 18, 2023. We prepared the following responses to the TRC Staff Reports comments. City of Okeechobee staff comments are shown below in bold, followed by our responses in italics.

1. Potable water and Sewer: Availability of potable water and sewer service for this site has not been included within the application submittal package.

Response: Please see the attached availability letter dated September 26, 2023.

2. Traffic Generation: The applicant has provided a traffic analysis prepared by American Civil Engineering Co. which demonstrates available roadway capacity for 4,600 sq/ft drive-through restaurant, which is less than the proposed 4,827 sq/ft facility.

Response: The master developer has previously provided the Traffic analysis and has determined that the difference in square footage is deemed negligible where a net increase of 13 total cars was accounted for.

3. Access and Internal Circulation: The site plan does not demonstrate that the access driveways and the internal drive aisles are a minimum 24' wide.

Response: Please see Sheets C-06 and C-07 for driveway and internal drive aisle dimensions.

4. Furthermore, the proposed cross-access easement is 20' wide, which is not consistent with the minimum required 24' wide drive aisle width. It is recommended that the proposed easement width is revised in the proposed plat.

Response: Please see Sheets C-06 and C-07 for cross-access easement width of 24' per the master developer's plat information provided.

5. Functionality of the drive through will be determined as part of the special use exception for that feature.

Response: Acknowledged.

6. A crosswalk for pedestrians should be provided through the drive-through lanes to access the proposed sidewalk.

Response: Please see sheet C-06 for northern most pedestrian walkway/crosswalk along eastern side of the building with appropriate signage.

 Dumpster Location and/or Trash Collection: Multiple parking spaces obstruct access to the dumpster. In addition, accessing the dumpster requires the obstruction of the proposed pedestrian walkway to provide removal services.

Response: Please see Sheet C-06 for revised dumpster pad area. The parking stalls have been removed from in front of the dumpster. Also, per our discussion with Ben Smith on 10/11/23, the Culver's will coordinate with the solid waste entity to ensure that the dumpster is serviced after hours so that it can traverse the parking areas without conflict with customer vehicles.

8. Minimum parking space dimensions: Dimensions of the proposed spaces are not provided.

Response: Please see Sheets C-06 and C-07 for parking dimensions.

 Minimum parking access dimensions: Drive aisle width is not indicated on the site plan. A minimum 24' width must be provided for all driveways and 2-way access aisles, including cross-access easement.

Response: Please see Sheets C-06 and C-07 for drive aisle and cross-access easement widths.

10. Parking and loading space layout: No designated pedestrian crosswalks depicted.

Response: Please see sheet C-06 for northern most pedestrian walkway/crosswalk along eastern side of the building with appropriate signage.

11. Parking and loading space layout: No pedestrian walkway/crosswalk provided from northern most parking spaces to the proposed pedestrian walkway along the eastern side of the building.

Response: Please see sheet C-06 for northern most pedestrian walkway/crosswalk along eastern side of the building with appropriate signage.

12. Parking and loading space layout: Minimum 20' setback from "Lincoln Street" not indicated on the site plan.

Response: Please see Sheet C-06 for the 20' building setback label.

13. Minimum number of ADA parking spaces: Only 3 ADA spaces proposed.

Response: Please see Sheet C-06 for the additional ADA space that has been added.

14. Minimum ADA parking space dimensions: Space dimensions are not indicated on the site plan.

Response: Please see Sheets C-06 and C-07 for ADA parking space dimensions.

Landscaping Requirements for Parking and Vehicular Use Areas: 65 spaces required;
 1,170 sq/ft of landscaped area required.

Response: 77 parking spaces and 28,341 sq/ft of landscaped area has been provided.

16. Landscaping Requirements for Parking and Vehicular Use Areas: Width of the landscaped areas between the proposed building and the vehicular use areas is not provided.

Response: Please see Sheets C-06 and C-07 for landscaped areas width.

17. Landscaping Requirements for Parking and Vehicular Use Areas: Dimensions of the landscaped islands not provided.

Response: Please see Sheets C-06 and C-07 for landscaped island dimensions.

18. Landscape buffer areas: Buffer widths not completely provided. Only an 8' landscape buffer is proposed along SR-70 frontage.

Response: Please see Sheet C-06 for the proposed 10' Landscape Buffer.

19. Landscape design and plan: No pedestrian walkway proposed between the northern parking spaces and the proposed building.

Response: Please see sheet C-06 for northern most pedestrian walkway/crosswalk along eastern side of the building with appropriate signage.

20. Sidewalks: No sidewalks proposed along NE 13th Ave.

Response: The master developer shall provide sidewalks along NE 13th Ave as shown on Sheet C-06.

Page 3

Kimley *W* Horn

Recommendations:

1. Plans must be consistent with final plat approval and all conditions of that approval.

Response: Acknowledged, packages have been revised for consistency.

2. Building permits may not be issued until final plat approval of 23-003-TRC.

Response: Acknowledged.

3. A monument sign is proposed, although no sign plan has been submitted with the site plan application. (Appendix B, Information 11). Proposed signage location and design plans must be submitted for review.

Response: Please see Sheet C-06 for monument sign location as well as architectural details for design plans.

4. Availability of potable water and sewer service for this site must be demonstrated.

Response: Please see the attached availability letter dated September 26, 2023.

The traffic study must be revised for consistency with the proposed intensity of the principal structure and for uses allowed within the CHV zoning district.

Response: The traffic study was previously provided by the master developer and has been determined that the difference in net trips between the discrepancy between building SF to be negligible.

- 6. The applicant must provide dimensions and length of the proposed drive-through queue (all proposed lanes) to be determine adequacy of on-site traffic circulation.
 - a. Stacking lanes should be designed to minimize traffic congestion.
 - b. Typical vehicle dimensions for proposed queueing should be provided.

Response: The traffic design for the drive-through has been deemed appropriate through conversation with the city's planner.

7. Revise dumpster enclosure location to address conflicts with parking facility.

Response: Please see Sheet C-06 for revised dumpster pad area. The parking stalls have been removed from in front of the dumpster. Also, per our discussion with Ben Smith on 10/11/23, the Culver's will coordinate with the solid waste entity to ensure that the dumpster is serviced after hours so that it can traverse the parking areas without conflict with customer vehicles.

8. Landscaping is proposed within the utility easement. Public works to determine whether a condition of approval is necessary to address removal and replacement for utility maintenance.

Response: Acknowledged.

9. Dimensions for parking spaces, drive aisles, setbacks, landscape buffers and other features must be provided to determine compliance.

Response: Please see Sheet C-06 for parking spaces, drive aisles, setbacks, and landscape buffers dimensions.

10. Internal pedestrian walkways should be designated.

Response: Please see Sheet C-06 for designated pedestrian walkways.

11. A crosswalk for pedestrians should be provided through the drive-through lanes to access the proposed sidewalk.

Response: Please see sheet C-06 for northern most pedestrian walkway/crosswalk along eastern side of the building with appropriate signage.

12. A sidewalk should be provided on 13th Ave.

Response: The master developer shall provide and construct sidewalks along NE 13th Ave as shown on Sheet C-06.

I hope that the above responses are sufficient for you to complete the review of our application. Should you have any further questions, please feel free to contact me at (904) 828-3900.

Very truly yours,

KIMLEY-HORN AND ASSOCIATES, INC.

Josh Cockriel, PE Project Manager

MONUMENT SERIES

D/F Illuminated Monument Sign with EMC





C 30% Sammfield Sign & Scausers Int., Samingfield Sign, saringfieldrige sum

5. Of it a regiment fadorer of Women's Hung & Residuinty 3. Boston by Spring Tee & Barrison





SIGN DETAILS:

Colver's sign cobinet of aluminum construction and internally illuminated with LED's

Thermaterned polycarbonate faces, embassed Calver's, 3M 'HP viryt decoration

Watchfirs' 48x128 B68 tuit color 15mm Electronic Message Center (EMC) with RF wireless contanuncation

Amber EMC option available

Broadband communication option available

UL marked product

Culvers

SIGN & MENU BOARD BRAND STANDARDS WITH TECHNICAL SPECIFICATIONS

2-12